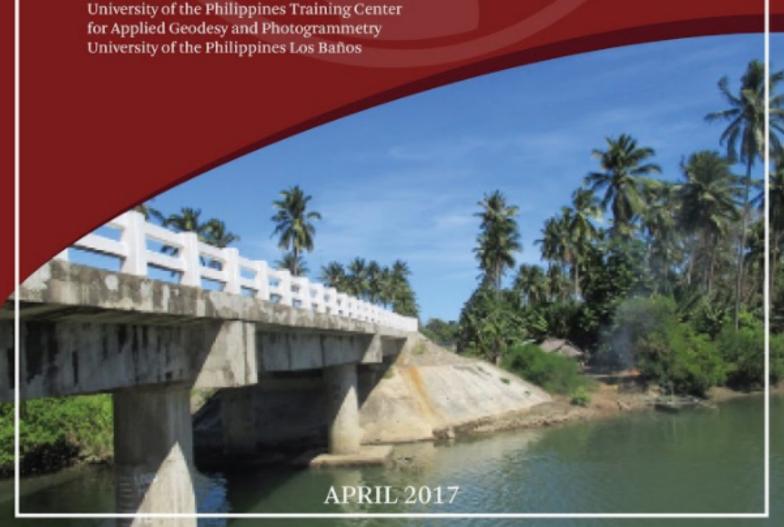
HAZARD MAPPING OF THE PHILIPPINES USING LIDAR (PHIL-LIDAR I)

# LiDAR Surveys and Flood Mapping of Inagauan River





Hazard Mapping of the Philippines Using LIDAR (Phil-LIDAR 1)





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Hazard Mapping of the Philippines Using LIDAR (Phil-LIDAR 1)

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# LIST OF ACRONYMS AND ABBREVIATIONS

AAC	Asian Aerospace Corporation
Ab	abutment
ALTM	Airborne LiDAR Terrain Mapper
ARG	automatic rain gauge
ATQ	Antique
AWLS	Automated Water Level Sensor
BA	Bridge Approach
BM	benchmark
CAD	Computer-Aided Design
CN	Curve Number
CSRS	Chief Science Research Specialist
DAC	Data Acquisition Component
DEM	Digital Elevation Model
DENR	Department of Environment and Natural Resources
DOST	Department of Science and Technology
DPPC	Data Pre-Processing Component
DREAM	Disaster Risk and Exposure Assessment for Mitigation [Program]
DRRM	Disaster Risk Reduction and Management
DSM	Digital Surface Model
DTM	Digital Terrain Model
DVBC	Data Validation and Bathymetry Component
FMC	Flood Modeling Component
FOV	Field of View
GiA	Grants-in-Aid
GCP	Ground Control Point
GNSS	Global Navigation Satellite System
GPS	Global Positioning System
HEC-HMS	Hydrologic Engineering Center - Hydrologic Modeling System
HEC-RAS	Hydrologic Engineering Center - River Analysis System
НС	High Chord
IDW	Inverse Distance Weighted [interpolation method]

IMU	Inertial Measurement Unit
kts	knots
LAS	LiDAR Data Exchange File format
LC	Low Chord
LGU	local government unit
LiDAR	Light Detection and Ranging
LMS	LiDAR Mapping Suite
m AGL	meters Above Ground Level
MMS	Mobile Mapping Suite
MSL	mean sea level
NSTC	Northern Subtropical Convergence
PAF	Philippine Air Force
PAGASA	Philippine Atmospheric Geophysical and Astronomical Services Administration
PDOP	Positional Dilution of Precision
PPK	Post-Processed Kinematic [technique]
PRF	Pulse Repetition Frequency
PTM	Philippine Transverse Mercator
QC	Quality Check
QT	Quick Terrain [Modeler]
RA	Research Associate
RIDF	Rainfall-Intensity-Duration-Frequency
RMSE	Root Mean Square Error
SAR	Synthetic Aperture Radar
SCS	Soil Conservation Service
SRTM	Shuttle Radar Topography Mission
SRS	Science Research Specialist
SSG	Special Service Group
ТВС	Thermal Barrier Coatings
UPC	University of the Philippines Cebu
UP-TCAGP	University of the Philippines – Training Center for Applied Geodesy and Photogrammetry
UTM	Universal Transverse Mercator
WGS	World Geodetic System

# CHAPTER 1: OVERVIEW OF THE PROGRAM AND INAGAUAN RIVER

Enrico C. Paringit, Dr. Eng., Asst. Prof. Edwin R. Abucay, and Ms. Sandra S. Samantela

# 1.1 Background of the Phil-LIDAR 1 Program

The University of the Philippines Training Center for Applied Geodesy and Photogrammetry (UP-TCAGP) launched a research program entitled "Nationwide Hazard Mapping using LiDAR" or Phil-LiDAR 1, supported by the Department of Science and Technology (DOST) Grants-in-Aid (GiA) Program. The program was primarily aimed at acquiring a national elevation and resource dataset at sufficient resolution to produce information necessary to support the different phases of disaster management. Particularly, it targeted to operationalize the development of flood hazard models that would produce updated and detailed flood hazard maps for the major river systems in the country.

Also, the program was aimed at producing an up-to-date and detailed national elevation dataset suitable for 1:5,000 scale mapping, with 50 cm and 20 cm horizontal and vertical accuracies, respectively. These accuracies were achieved through the use of the state-of-the-art Light Detection and Ranging (LiDAR) airborne technology procured by the project through DOST.

The implementing partner university for the Phil-LiDAR 1 Program is the University of the Philippines Los Baños (UPLB). UPLB is in charge of processing LiDAR data and conducting data validation reconnaissance, cross section, bathymetric survey, validation, river flow measurements, flood height and extent data gathering, flood modeling, and flood map generation for the 45 river basins in the Southern Luzon region. The university is located in Los Baños in the province of Laguna.

## 1.2 Overview of the Inagauan River Basin

The Inagauan River Basin is a 16,885-ha watershed covering a small portion of the City of Puerto Princesa in the province of Palawan. It encompasses barangay Aporawan, and Sagpangan in the municipality of Aborlan; Inagawan, Napsan, Inagawan Sub-Colony in Puerto Princesa City. The DENR River Basin Control Office (RBCO) states that the Inagauan River Basin has a drainage are of 171 km² and an estimated 274 cubic meter (MCM) annual run-off (RBCO, 2015).

The Inagauan River Basin's main stem, the Inagauan River, is part of the forty-five (45) river systems under the PHIL-LIDAR 1 Program partner State University and College (SUC), University of the Philippines Los Baños. The Inagauan River Basin passes through Napsan, Inagawan Sub-colony, and Inagawan. According to the 2015 national census of Philippine Statistics Authority (PSA), a total of 4,052 persons are residing in Brgy. Inagawan Sub-Colony in Puerto Princesa City, which is within the immediate vicinity of the river. The economic activities of the people in the Inagauan River Basin of the province of Palawan is primarily agriculture-based, with fishing, tourism, trade, commerce, and mineral extraction as the major sources of income (Palawan Knowledge Platform for Biodiversity and Sustainable Development, 2007).

Within a tropical region, Climate Type I and III prevails in the Inagauan River Basin based on the Modified Corona Classification of climate. Type I has two pronounced seasons, dry from November to April, and wet the rest of the year with maximum rain period from June to September. On the other hand, Type III has no very pronounced maximum rain period and with short dry season lasting only from one to three months, during the period from December to February or from March to May. The same climate type prevails in the Mindoro, Marinduque, Romblon, and Palawan (MIMAROPA) region, where the Inagauan River Basin is located.

In terms of geology, the basin area is classified as predominatly Undifferentiated (Sedimentary & Metamorphic Rocks) and Cretaceous-Paleogene. Other areas area classified as Basement Complex (prejurassic), Pliocene-pleistocene and Recent. Generally, the slope in the area can be classified as gently sloping to steep with elevation ranging from 50 to more than 300 meters above sea level (masl). The soil in the large extent of rough mountainous land is still unclassified. However, other areas have Taburos clay and Aborlan loam. Dense vegetation of closed forest (broadleaved) dominates the basin area along with other wooded land (shrubs), other wooded land (wooded grassland), and other land (cultivated perennial).

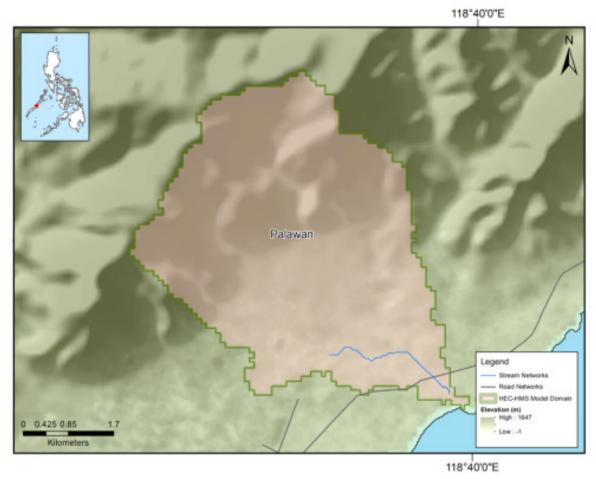


Figure 1. Map of Inagauan River Basin (in brown)

Based on the study conducted by the Mines and Geosciences Bureau, generally the barangays in the basin area have no susceptibility to flooding except for the portion of barangay Inagauan that has moderate to high susceptibility. The field surveys conducted by the PHIL-LiDAR 1 validation team showed that there were several notable weather disturbances that caused flooding in 2000 (Seniang), 2004 (Rolly), 2005 (Quedan), 2008 (Frank), 2013 (Yolanda), 2015 (Nona and Lando) and 2016 (Lawin). Meanwhile, due to the continuous rains brought about by the tail-end of a cold front, flooding incidents occurred in the Municipality of Narra and Puerto Princesa City on January 2011 as per NDRRMC report (National Disaster Risk Reduction and Management Council, 2011). For landslide risk susceptibility, barangay Napsan and Aporawan has moderate to high while Inagawan Sub-Colony has moderate susceptibility.

# CHAPTER 2: LIDAR DATA ACQUISITION IN INAGAUAN FLOODPLAIN

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The methods applied in this Chapter were based on the DREAM methods manual (Sarmiento, et al., 2014) and further enhanced and updated in Paringit, et al. (2017).

# 2.1 Flight Plans

Plans were made to acquire LiDAR data within the delineated priority area for Inagauan floodplain in Palawan. These missions were planned for 12 lines and ran for at most four and a half (4.5) hours including take-off, landing and turning time. The flight planning parameters for the LiDAR system is found in Table 1. Figure 2 shows the flight plan for Inagauan floodplain.

Table 1. Flight planning parameters for Aquarius LiDAR system

Block Name	Flying Height (AGL)	Overlap (%)	Field of View	Pulse Repetition Frequency (PRF) (kHz)	Scan Frequency	Average Speed	Average Turn Time (Minutes)	
BLK42A	800	30	50	200	30	130	5	
BLK42E	800	30	50	200	30	130	5	
BLK42F	800	30	50	200	30	130	5	

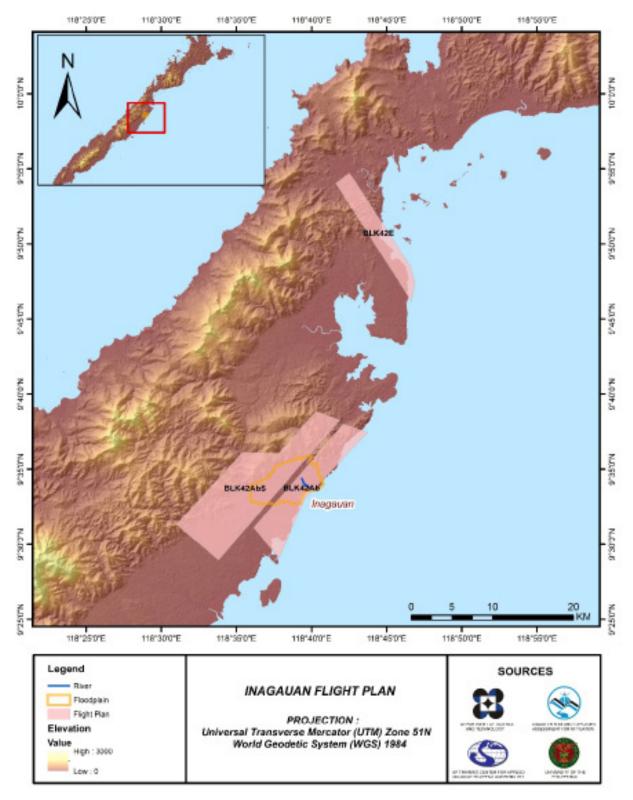


Figure 2. Flight plans used for Inagauan Floodplain

#### 2.2 Ground Base Station

The project team was able to recover one (1) NAMRIA ground control point (GCP), PLW-50 which is of second (2nd) order accuracy, and one (1) NAMRIA benchmark, PL-92. The certifications for the NAMRIA GCP and benchmark are found in Annex 2, while the baseline processing report for the established GCP is found in Annex 3. These were used as base stations during flight operations for the entire duration of the survey (June 14 and 17, 2015). Base stations were observed using dual frequency GPS receivers, TRIMBLE SPS 882 and SPS 852. Flight plans and location of base stations used during the aerial LiDAR acquisition in Inagauan floodplain are shown in Figure 2. The list of team members are shown in Annex 4.

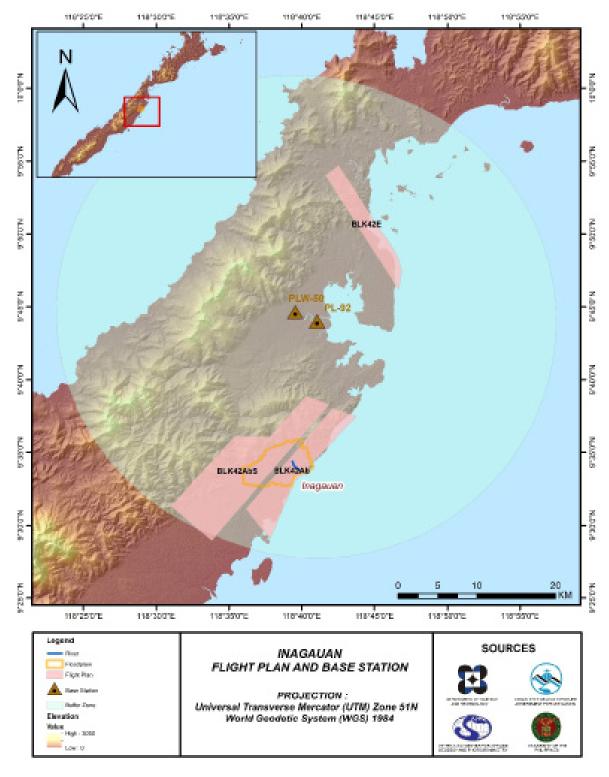


Figure 3. Flight plans and base stations for Inagauan Floodplain.

Figure 4 shows the recovered NAMRIA reference points within the area. In addition, Table 2 and Table 3 show the details about the following NAMRIA control stations. Table 4 shows the list of all ground control points occupied during the acquisition together with the corresponding dates of utilization.

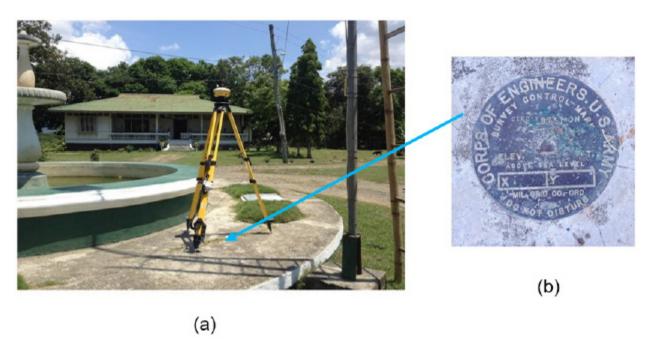


Figure 4. GPS set-up over PLW-50 within the vicinity of Ihawig Prison and Penal Farm at the fountain in front of Quarter 1, Brgy. Iwahig Puerto Princesa City (a) and NAMRIA reference point PLW-50 (b) as recovered by the field team.

Table 2. Details of the recovered NAMRIA horizontal control point PLW-50 used as base station for the LiDAR acquisition.

Station Name	PLW-50			
Order of Accuracy	2nd			
Relative Error (horizontal positioning)	1 in 50,000			
Geographic Coordinates, Philippine Reference of 1992 Datum (PRS 92)	Latitude Longitude Ellipsoidal Height	9° 44′ 42.16318″ North 118° 39′ 28.02050″ East 16.81300 meters		
Grid Coordinates, Philippine Transverse Mercator Zone 1A (PTM Zone 1A PRS 92)	Easting Northing	517311.956 meters 1077537.527 meters		
Geographic Coordinates, World Geodetic System 1984 Datum (WGS 84)	Latitude Longitude Ellipsoidal Height	9° 44′ 37.72390″ North 118° 39′ 33.34598″ East 66.85300 meters		
Grid Coordinates, Universal Transverse Mercator Zone 51 North (UTM 51N PRD 1992)	Easting Northing	681,851.72 meters 1,077,601.73 meters		

Table 3. Details of the recovered NAMRIA vertical control point PL-92 used as base station for the LiDAR acquisition with re-processed coordinates.

Station Name	PL-92			
Order of Accuracy	2nd			
Relative Error (horizontal positioning)	1 in	50,000		
Geographic Coordinates, Philippine Reference of 1992 Datum (PRS 92)	Latitude Longitude Ellipsoidal Height	9° 44' 04.01581" North 118° 40' 58.28065" East 8.218 meters		
Grid Coordinates, Philippine Transverse Mercator Zone 1A (PTM Zone 1A PRS 92)	Easting Northing	26049.752 meters 1079008.192 meters		
Geographic Coordinates, World Geodetic System 1984 Datum (WGS 84)	Latitude Longitude Ellipsoidal Height	99° 43′ 59.58138″ North 118° 41′ 3.60701″ East 58.344 meters		

Table 4. Ground control points used during LiDAR data acquisition

Date Surveyed	Flight Number	Mission Name	Ground Control Points
June 14, 2015 3049P		1BLK42S165A	PL-92, PLW-50
June 17, 2015	3061P	1BLK42Ab168A	PL-92, PLW-50

# 2.3 Flight Missions

Two (2) missions were conducted to complete LiDAR data acquisition in Inagauan floodplain, for a total of six hours and four minutes (6+04) of flying time for RP-C9022. All missions were acquired using the Pegasus LiDAR systems. Table 5 shows the total area of actual coverage and the corresponding flying hours per mission, while Table 6 presents the actual parameters used during the LiDAR data acquisition.

Table 5. Flight missions for LiDAR data acquisition in Inagauan Floodplain

				Area	Area		Flying	Hours
Date Surveyed	Flight Number	Flight Plan Area (km2)	Surveyed Area (km2)	Surveyed within the Floodplain (km2)	Surveyed Outside the Floodplain (km2)	No. of Images (Frames)	Hr	Min
June 14, 2015	3049P	128.25	149.83	20.14	128.98	501	2	50
June 17, 2015	3061P	208.16	173.22	18.67	153.69	603	3	14
TOTA	۱L	236.41	323.05	38.82	282.67	1104	6	4

Table 6. Actual parameters used during LiDAR data acquisition.

Flight Number	Flying Height (AGL)	Overlap (%)	Field of View (θ)	Pulse Repetition Frequency (PRF) (kHz)	Scan Frequency (Hz)	Average Speed (kts)	Average Turn Time (Minutes)
3049P	800	30	50	200	30	130	5
3061P	800	30	50	200	30	130	5

# 2.4 Survey Coverage

Inagauan floodplain is located in the province of Palawan. The list of municipalities and cities, with at least one (1) square kilometer coverage, is shown in Table 7. The actual coverage of the LiDAR acquisition for Inagauan floodplain is shown in Figure 5.

Table 7. List of municipalities and cities surveyed during Inagauan Floodplain LiDAR survey.

Province	Municipality/City	Area of Municipality/City (km2)	Total Area Surveyed (km2)	Percentage of Area Surveyed
Palawan	Aborlan	645.11	77.72	12%
	Puerto Princesa City	2186.36	206.72	9%
Total		2831.47	284.44	10.05%

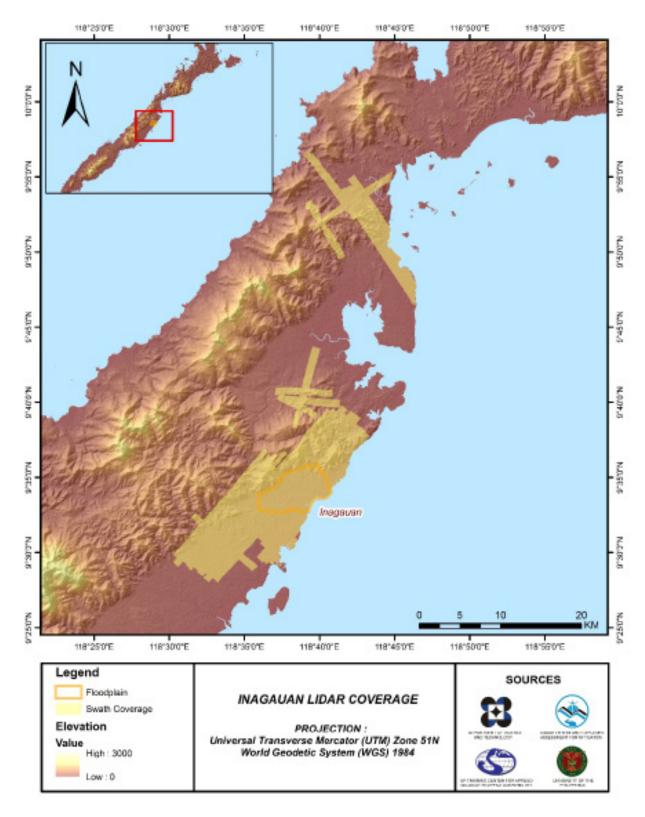


Figure 5. Actual LiDAR data acquisition for Inagauan floodplain.

# CHAPTER 3: LIDAR DATA PROCESSING FOR INAGAUAN FLOODPLAIN

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The methods applied in this Chapter were based on the DREAM methods manual (Ang, et al., 2014) and further enhanced and updated in Paringit, et al. (2017).

# 3.1 Overview of LiDAR Data Pre-Processing

The data transmitted by the Data Acquisition Component are checked for completeness based on the list of raw files required to proceed with the pre-processing of the LiDAR data. Upon acceptance of the LiDAR field data, georeferencing of the flight trajectory is done in order to obtain the exact location of the LiDAR sensor when the laser was shot. Point cloud georectification is performed to incorporate correct position and orientation for each point acquired. The georectified LiDAR point clouds are subject for quality checking to ensure that the required accuracies of the program, which are the minimum point density, vertical and horizontal accuracies, are met. The point clouds are then classified into various classes before generating Digital Elevation Models such as Digital Terrain Model and Digital Surface Model.

Using the elevation of points gathered in the field, the LiDAR-derived digital models are calibrated. Portions of the river that are barely penetrated by the LiDAR system are replaced by the actual river geometry measured from the field by the Data Validation and Bathymetry Component. LiDAR acquired temporally are then mosaicked to completely cover the target river systems in the Philippines. Orthorectification of images acquired simultaneously with the LiDAR data is done through the help of the georectified point clouds and the metadata containing the time the image was captured.

These processes are summarized in the flowchart shown in Figure 6.

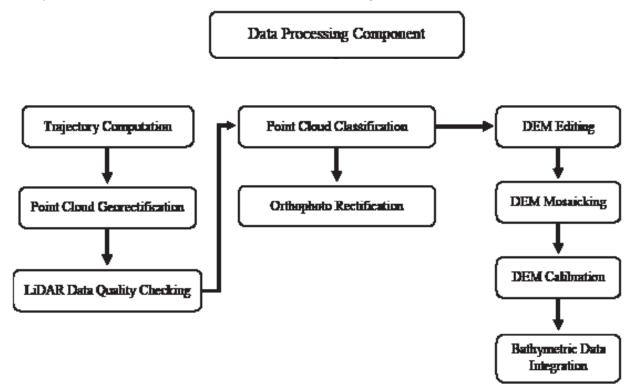


Figure 6. Schematic Diagram for Data Pre-Processing Component

## 3.2 Transmittal of Acquired LiDAR Data

Data transfer sheets for all the LiDAR missions for Inagauan floodplain can be found in Annex 5. Missions flown during the first survey conducted on June 2015 used the Airborne LiDAR Terrain Mapper (ALTM™ Optech Inc.) Pegasus system while missions acquired during the second survey on July 2015 were flown using the Aquarius system over Puerto Princesa, Palawan.

The Data Acquisition Component (DAC) transferred a total of 42.70 Gigabytes of Range data, 367 Megabytes of POS data, 21.33 Megabytes of GPS base station data, and 70.9 Gigabytes of raw image data to the data server on July 13, 2015 for the first survey and August 3, 2015 for the second survey. The Data Preprocessing Component (DPPC) verified the completeness of the transferred data. The whole dataset for Babuyan was fully transferred on August 5, 2015, as indicated on the Data Transfer Sheets for Inagauan floodplain.

## 3.3 Trajectory Computation

The Smoothed Performance Metrics of the computed trajectory for flight 3061P, one of the Inagauan flights, which is the North, East, and Down position RMSE values are shown in Figure 7. The x-axis corresponds to the time of flight, which is measured by the number of seconds from the midnight of the start of the GPS week, which on that week fell on June 17, 2015 00:00AM. The y-axis is the RMSE value for that particular position.

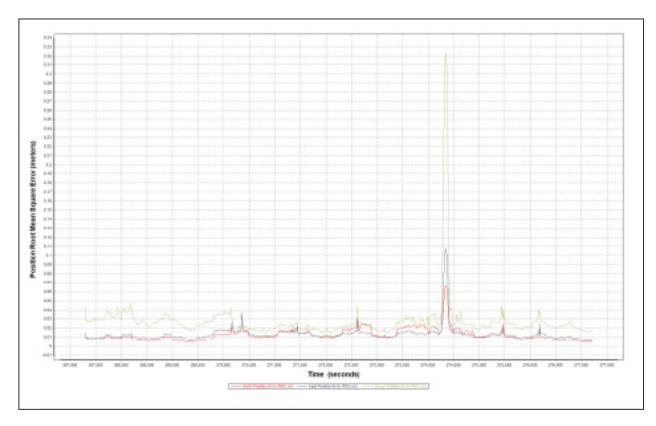


Figure 7. Smoothed Performance Metrics of Inagauan Flight 3061P.

The time of flight was from 267000 seconds to 277500 seconds, which corresponds to morning of June 17, 2015. The initial spike that is seen on the data corresponds to the time that the aircraft was getting into position to start the acquisition, and the POS system starts computing for the position and orientation of the aircraft. Redundant measurements from the POS system quickly minimized the RMSE value of the positions. The periodic increase in RMSE values from an otherwise smoothly curving RMSE values correspond to the turn-around period of the aircraft, when the aircraft makes a turn to start a new flight line. Figure 8 shows that the North position RMSE peaks at 1.10 centimeters, the East position RMSE peaks at 0.70 centimeters, and the Down position RMSE peaks at 3.20 centimeters, which are within the prescribed accuracies described in the methodology.

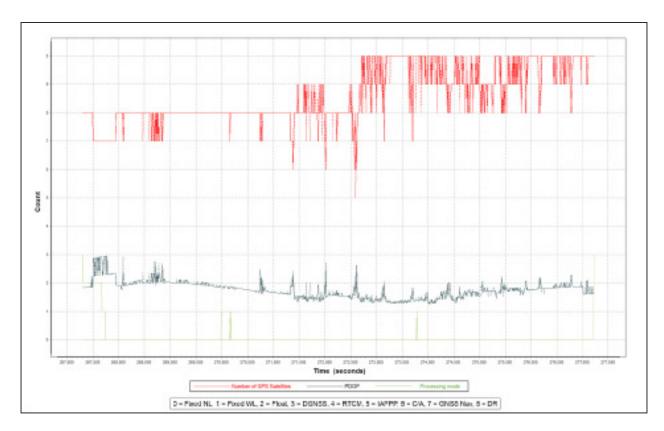


Figure 8. Solution Status Parameters of Inagauan Flight 3061P.

The Solution Status parameters of flight 3061P, one of the Inagauan flights, which are the number of GPS satellites, Positional Dilution of Precision (PDOP), and the GPS processing mode used, are shown in Figure 8. The graphs indicate that the number of satellites during the acquisition did not go down to 6. Majority of the time, the number of satellites tracked was between 6 and 10. The PDOP value also did not go above the value of 3, which indicates optimal GPS geometry. The processing mode stayed at the value of 0 for majority of the survey with some peaks up to 1 attributed to the turns performed by the aircraft. The value of 0 corresponds to a Fixed, Narrow-Lane mode, which is the optimum carrier-cycle integer ambiguity resolution technique available for POSPAC MMS. All of the parameters adhered to the accuracy requirements for optimal trajectory solutions, as indicated in the methodology. The computed best estimated trajectory for all Inagauan flights is shown in Figure 9.

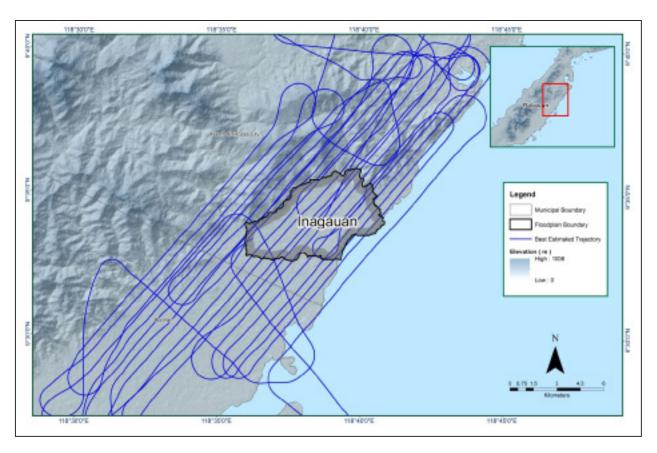


Figure 9. Best Estimated Trajectory for Inagauan Floodplain.

# 3.4 LiDAR Point Cloud Computation

The produced LAS data contains 60 flight lines, with each flight line containing two channels, since the Pegasus system contains two one channels. The summary of the self-calibration results obtained from LiDAR processing in LiDAR Mapping Suite (LMS) software for all flights over Babuyan floodplain are given in Table 8.

Parameter	Acceptable Value	Value	
Boresight Correction stdev	(<0.001degrees)	0.000288	
IMU Attitude Correction Roll and Pitch Corrections stdev	(<0.001degrees)	0.0021	
GPS Position 7-correction stdev	(<0.01meters)	0.0024	

Table 8. Self-Calibration Results values for Inagauan flights.

The optimum accuracy is obtained for all Inagauan flights based on the computed standard deviations of the corrections of the orientation parameters. Standard deviation values for individual blocks are available in Annex 8: Mission Summary Reports.

# 3.5 LiDAR Data Quality Checking

The boundary of the processed LiDAR data on top of a SAR Elevation Data over Inagauan Floodplain is shown in Figure 10. The map shows gaps in the LiDAR coverage that are attributed to cloud coverage.

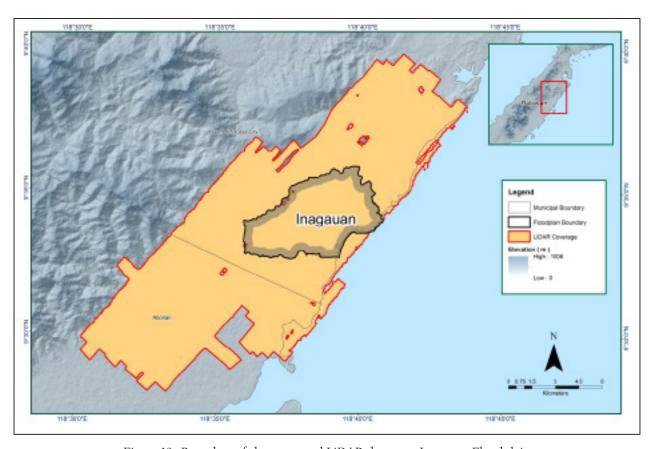


Figure 10. Boundary of the processed LiDAR data over Inagauan Floodplain

The total area covered by the Inagauan missions is 227.70 sq.km that is comprised of two (2) flight acquisitions grouped and merged into two (2) blocks as shown in Table 9.

Table 9. List of LiDAR blocks for Inagauan Floodplain.

LiDAR Blocks	Flight Numbers	Area (sq. km)	
Palawan_Blk42AB	3061P	148.98	
Palawan_Blk42AB_supplement	3049P	78.72	
TOTAL		227.70 sq.km	

The overlap data for the merged LiDAR blocks, showing the number of channels that pass through a particular location is shown in Figure 11. Since the Pegasus system employs two channels, we would expect an average value of 2 (blue) for areas where there is limited overlap, and a value of 3 (yellow) or more (red) for areas with three or more overlapping flight lines.

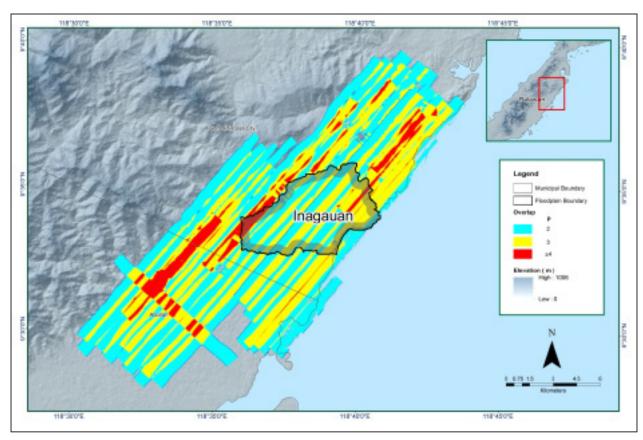


Figure 11. Image of data overlap for Inagauan floodplain.

The overlap statistics per block for the Inagauan floodplain can be found in Annex 8. One pixel corresponds to 25.0 square meters on the ground. For this area, the minimum and maximum percent overlaps are 47.37% and 56.02% respectively, which passed the 25% requirement.

The pulse density map for the merged LiDAR data, with the red parts showing the portions of the data that satisfy the 2 points per square meter criterion is shown in Figure 12. It was determined that all LiDAR data for Inagauan floodplain satisfy the point density requirement, and the average density for the entire survey area is 4.79 points per square meter.

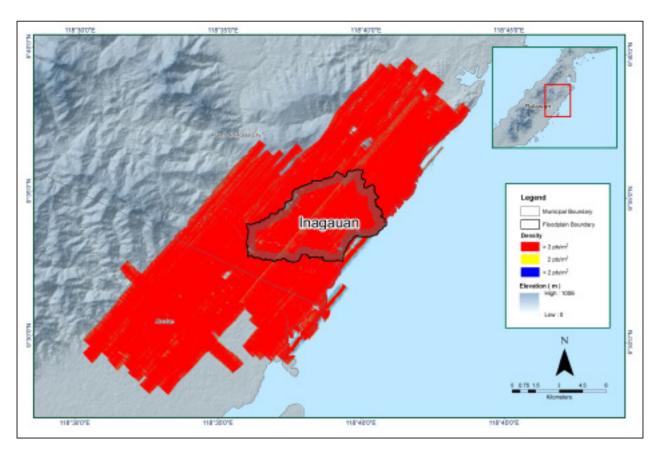


Figure 12. Pulse density map of merged LiDAR data for Inagauan Floodplain.

The elevation difference between overlaps of adjacent flight lines is shown in Figure 13. The default color range is from blue to red, where bright blue areas correspond to portions where elevations of a previous flight line, identified by its acquisition time, are higher by more than 0.20m relative to elevations of its adjacent flight line. Bright red areas indicate portions where elevations of a previous flight line are lower by more than 0.20m relative to elevations of its adjacent flight line. Areas with bright red or bright blue need to be investigated further using Quick Terrain Modeler software.

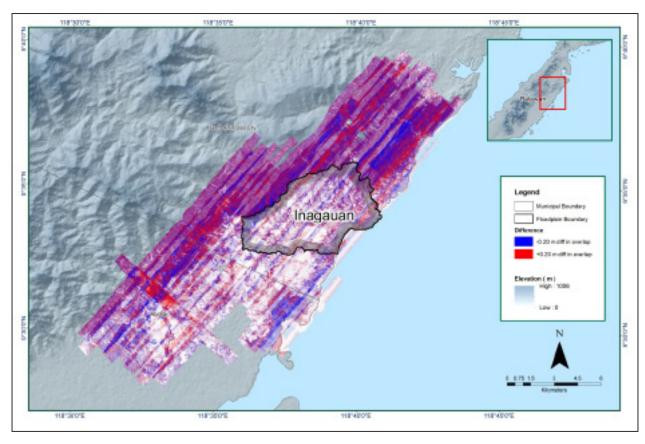


Figure 13. Elevation difference map between flight lines for Inagauan Floodplain.

A screen capture of the processed LAS data from a Babuyan flight 3061P loaded in QT Modeler is shown in Figure 14. The upper left image shows the elevations of the points from two overlapping flight strips traversed by the profile, illustrated by a dashed red line. The x-axis corresponds to the length of the profile. It is evident that there are differences in elevation, but the differences do not exceed the 20-centimeter mark. This profiling was repeated until the quality of the LiDAR data becomes satisfactory. No reprocessing was done for this LiDAR dataset.

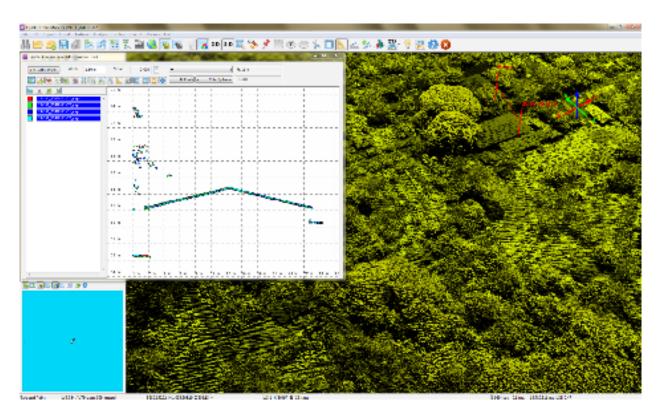


Figure 14. Quality checking for Inagauan flight 3061P using the Profile Tool of QT Modeler.

#### 3.6 LiDAR Point Cloud Classification and Rasterization

Table 10. Inagauan classification results in TerraScan.
---

Pertinent Class	Total Number of Points		
Ground	203,865,061		
Low Vegetation	163,178,413		
Medium Vegetation	332,054,303		
High Vegetation	1,244,112,706		
Building	10,395,438		

The tile system that TerraScan employed for the LiDAR data and the final classification image for a block in Inagauan floodplain is shown in Figure 15. A total of 309 1km by 1km tiles were produced. The number of points classified to the pertinent categories is illustrated in Table 10. The point cloud has a maximum and minimum height of 695 meters and 42.29 meters respectively.

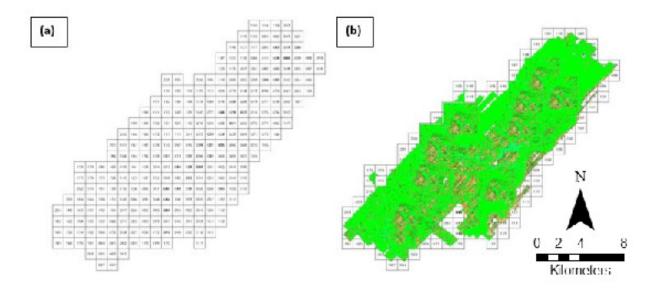


Figure 15. Tiles for Inagauan Floodplain (a) and classification results (b) in TerraScan.

An isometric view of an area before and after running the classification routines is shown in Figure 16. The ground points are in orange, the vegetation is in different shades of green, and the buildings are in cyan. It can be seen that residential structures adjacent or even below canopy are classified correctly, due to the density of the LiDAR data.

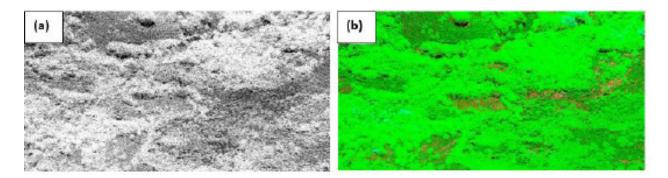


Figure 16. Point cloud before (a) and after (b) classification.

The production of last return (V\_ASCII) and the secondary (T\_ASCII) DTM, first (S\_ASCII) and last (D\_ASCII) return DSM of the area in top view display are shown in Figure 17. It shows that DTMs are the representation of the bare earth while on the DSMs, all features are present such as buildings and vegetation.

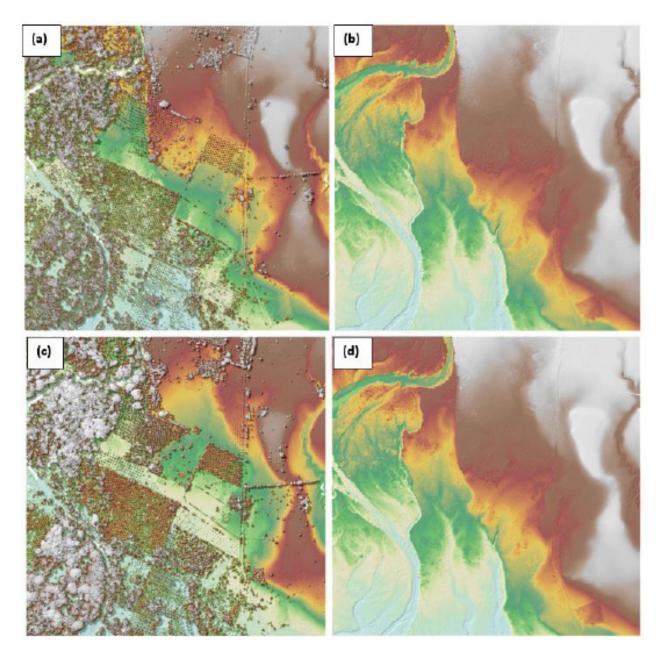
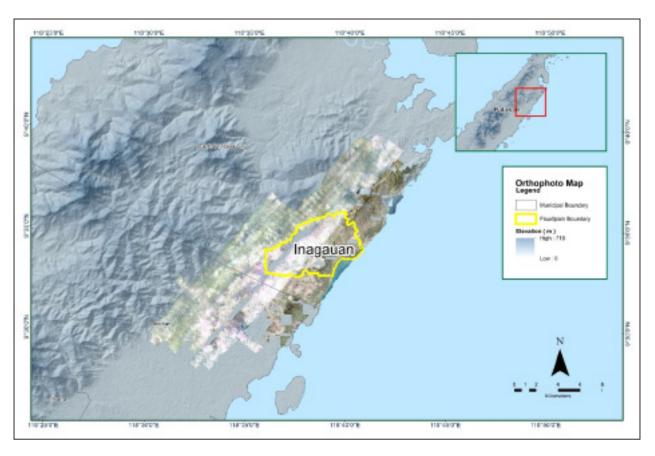


Figure 17. The production of last return DSM (a) and DTM (b), first return DSM (c) and secondary DTM (d) in some portion of Inagauan Floodplain.

# 3.7 LiDAR Image Processing and Orthophotograph Rectification

The 311 1km by 1km tiles area covered by Inagauan floodplain is shown in Figure 18. After tie point selection to fix photo misalignments, color points were added to smoothen out visual inconsistencies along the seamlines where photos overlap. The Inagauan floodplain has a total of 202.97 sq.km orthophotogaph coverage comprised of 1,014 images. A zoomed in version of sample orthophotographs named in reference to its tile number is shown in Figure 19.



 $Figure\ 18.\ Inagauan\ Floodplain\ with\ available\ orthophotographs.$ 

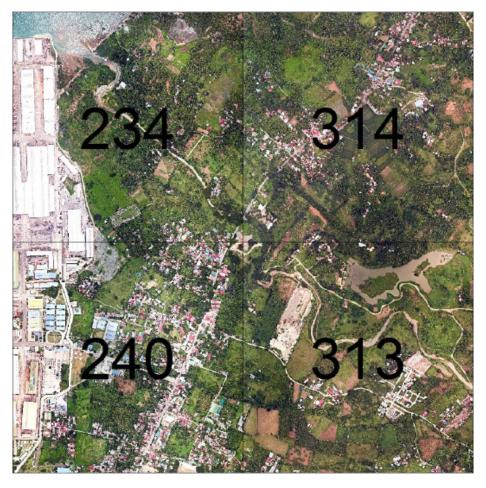


Figure 19. Sample orthophotograph tiles for Inagauan Floodplain.

# 3.8 DEM Editing and Hydro-Correction

Two (2) mission blocks were processed for Inagauan floodplain. These blocks are composed of Palawan blocks with a total area of 227.70 square kilometers. Table 11 shows the name and corresponding area of each block in square kilometers.

LiDAR Blocks	Area (sq.km)		
Palawan_Blk42AB	148.98		
Palawan_Blk42AB_supplement	78.72		
TOTAL	227.70 sq.km		

Table 11. LiDAR blocks with its corresponding area.

Portions of DTM before and after manual editing are shown in Figure 20. The bridge in Figure 20a would be an impedance to the flow of water along the river and was removed in order to hydrologically correct the river, as done in Figure 20b. Another portion of the DTM presented in Figure 20c shows the part of the river which have data gaps. This part was filled as done in Figure 20d.

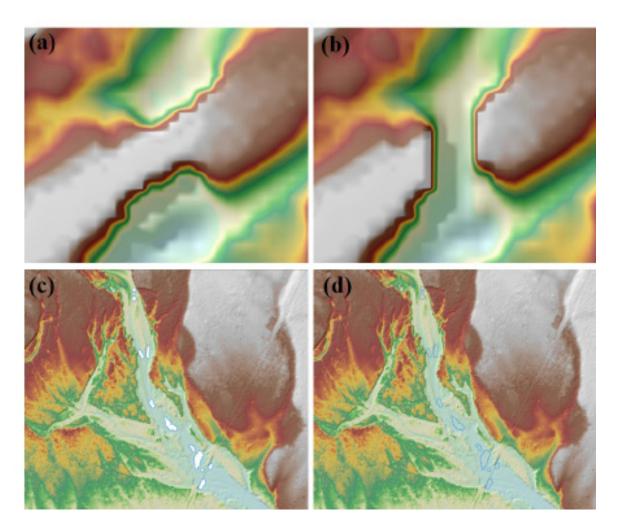


Figure 20. Figure 20. Portions in the DTM of Inagauan Floodplain – a bridge before (a) and and after (b) manual data editing and a part of the river with no data before (c) and after (d) filling data gaps.

# 3.9 Mosaicking of Blocks

Palawan Block 42AB was used as the reference block at the start of mosaicking because it was the first block mosaicked to the larger DTM of West Coast Palawan. Upon inspection of the blocks mosaicked for the Inagauan floodplain, it was concluded that there is no need to adjust the elevation of the DTM for all of the blocks merged, as presented in Table 12.

Mosaicked LiDAR DTM for Inagauan floodplain is shown in Figure 21. It can be seen that the entire Inagauan floodplain is 99.70% covered by LiDAR data.

Table 12. Shift Values of each LiDAR Block of Inagauan Floodplain

Mission Blocks	Shift Values (meters)			
WIISSION DIOCKS	х	У	z	
Palawan_Blk42AB	0.00	0.00	0.00	
Palawan_Blk42AB_supplement	0.00	0.00	0.00	

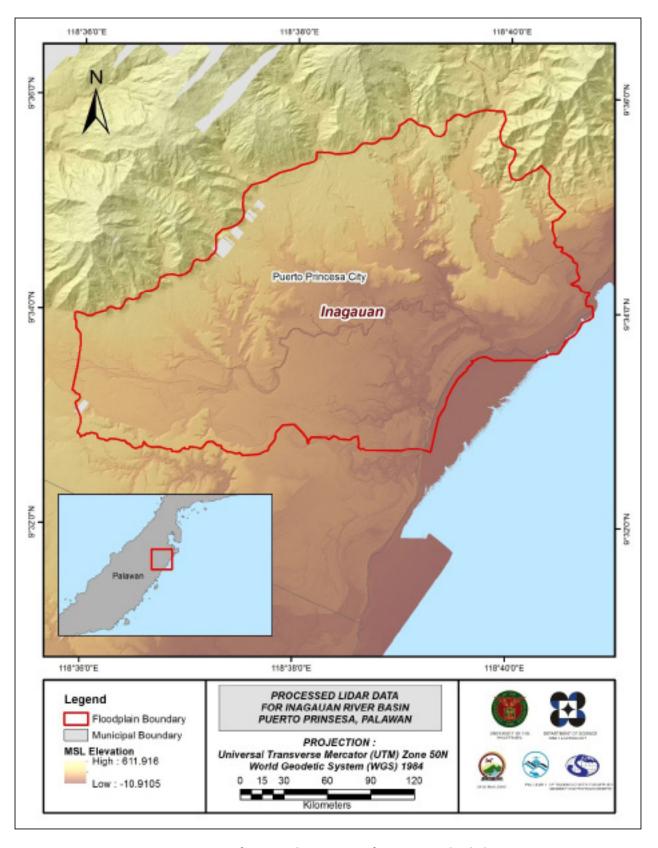


Figure 21. Map of Processed LiDAR Data for Inagauan Flood Plain.

## 3.10 Calibration and Validation of Mosaicked LiDAR Digital Elevation Model

The extent of the validation survey done by the Data Validation and Bathymetry Component (DVBC) in Inagauan to collect points with which the LiDAR dataset is validated is shown in Figure 22. A total of 3,498 survey points were used for calibration and validation of Inagauan LiDAR data. Random selection of 80% of the survey points, resulting to 2,794 points, were used for calibration.

A good correlation between the uncalibrated mosaicked LiDAR elevation values and the ground survey elevation values is shown in Figure 23. Statistical values were computed from extracted LiDAR values using the selected points to assess the quality of data and obtain the value for vertical adjustment. The computed height difference between the LiDAR DTM and calibration elevation values is 9.61 meters with a standard deviation of 0.15 meters. Calibration of Inagauan LiDAR data was done by adding the height difference value, 9.61 meters, to Inagauan mosaicked LiDAR data. Table 13 shows the statistical values of the compared elevation values between LiDAR data and calibration data.

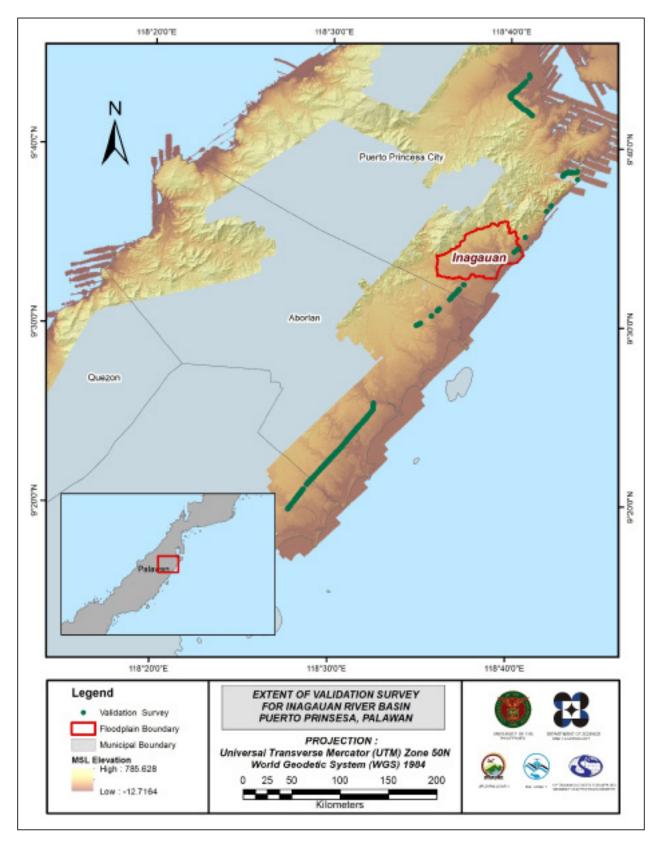


Figure 22. Map of Inagauan Floodplain with validation survey points in green.

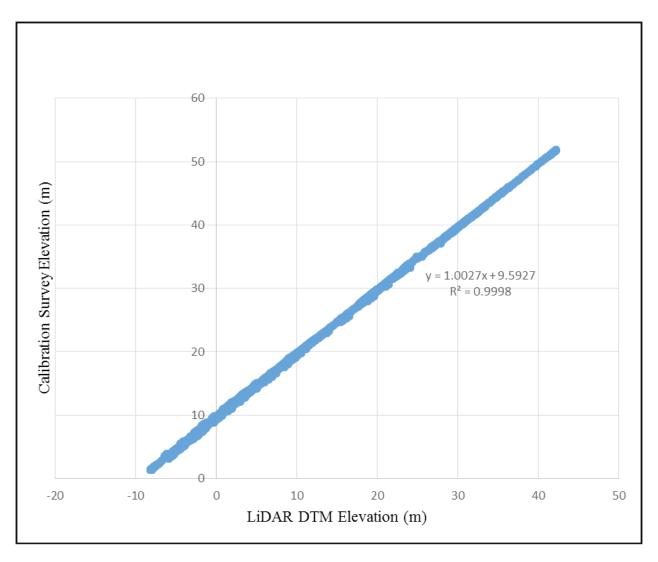


Figure 23. Correlation plot between calibration survey points and LiDAR data.

Table 13. Calibration Statistical Measures.

Calibration Statistical Measures	Value (meters)
Height Difference	9.61
Standard Deviation	0.15
Average	9.61
Minimum	9.30
Maximum	9.92

The remaining 20% of the total survey points, resulting to 701 points, were used for the validation of calibrated Inagauan DTM. A good correlation between the calibrated mosaicked LiDAR elevation values and the ground survey elevation, which reflects the quality of the LiDAR DTM is shown in Figure 24. The computed RMSE between the calibrated LiDAR DTM and validation elevation values is 0.15 meters with a standard deviation of 0.15 meters, as shown in Table 14.

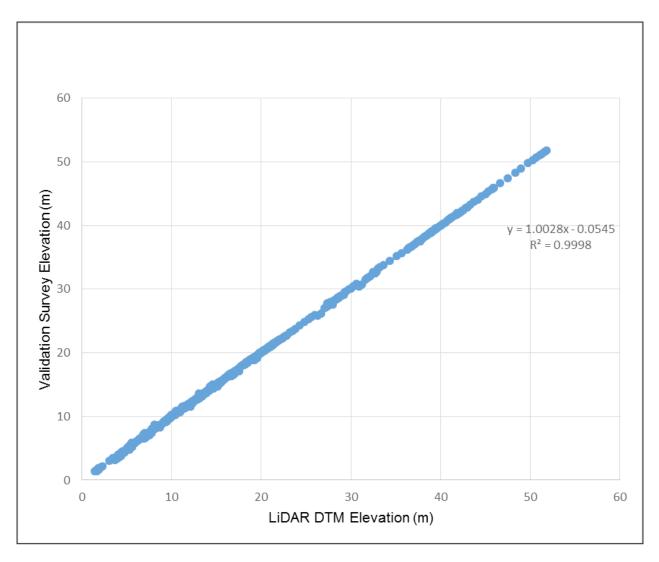


Figure 24. Correlation plot between validation survey points and LiDAR data.

Table 14. Validation Statistical Measures.

Validation Statistical Measures	Value (meters)
RMSE	0.15
Standard Deviation	0.15
Average	-0.005
Minimum	-0.31
Maximum	0.30

## 3.11 Integration of Bathymetric Data into the LiDAR Digital Terrain Model

For bathy integration, centerline and zigzag data were available for Inagauan with 2,487 and 417 bathymetric survey points, respectively. The resulting raster surface produced was done by Kernel Interpolation with Barriers method. After burning the bathymetric data to the calibrated DTM, assessment of the interpolated surface is represented by the computed RMSE value of 0.44 meters. The extent of the bathymetric survey done by the Data Validation and Bathymetry Component (DVBC) in Inagauan integrated with the processed LiDAR DEM is shown in Figure 25.

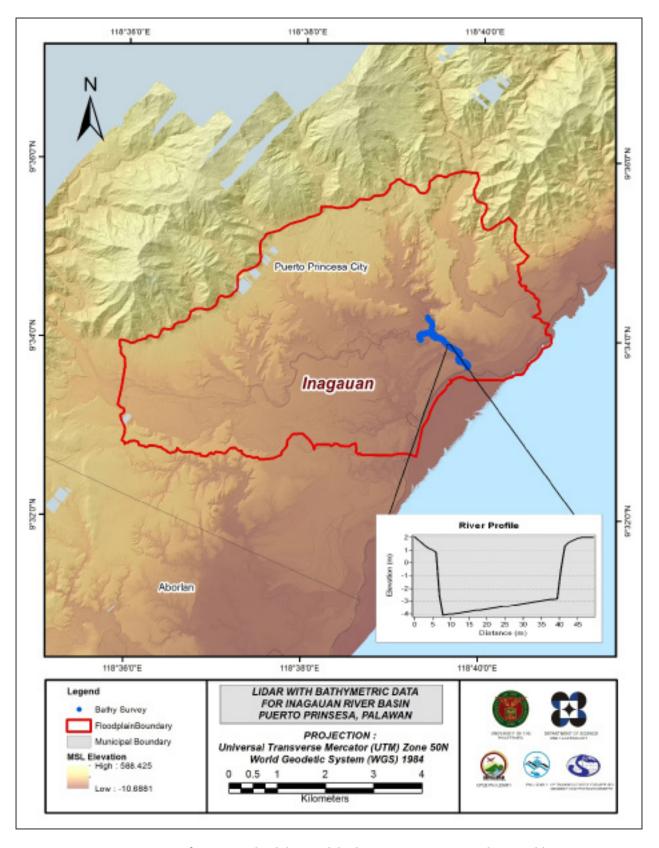


Figure 25. Map of Inagauan Floodplain with bathymetric survey points shown in blue.

#### 3.12 Feature Extraction

The features salient in flood hazard exposure analysis include buildings, road networks, bridges and water bodies within the floodplain area with 200 m buffer zone. Mosaicked LiDAR DEM with 1 m resolution was used to delineate footprints of building features, which consist of residential buildings, government offices, medical facilities, religious institutions, and commercial establishments, among others. Road networks comprise of main thoroughfares such as highways and municipal and barangay roads essential for routing of disaster response efforts. These features are represented by a network of road centerlines.

## 3.12.1 Quality Checking (QC) of Digitized Features' Boundary

Inagauan floodplain, including its 200 m buffer, has a total area of 39.33 sq km. For this area, a total of 5.0 sq km, corresponding to a total of 539 building features, are considered for QC. Figure 26 shows the QC blocks for Inagauan floodplain.

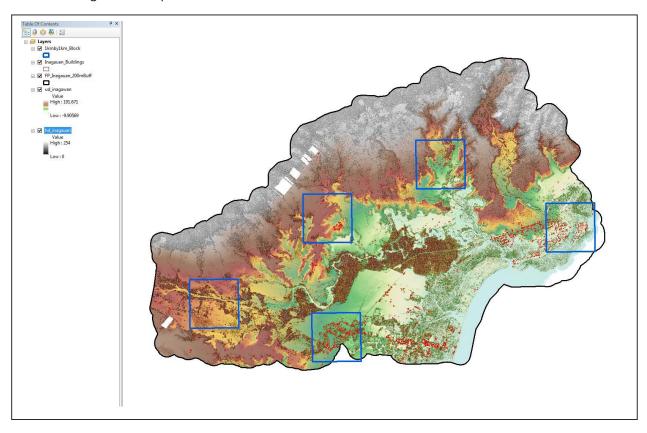


Figure 26. QC blocks for Inagauan building features.

Quality checking of Inagauan building features resulted in the ratings shown in Table 15.

Table 15. Quality Checking Ratings for Inagauan Building Features.

FLOODPLAIN	COMPLETENESS	CORRECTNESS	QUALITY	REMARKS
Inagauan	87.62	99.81	82.19	PASSED

#### 3.12.2 Height Extraction

Height extraction was done for 5,690 building features in Inagauan floodplain. Of these building features, none was filtered out after height extraction, resulting to 5,690 buildings with height attributes. The lowest building height is at 2.00 m, while the highest building is at 8.74 m.

#### 3.12.3 Feature Attribution

The digitized features were marked and coded in the field using handheld GPS receivers. The attributes of non-residential buildings were first identified; all other buildings were then coded as residential. An nDSM was generated using the LiDAR DEMs to extract the heights of the buildings. A minimum height of 2 meters was used to filter out the terrain features that were digitized as buildings. Buildings that were not yet constructed during the time of LiDAR acquisition were noted as new buildings in the attribute table.

Table 16 summarizes the number of building features per type. On the other hand, Table 17 shows the total length of each road type, while Table 18 shows the number of water features extracted per type.

Table 16. Building Features Extracted for Inagauan Floodplain.

Facility Type	No. of Features
Residential	1470
School	26
Market	0
Agricultural/Agro-Industrial Facilities	1
Medical Institutions	0
Barangay Hall	1
Military Institution	0
Sports Center/Gymnasium/Covered Court	2
Telecommunication Facilities	0
Transport Terminal	0
Warehouse	2
Power Plant/Substation	0
NGO/CSO Offices	0
Police Station	8
Water Supply/Sewerage	0
Religious Institutions	16
Bank	0
Factory	0
Gas Station	0
Fire Station	0
Other Government Offices	2
Other Commercial Establishments	51
Total	1579

Table 17. Total Length of Extracted Roads for Inagauan Floodplain.

	Road Network Length (km)					
Floodplain	Barangay City/Municipal Road Road		Provincial Road	National Road	Others	Total
Inagauan	6.9	0.00	0.00	7.01	0.00	13.91

Table 18. Number of Extracted Water Bodies for Inagauan Floodplain.

	Water Body Type					
Floodplain	Rivers/Streams	Lakes/Ponds	Sea	Dam	Fish Pen	Total
Inagauan	3	5	0	0	0	8

A total of 6 bridges and culverts over small channels that are part of the river network were also extracted for the floodplain.

# 3.12.4 Final Quality Checking of Extracted Features

All extracted ground features were completely given the required attributes. All these output features comprise the flood hazard exposure database for the floodplain. This completes the feature extraction phase of the project.

Figure 27 shows the Digital Surface Model (DSM) of Inagauan floodplain overlaid with its ground features.

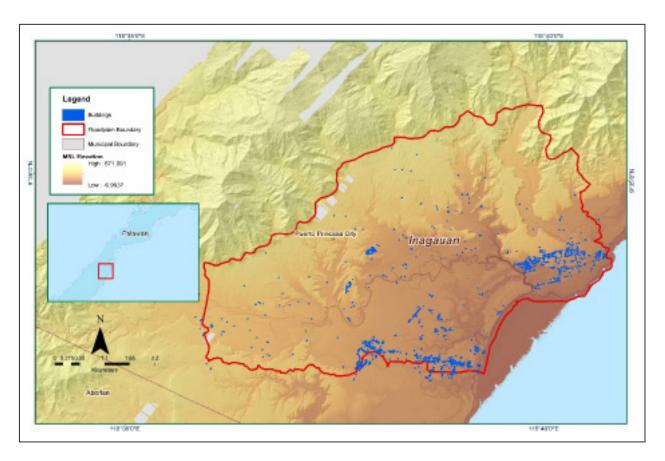


Figure 27. Extracted features for Inagauan Floodplain.

# CHAPTER 4: DATA VALIDATION SURVEY AND MEASUREMENTS IN THE INAGAUAN RIVER BASIN

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The methods applied in this Chapter were based on the DREAM methods manual (Balicanta, et al., 2014) and further enhanced and updated in Paringit, et al. (2017).

#### 4.1 Summary of Activities

AB Surveying and Development (ABSD) conducted a field survey in Inagauan River on November 27 and 28, 2015, and February 6, 2016 with the following scope: reconnaissance; control survey; and cross-section and as-built survey at Inagauan Bridge in Brgy. Inagawan Sub-Colony, Puerto Princesa City, Palawan. Random checking points for the contractor's cross-section and bathymetry data were gathered by DVBC on August 16-28, 2016 using an Ohmex™ Single Beam Echo Sounder and Trimble® SPS 985 GNSS PPK survey technique. In addition to this, validation points acquisition survey was conducted covering the Inagauan River Basin area. The entire survey extent is illustrated in Figure 28.

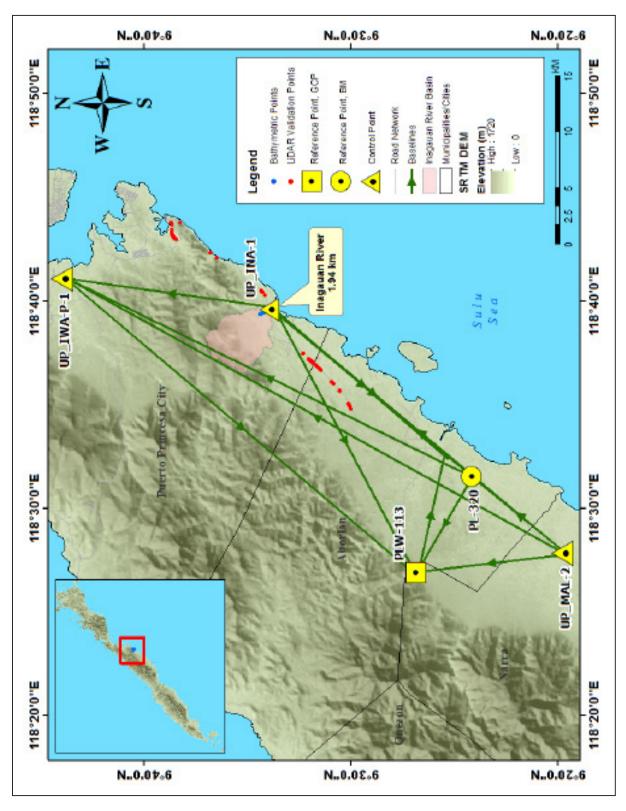


Figure 28. Inagauan River Survey Extent

## 4.2 Control Survey

The GNSS network used for Inagauan River is composed of nine (9) loops established on August 25, 2016 occupying the following reference points: PLW-113 a second-order GCP, in Brgy. Dumagueña, Narra, Palawan and PL-320, a first-order BM, in Brgy. Ramon Magsaysay, Aborlan, Palawan.

Four (4) control points established in the area by ABSD were also occupied: UP\_MAL-2 at the approach of Malatgao Bridge in Brgy. Tinagong Dagat, Narra, Province of Palawan, UP\_IWA-P-1 at the approach of Iwahig Penal Bridge in Brgy. Iwahig, Puerto Princesa City, Palawan, UP\_ABO-1 located beside the approach of Aborlan Bridge in Brgy. Gogognan, Aborlan, Palawan, and UP\_INA-1 located beside the approach of Inagauan Bridge in Brgy. Inagauan Sub-Colony, Puerto Princesa City, Palawan.

The summary of reference and control points and its location is summarized in Table 19 while GNSS network established is illustrated in Figure 29.

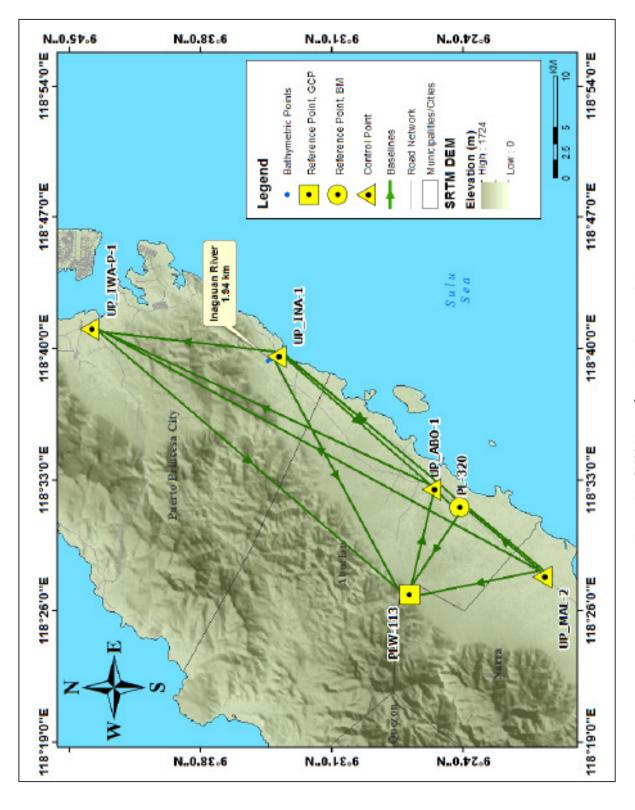


Figure 29. GNSS Network covering Inagauan River

Table 19. List of reference and control points used during the survey in Inagauan River (Source: NAMRIA, UP-TCAGP)

			Geographic Coordinates (WGS 84)						
Control Point	Order of Accuracy	Latitude	Longitude	Ellipsoidal Height (Meter)	Elevation in MSL (Meter)	Date Established			
PLW-113	2nd order, GCP	9° 26' 50.78624" N	118° 26' 52.23491"E	144.388	93.784	2007			
PL-320	1st order, BM	9° 24' 10.67926" N	118° 31' 31.30061"E	58.025	7.089	2008			
UP_MAL-2	Established	9° 19' 47.08536"N	118° 27' 48.23703"E	67.449	16.469	11-27-15			
UP_IWA-P-1	Established	9° 43' 58.38961"N	118° 41' 03.58218"E	55.529	5.044	11-25-15			
UP_ABO-1	Established	9° 25' 39.66712"N	118° 32' 29.34660"E	59.322	8.415	11-26-15			
UP_INA-1	Established	9° 33' 58.62160"N	118° 39' 34.84567"E	56.382	5.672	11-27-15			

The GNSS set-ups on recovered reference points and established control points in Inagauan River are shown from Figure 30 to Figure 35.

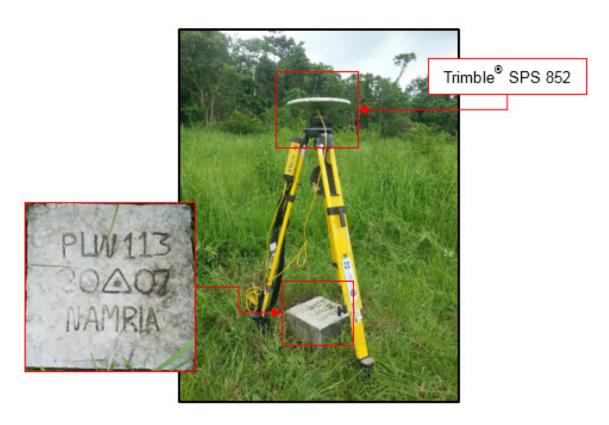


Figure 30. GNSS base set up, Trimble® SPS 852, at PLW-113, located southwest of the Aborlan Water System in Brgy. Dumagueña, Narra, Province of Palawan

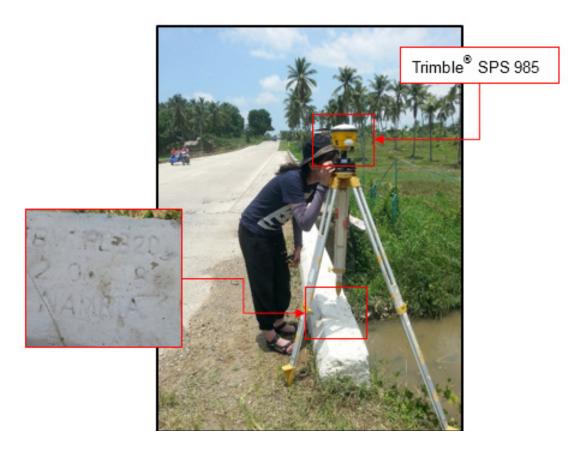


Figure 31. GNSS receiver set up, Trimble® SPS 985, at PL-320, located on top of a culvert headwall along the National Road in Brgy. Ramon Magsaysay, Aborlan, Province of Palawan

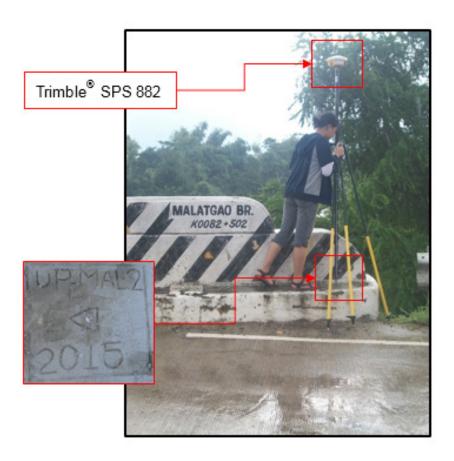


Figure 32. GNSS receiver set up, Trimble® SPS 882, at UP\_MAL-2, located at the approach of Malatgao Bridge in Brgy. Tinagong Dagat, Narra, Province of Palawan

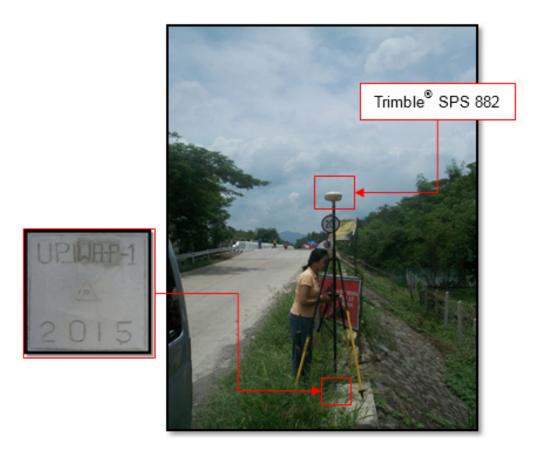


Figure 33. GNSS receiver set up, Trimble® SPS 982, at UP\_IWA-P-1, located at the approach of Iwahig Penal Bridge in Brgy. Iwahig, Puerto Princesa City, Palawan



Figure 34. GNSS receiver set-up, Trimble® SPS 852, at UP\_ABO-1, an established control point, beside the approach of Aborlan Bridge in Brgy. Gogognan, Aborlan, Palawan



Figure 35. GNSS receiver set up, Trimble® SPS 882, at UP\_INA-1, located beside the approach of Inagauan Bridge in Brgy. Inagauan Sub-Colony, Puerto Princesa City, Palawan

## 4.3 Baseline Processing

GNSS Baselines were processed simultaneously in TBC by observing that all baselines have fixed solutions with horizontal and vertical precisions within +/- 20 cm and +/- 10 cm requirement, respectively. In case where one or more baselines did not meet all of these criteria, masking is performed. Masking is done by removing/masking portions of these baseline data using the same processing software. It is repeatedly processed until all baseline requirements are met. If the reiteration yields out of the required accuracy, resurvey is initiated. Baseline processing result of control points in Inagauan River Basin is summarized in Table 20 generated by TBC software.

Table 20. Baseline Processing Report for Inagauan River Static Survey (Source: NAMRIA, UP-TCAGP)

Observation	Date of Observation	Solution Type	H. Prec. (Meter)	V. Prec. (Meter)	Geodetic Az.	Ellipsoid Dist. (Meter)	ΔHeight (Meter)
PLW-113 UP_ ABO-1	8-25-2016	Fixed	0.009	0.023	101°59'15"	10513.593	-85.092
UP_IWA-P-1 PLW-113	8-25-2016	Fixed	0.004	0.024	219°26'55"	40874.066	88.833
PL-320 PLW- 113	8-25-2016	Fixed	0.018	0.029	300°01'31"	9832.467	86.391
PL-320 UP_ IWA-P-1	8-25-2016	Fixed	0.004	0.018	205°34'21"	40449.118	2.530
UP_MAL-2 PL- 320	8-25-2016	Fixed	0.010	0.021	220°02'59"	10578.751	9.435
UP_INA-1 UP_ ABO-1	8-25-2016	Fixed	0.008	0.025	220°15'41"	20085.570	2.974
UP_INA-1 PLW- 113	8-25-2016	Fixed	0.005	0.025	240°32'45"	26716.978	88.012
UP_INA-1 PL- 320	8-25-2016	Fixed	0.010	0.019	219°14'35"	23320.185	1.618
UP_INA-1 UP_ IWA-P-1	8-25-2016	Fixed	0.005	0.019	188°21'15"	18624.653	0.847
UP_MAL-2 UP_INA-1	8-25-2016	Fixed	0.005	0.014	39°28'10"	33898.188	-11.058
UP_MAL-2 UP_ IWA-P-1	8-25-2016	Fixed	0.024	0.024	208°33'52"	50759.890	11.894
UP_MAL-2 PLW-113	8-25-2016	Fixed	0.005	0.021	352°31'24"	13129.154	76.935

As shown Table 20 a total of twelve (12) baselines were processed with coordinate and elevation values of UP IWA-P-1 and the coordinate values of PLW-113 held fixed. All of them passed the required accuracy.

## 4.4 Network Adjustment

After the baseline processing procedure, network adjustment is performed using TBC. Looking at the Adjusted Grid Coordinates table of the TBC generated Network Adjustment Report, it is observed that the square root of the sum of the squares of x and y must be less than 20 cm and z less than 10 cm or in equation form:

$$\sqrt{((x_e)^2+(y_e)^2)}$$
 <20cm and  $z_e<10~cm$ 

Where:

xe is the Easting Error, ye is the Northing Error, and ze is the Elevation Error

for each control point. See the Network Adjustment Report shown in Table 21 to Table 24 for complete details.

The six (6) control points, PLW-113, PL-320, UP-MAL-2, UP-IWA-P-1, UP\_ABO-1, and UP\_INA-1 were occupied and observed simultaneously to form a GNSS loop. The coordinates and elevation of UP\_IWA-P-1 and the coordinates of PLW-113 were held fixed during the processing of the control points as presented in Table 21. Through these reference points, the coordinates and elevations of the unknown control points will be computed.

East σ North σ Height σ Elevation σ **Point ID** Type (Meter) (Meter) (Meter) (Meter) PLW-113 Global Fixed Fixed UP\_IWA-P-1 Grid Fixed UP IWA-P-1 Global Fixed Fixed

Table 21. Control Point Constraints

Fixed = 0.000001 (Meter)

The list of adjusted grid coordinates, i.e. Northing, Easting, Elevation and computed standard errors of the control points in the network is indicated in Table 22. All fixed control points have no values for grid and elevation errors.

Point ID	Easting	Easting Error (Meter)	Northing (Meter)	Northing Error (Meter)	Elevation (Meter)	Elevation Error (Meter)	Constraint
PL-320	667487.736	0.013	1039767.829	0.008	7.089	0.049	
PLW-113	658953.945	?	1044650.284	?	93.784	0.054	LL
UP_ABO-1	669246.540	0.018	1042509.427	0.016	8.415	0.080	
UP_INA-1	682153.657	0.009	1057898.445	0.007	5.672	0.047	
UP_IWA-P-1	684768.852	?	1076338.886	?	5.044	?	LLe
UP_MAL-2	660716.408	0.012	1031641.078	0.009	16.469	0.047	

Table 22. Adjusted Grid Coordinates

With the mentioned equation  $\sqrt{((x_e)^2 + (y_e)^2)} < 20cm$  for the horizontal and  $z_e < 10 \ cm$  for the vertical; the computation for the accuracy for:

a. PL-320

horizontal accuracy =  $V((1.3)^2 + (0.8)^2$ 

√ (1.69 + 0.64)

= 2.33 < 20 cm

vertical accuracy = 4.9 < 10 cm

b. PLW-113

horizontal accuracy = Fixed vertical accuracy = Fixed

c. UP ABO-1

horizontal accuracy =  $V((1.8)^2 + (1.6)^2$ 

√ (3.24 + 2.56) 5.8 < 20 cm

vertical accuracy = 8.0 < 10 cm

d. UP\_INA-1

horizontal accuracy =  $V((0.9)^2 + (0.7)^2$ 

=  $\sqrt{(0.81 + 0.49)}$ 

= 1.30 < 20 cm vertical accuracy = 4.7 < 10 cm

e. UP\_IWA-P-1

horizontal accuracy = Fixed vertical accuracy = Fixed

f. UP MAL-2

horizontal accuracy =  $V((1.2)^2 + (0.9)^2$ 

=  $\sqrt{(1.44 + 0.81)}$ 

= 2.25 < 20 cm

vertical accuracy = 4.7 < 10 cm

Following the given formula, the horizontal and vertical accuracy result of the four (4) occupied control points are within the required precision.

Table 23. Adjusted Geodetic Coordinates

Point ID	Latitude	Longitude	Height (Meter)	Height Error (Meter)	Constraint
PL-320	N9°24'10.67926"	E118°31'31.30061"	58.025	0.049	
PLW-113	N9°26'50.78624"	E118°26'52.23491"	144.388	0.054	LL
UP_ABO-1	N9°25'39.66712"	E118°32'29.34660"	59.322	0.080	
UP_INA-1	N9°33'58.62160"	E118°39'34.84567"	56.382	0.047	
UP_IWA-P-1	N9°43'58.38961"	E118°41'03.58218"	55.529	?	LLe
UP_MAL-2	N12°57'27.19115"	E120°53'33.54442"	88.823	0.091	LL

The corresponding geodetic coordinates of the observed points are within the required accuracy as shown in Table 23. Based on the result of the computation, the equation is satisfied; hence, the required accuracy for the program was met.

The summary of reference control points used is indicated in Table 24.

Table 24. Reference and control points used and its location (Source: NAMRIA, UP-TCAGP)

		Geographic	Coordinates (WO	GS 84)	UT	M ZONE 51 N	
Control Point	Order of Accuracy	Latitude	Longitude	Ellipsoidal Height (m)	Northing	Easting	MSL Elevation (m)
PLW-113	2nd order, GCP	9° 26' 50.78624" N	118° 26' 52.23491"E	144.388	1044650.284	658953.945	93.784
PL-320	1st order, BM	9° 24' 10.67926" N	118° 31' 31.30061"E	58.025	1039767.829	667487.736	7.089
UP_ MAL-2	Established	9° 19' 47.08536"N	118° 27' 48.23703"E	67.449	1031641.078	660716.408	16.469
UP_ IWA-P-1	Established	9° 43' 58.38961"N	118° 41' 03.58218"E	55.529	1076338.886	684768.852	5.044
UP_ ABO-1	Established	9° 25' 39.66712"N	118° 32' 29.34660"E	59.322	1042509.427	669246.54	8.415
UP_INA- 1	Established	9° 33' 58.62160"N	118° 39' 34.84567"E	56.382	1057898.445	682153.657	5.672

# 4.5 Cross-section and Bridge As-Built survey and Water Level Marking

Cross-section and as-built surveys were conducted on November 27, 2015 at the downstream side of Inagauan Bridge in Brgy. Inagawan Sub-Colony, Puerto Princesa City as shown in Figure 36. A total station was utilized for this survey as shown in Figure 37.



Figure 36. Inagauan Bridge facing upstream

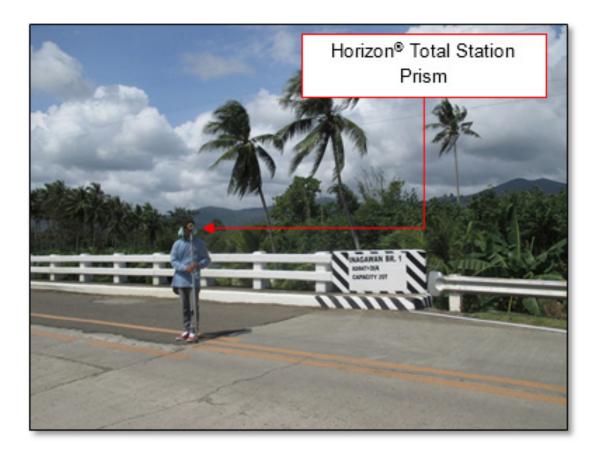


Figure 37. As-built survey of Inagauan Bridge

The cross-sectional line of Inagauan Bridge is about 227 m with sixty-two (62) cross-sectional points using the control points UP\_INA-1 and UP\_INA-2 as the GNSS base stations. The location map, cross-section diagram, and bridge data form are shown in Figure 38, Figure 39, and Figure 40.

Gathering of random points for the checking of ABSD's bridge cross-section and bridge points data was performed by DVBC on August 25, 2016 using a survey grade GNSS Rover receiver attached to a 2-m pole.

Linear square correlation (R2) and RMSE analysis were performed on the two (2) datasets. The linear square coefficient range is determined to ensure that the submitted data of the contractor is within the accuracy standard of the project which is  $\pm 20$  cm and  $\pm 10$  cm for horizontal and vertical, respectively. The R2 value must be within 0.85 to 1. An R2 approaching 1 signifies a strong correlation between the vertical (elevation values) of the two datasets. A computed R2 value of 0.997 was obtained by comparing the data of the contractor and DVBC; signifying a strong correlation between the two (2) datasets.

In addition to the Linear Square correlation, Root Mean Square (RMSE) analysis is also performed in order to assess the difference in elevation between the DVBC checking points and the contractor's. The RMSE value should only have a maximum radial distance of 5 m and the difference in elevation within the radius of 5 meters should not be beyond 0.50 m. For the bridge cross-section data, a computed value of 0.219 was acquired. The computed R2 and RMSE values are within the accuracy requirement of the program.

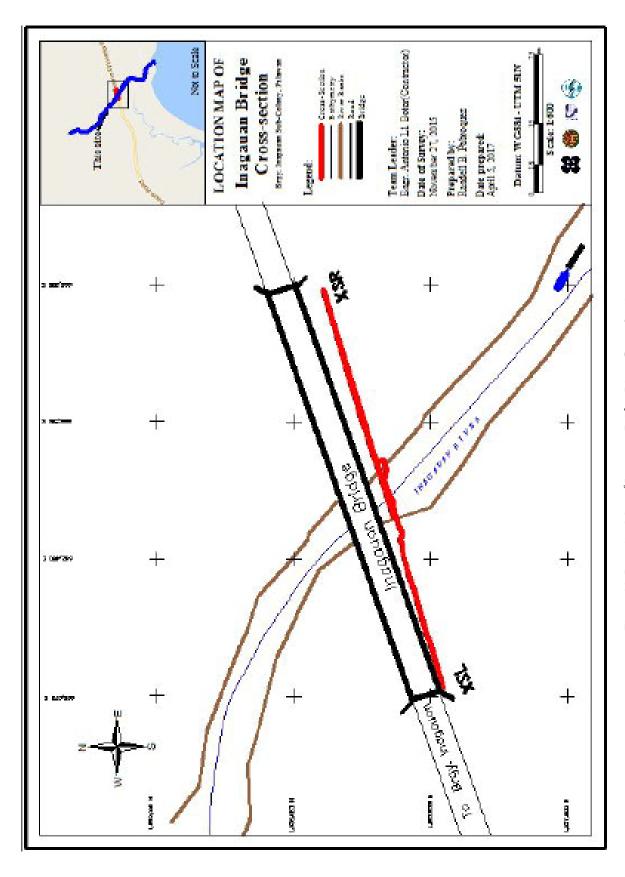


Figure 38. Location Map of Inagauan Bridge River Cross-Section survey

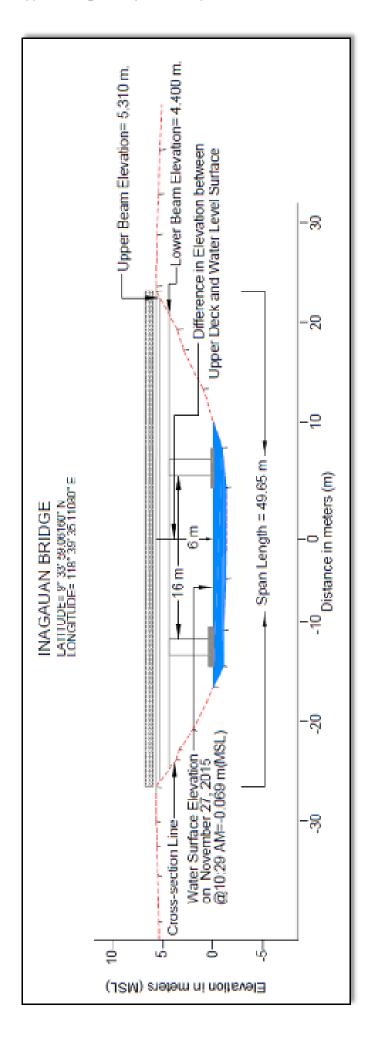
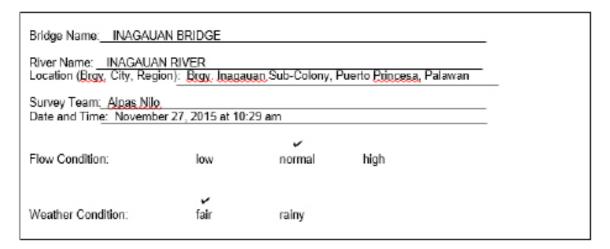
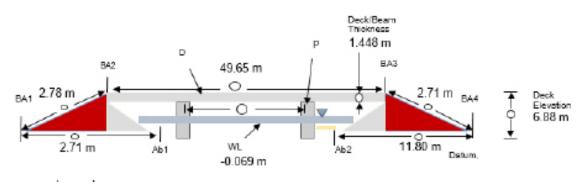


Figure 39. Inagauan Bridge cross-section diagram

#### Bridge Data Form



Cross-sectional View (not to scale)



Legend:
BA = Bridge Approach
P = Pier
Ab = Abutment
D = Deck
WL = Water Level/Surface
MSL = Mean Sea Level
= Measurement Value

Line Segment	Measurement (m)	Remarks
1. BA1-BA2	2.78 m	
2. BA2-BA3	49.65 m	
3. BA3-BA4	2.71 m	
4. BA1-Ab1	2.71 m	
5. Ab2-BA4	11.80 m	
<ol><li>Deck/beam thickness</li></ol>	1.448 m	
7. Deck elevation	6.88 m	

Note: Observer should be facing downstream

Figure 40. Bridge as-built form of Inagauan Bridge

Water surface elevation of Inagauan River was determined by a Horizon® Total Station on November 27, 2015 at 10:29 AM at Inagauan Bridge area with a value of -0.069 m in MSL as shown in Figure 39. This was translated into marking on the bridge's pier as shown in Figure 41. The marking will serve as reference for flow data gathering and depth gauge deployment of the partner HEI responsible for Inagauan River, the University of the Philippines Los Baños.



Figure 41. Water-level markings on Inagauan Bridge

# 4.6 Validation Points Acquisition Survey

Validation points acquisition survey was conducted by DVBC from August 16-28, 2016 using a survey grade GNSS Rover receiver, Trimble® SPS 985, mounted on a range pole which was attached on the side of the vehicle as shown in Figure 42.It was secured with cable ties and ropes to ensure that it was horizontally and vertically balanced. The antenna height was 2.590 m and measured from the ground up to the bottom of the quick release of the GNSS Rover receiver. The PPK technique utilized for the conduct of the survey was set to continuous topo mode with UP\_INA-1 occupied as the GNSS base station in the conduct of the survey.



Figure 42. Validation Points Acquisition Set-up for Inagauan River

The survey started from Brgy. Tagburos, City of Puerto Princesa, Palawan going southwest along the national highway, covering three (3) barangays in the City of Puerto Princesa and and ended in Brgy. Isaub, Municipality of Aborlan, Palawan. The survey gathered a total of 344 points with approximate length of 26.46 km using UP\_INA-1 as GNSS base station for the entire extent of validation points acquisition survey as illustrated in the map in Figure 43. More than 60% of the surveyed area for validation has no data due to heavy canopy and absence of GNSS receiver signal because of the mountainous terrain in the area.

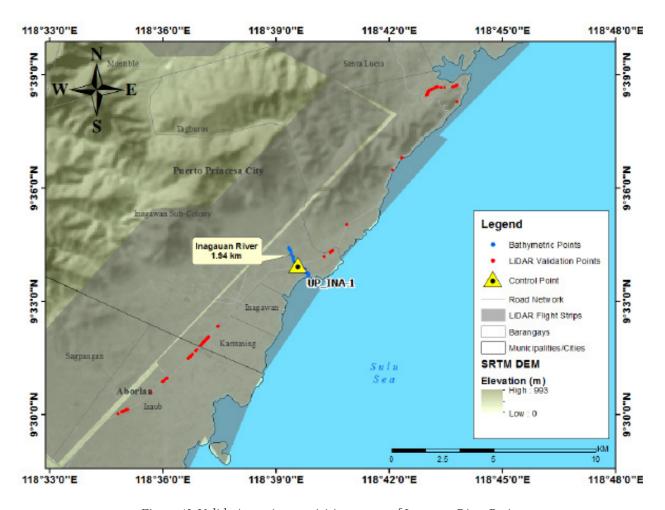


Figure 43. Validation point acquisition survey of Inagauan River Basin

## 4.7 Bathymetric Survey

Bathymetric survey was executed on November 28, 2016 using a single-beam echo sounder as illustrated in Figure 44. The survey started in Brgy. Inagawan Sub-Colony, Puerto Princesa City, Palawan with coordinates 9° 34′ 15.43106″N, 118° 39′ 24.05592″E and ended at the mouth of the river in Brgy. Inagawan Sub-Colony, Puerto Princesa City as well, with coordinates 9° 33′ 41.99750″N, 118° 39′ 51.07104″E. The control point UP INA-1 was used as GNSS base station all throughout the entire survey.

Gathering of random points for the checking of ABSD's bathymetric data was performed by DVBC on August 26, 2016 using a survey grade GNSS Rover receiver attached to a boat as seen in Figure 45. A map showing the DVBC bathymetric checking points is shown in Figure 47.

Linear square correlation (R2) and RMSE analysis were also performed on the two (2) datasets and a computed R2 values of 0.860 and 0.895 for the centerline and zigzag line bathymetric data, respectively, are within the required range for R2, which is 0.85 to 1. Additionally, an RMSE values of 0.304 and 0.314 for the centerline and zigzag line bathymetric data, respectively, were obtained. Both the computed R2 and RMSE values are within the accuracy required by the program.



Figure 44. Manual Bathymetric survey using a Trimble® SPS 985 in GNSS PPK survey technique in Caramay River

On the other hand, manual bathymetric survey was executed on November 9, 10, 11, and 14 2015 carrying a Trimble bag with installed Trimble® SPS 882 using GNSS PPK survey technique as shown in Figure 46. The survey started at the upstream part of the river in Brgy. Purnaga with coordinates 12°21′06.41749″ 121°11′13.79163″, traversed down by foot and ended at the starting point of bathymetric survey using boat. The control points GPS 4 was used as base station on November 9, 10, and 14 while MC-200 was used on November 13, 2015.



Figure 45. Gathering of random bathymetric points along Inagauan River

The bathymetric survey for Inagauan River gathered a total of 3,152 points covering 1.94 km of the river traversing Brgy. Inagawan Sub-Colony in Puerto Princesa City. Because of the narrow width of the river in the upstream, 0.34 km was not surveyed. Instead, 1.35 km was surveyed in the tributary of Inagauan River. (Figure 46).

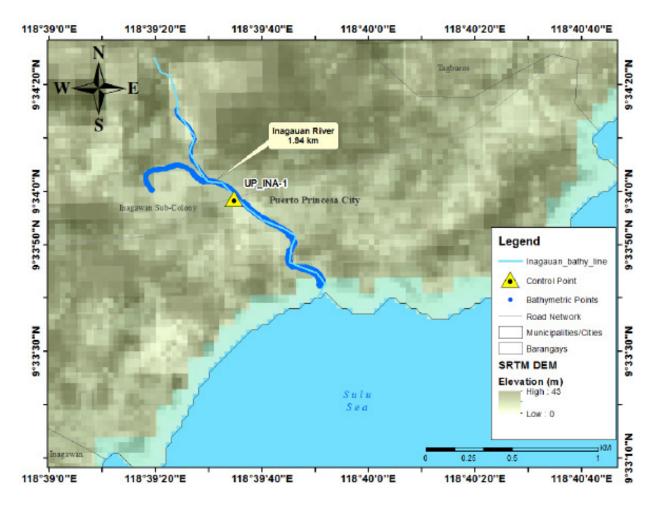


Figure 46. Bathymetric survey of Inagauan River

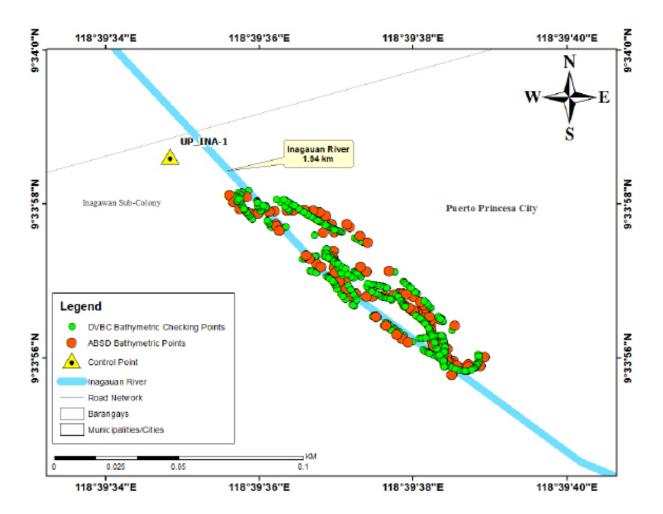


Figure 47. Inagauan centerline riverbed profile (Upstream)

A CAD drawing was also produced to illustrate the riverbed profile of Inagauan River. As shown in Figure 48, the highest and lowest elevation has a 4-m difference. The highest elevation observed was -1.483 m below MSL located in Brgy. Inagawan Sub-Colony, Puerto Princesa City while the lowest was -5.753 m below MSL located in Brgy. Inagawan Sub-Colony, Puerto Princesa City as well.

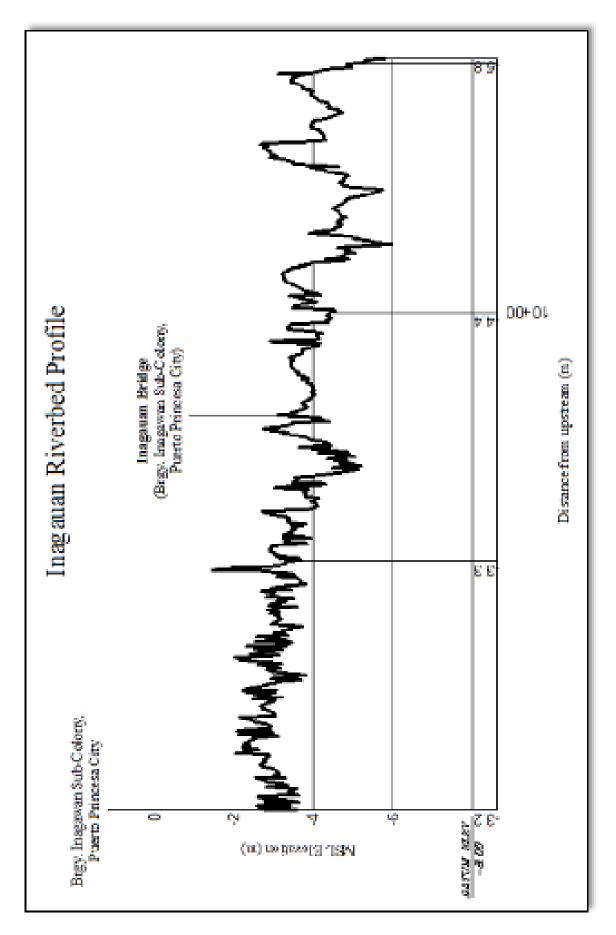


Figure 48. Inagauan centerline riverbed profile (Downstream)

#### **CHAPTER 5: FLOOD MODELING AND MAPPING**

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The methods applied in this Chapter were based on the DREAM methods manual (Lagmay, et al., 2014) and further enhanced and updated in Paringit, et al. (2017).

#### 5.1 Data used

No gathered rainfall data for Inagauan river basin. The HMS model is not calibrated. The values generated HMS model are by default.

#### 5.2 RIDF Station

The Philippines Atmospheric Geophysical and Astronomical Services Administration (PAGASA) computed Rainfall Intensity Duration Frequency (RIDF) values for the Romblon Rain Gauge. The RIDF rainfall amount for 24 hours was converted to a synthetic storm by interpolating and re-arranging the values in such a way a certain peak value will be attained at a certain time. This station chosen based on its proximity to the Inagauan watershed. The extreme values for this watershed were computed based on a 48-year record, with the computed extreme values shown in Table 28.

Table 25. RIDF values for Puerto Princesa Rain Gauge computed by PAGASA

COMPUTED EXTREME VALUES (in mm) OF PRECIPITATION									
T (yrs)	10 mins	20 mins	30 mins	1 hr	2 hrs	3 hrs	6 hrs	12 hrs	24 hrs
2	14.8	22	27.3	36.2	49.8	58.8	75.1	88	104.1
5	21.3	31.9	39.7	52.3	73	86.9	112.8	135.4	156.4
10	25.6	38.5	48	63	88.4	105.5	137.8	166.8	191.1
15	28.1	42.2	52.6	69	97	116	151.9	184.5	210.6
20	29.8	44.7	55.9	73.3	103.1	123.4	161.7	196.8	224.3
25	31.1	46.7	58.4	76.5	107.8	129.1	169.3	206.4	234.9
50	35.2	52.9	66.1	86.5	122.2	146.5	192.7	235.8	267.3
100	39.2	59	73.7	96.4	136.5	163.8	216	265	299.6

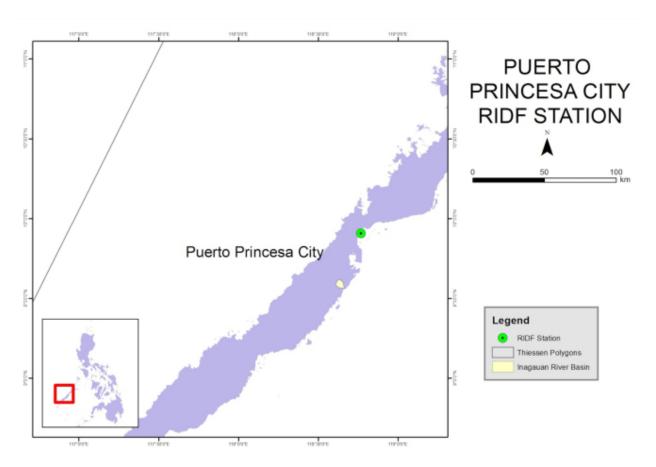


Figure 49. Location of Puerto Princesa RIDF relative to Inagauan River Basin

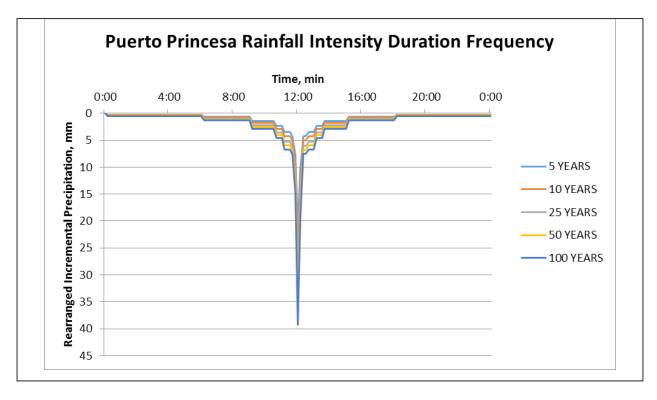


Figure 50. Synthetic storm generated for a 24-hr period rainfall for various return periods.

#### 5.3 HMS Model

The soil dataset was generated before 2004 by the Bureau of Soils and Water Management under the Department of Agriculture (DA-BSWM). The land cover dataset is from the National Mapping and Resource information Authority (NAMRIA). The soil and land cover of the Inagauan River Basin are shown in Figure 51 and Figure 52, respectively.

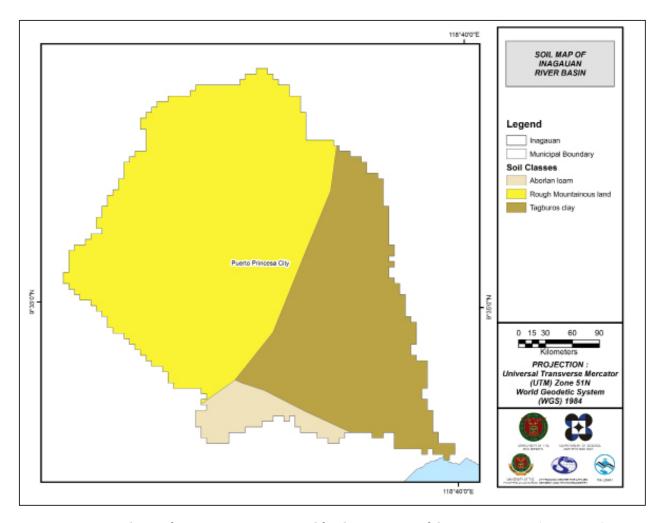


Figure 51. Soil map of Inagauan River Basin used for the estimation of the CN parameter. (Source: DA)

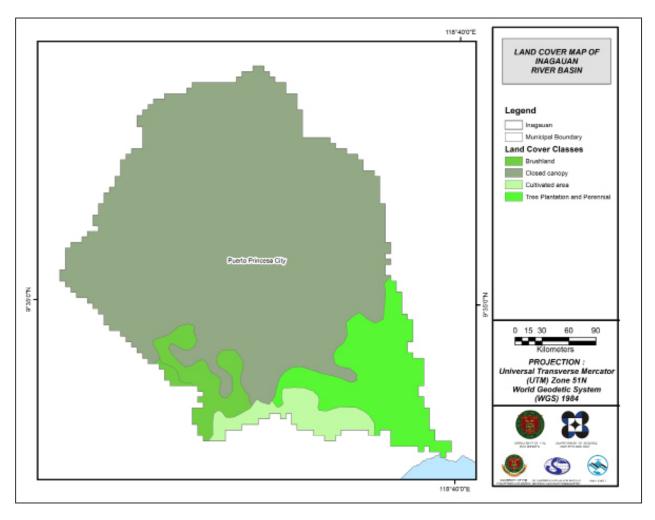


Figure 52. Land cover map of Caramay River Basin used for the estimation of the CN and watershed lag parameters of the rainfall-runoff model. (Source: NAMRIA)

For Inagauan river basin, three (3) soil classes were identified. The river basin area is largely rough mountainous land and Tagburos clay, with a smaller portion of Aborlan loam. Moreover, the four (4) land cover types identified were mostly closed canopy, followed by tree plantation and perennial, cultivated area, and brushland.

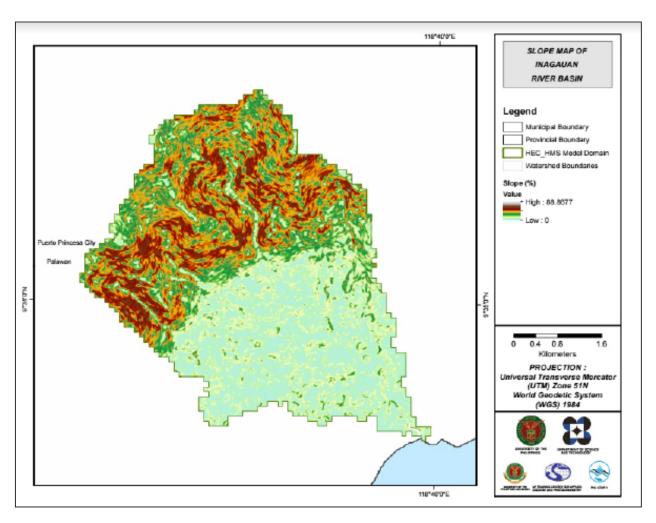


Figure 53. Slope map of Inagauan River Basin

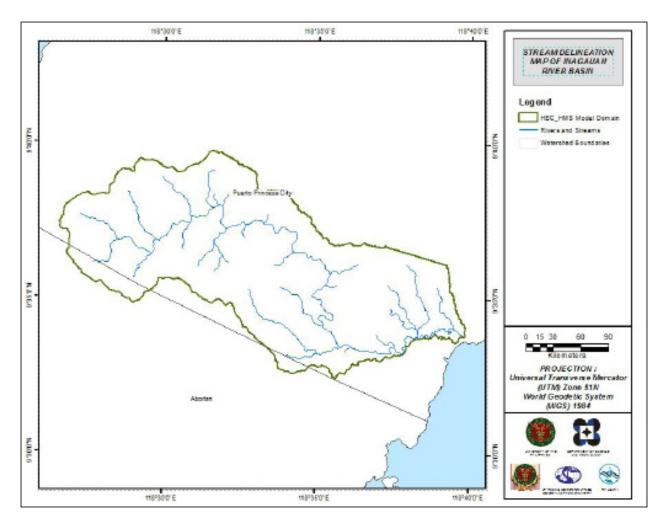


Figure 54. Stream delineation map of Inagauan River Basin

Using SAR-based DEM, the Inagauan basin was delineated and further subdivided into subbasins. The model consists of 54 sub basins, 26 reaches, and 25 junctions. The main outlet is labelled as Inagauan\_outlet. This basin model is illustrated in Figure 55. The basins were identified based on soil and land cover characteristics of the area.

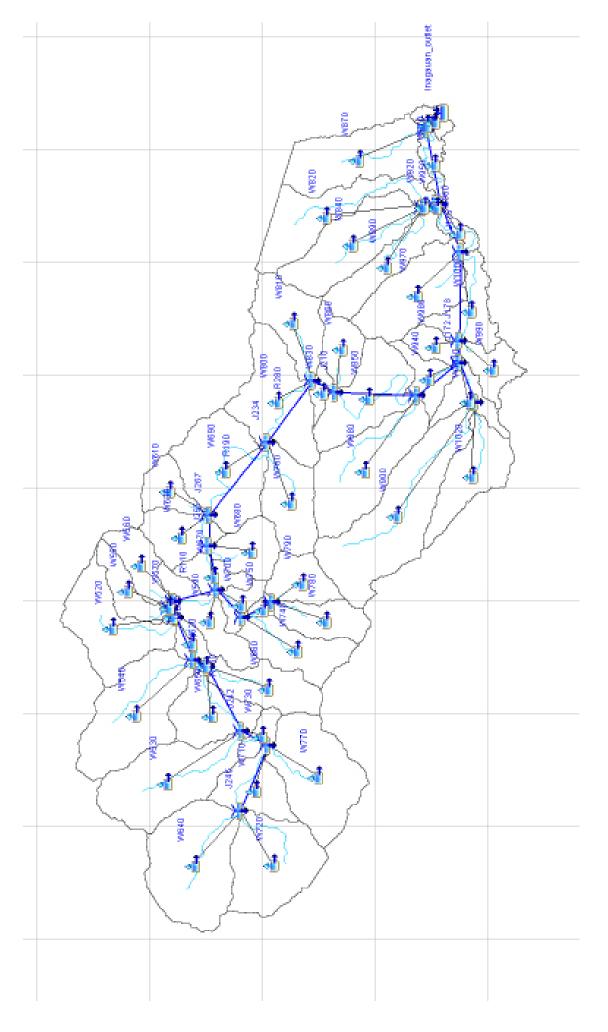
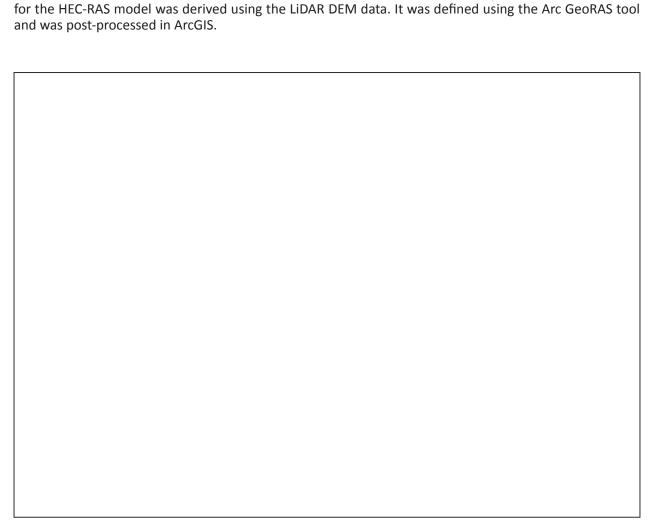


Figure 55. HEC-HMS generated Inagauan River Basin Model.

#### 5.4 Cross-section Data



Riverbed cross-sections of the watershed are crucial in the HEC-RAS model setup. The cross-section data

Figure 56. River cross-section of Inagauan River generated through Arcmap HEC GeoRAS tool

#### 5.5 Flo 2D Model

The automated modelling process allows for the creation of a model with boundaries that are almost exactly coincidental with that of the catchment area. As such, they have approximately the same land area and location. The entire area is divided into square grid elements, 10 meter by 10 meter in size. Each element is assigned a unique grid element number which serves as its identifier, then attributed with the parameters required for modelling such as x-and y-coordinate of centroid, names of adjacent grid elements, Manning coefficient of roughness, infiltration, and elevation value. The elements are arranged spatially to form the model, allowing the software to simulate the flow of water across the grid elements and in eight directions (north, south, east, west, northeast, northwest, southeast, southwest).

Based on the elevation and flow direction, it is seen that the water will generally flow from the west of the model to the east, following the main channel. As such, boundary elements in those particular regions of the model are assigned as inflow and outflow elements respectively.

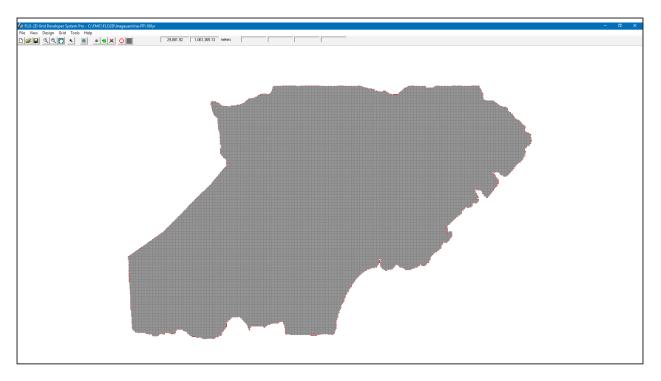


Figure 57. Screenshot of subcatchment with the computational area to be modeled in FLO-2D GDS Pro

The simulation is then run through FLO-2D GDS Pro. This particular model had a computer run time of 34.37207 hours. After the simulation, FLO-2D Mapper Pro is used to transform the simulation results into spatial data that shows flood hazard levels, as well as the extent and inundation of the flood. Assigning the appropriate flood depth and velocity values for Low, Medium, and High creates the following food hazard map. Most of the default values given by FLO-2D Mapper Pro are used, except for those in the Low hazard level. For this particular level, the minimum h (Maximum depth) is set at 0.2 m while the minimum vh (Product of maximum velocity (v) times maximum depth (h)) is set at 0 m2/s.

The creation of a flood hazard map from the model also automatically creates a flow depth map depicting the maximum amount of inundation for every grid element. The legend used by default in Flo-2D Mapper is not a good representation of the range of flood inundation values, so a different legend is used for the layout. In this particular model, the inundated parts cover a maximum land area of 38 286 800.00 m2.

There is a total of 48 222 036.13 m3 of water entering the model. Of this amount, 11 614 261.65 m3 is due to rainfall while 36 607 774.48 m3 is inflow from other areas outside the model. 4 851 129.50 m3 of this water is lost to infiltration and interception, while 5 848 386.93 m3 is stored by the flood plain. The rest, amounting up to 37 522 462.19 m3, is outflow.

# 5.6 HEC-HMS Model Values (Uncalibrated)

Enumerated in Table 26 are the range of values of the parameters in the model.

Table 26. Range of Calibrated Values for Inagauan River

Hydrologic Element	Calculation Type	Method	Parameter	Range of Calibrated Values
	Lana		Initial Abstraction (mm)	3 - 14
Dania	Loss	SCS Curve number	Curve Number	48 - 83
Basin	Tue ve ef e vee	Clark Unit	Time of Concentration (hr)	0.3 - 5
	Transform	Hydrograph	Storage Coefficient (hr)	0.5 - 9

Initial abstraction defines the amount of precipitation that must fall before surface runoff. The magnitude of the outflow hydrograph increases as initial abstraction decreases. The range of values from 3 to 14mm means that there is minimal amount of infiltration or rainfall interception by vegetation.

Curve number is the estimate of the precipitation excess of soil cover, land use, and antecedent moisture. The magnitude of the outflow hydrograph increases as curve number increases. For this watershed, there is a wide range of values from 48 to 83 depending on the subbasin.

Time of concentration and storage coefficient are the travel time and index of temporary storage of runoff in a watershed. The range of calibrated values from 0.3 hours to 9 hours determines the reaction time of the model with respect to the rainfall. The peak magnitude of the hydrograph also decreases when these parameters are increased.

Manning's roughness coefficient from 0.04 to 0.5 is high compared to the the common roughness of Philippine watersheds. This means that the riverbed is relatively rough and water will most likely flow slower. (Brunner, 2010).

# 5.7 River Analysis Model Simulation

The HEC-RAS Flood Model produced a simulated water level at every cross-section for every time step for every flood simulation created. The resulting model will be used in determining the flooded areas within the model. The simulated model will be an integral part in determining real-time flood inundation extent of the river after it has been automated and uploaded on the DREAM website. The sample map of Inagauan River using the HMS base flow is shown on Figure 67 below.

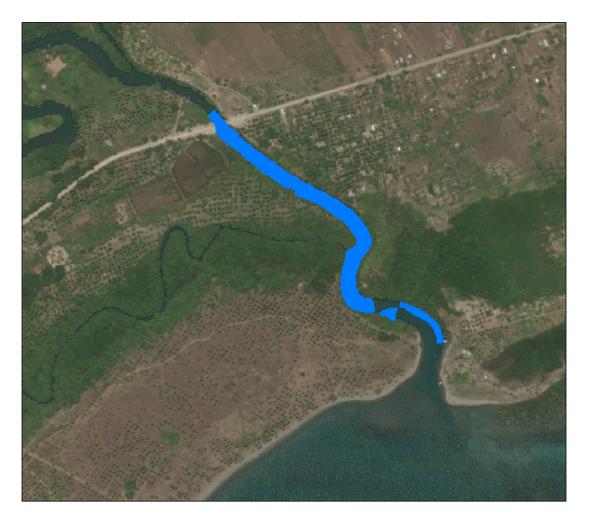


Figure 58. Sample output of Inagauan RAS Model

# 5.9 Flood Hazard and Flow Depth Map

The resulting hazard and flow depth maps for 5-, 25-, and 100-year rain return scenarios of the Inagauan floodplain are shown in Figure 59 to Figure 63. The floodplain, with an area of 38.29 sq. km., covers Puerto Princesa City. Table 27 shows the percentage of area affected by flooding per municipality.

Table 27. Municipalities affected in Inagauan Floodplain

City / Municipality	Total Area	Area Flooded	% Flooded
Puerto Princesa	2186.36	37.97	1.74%

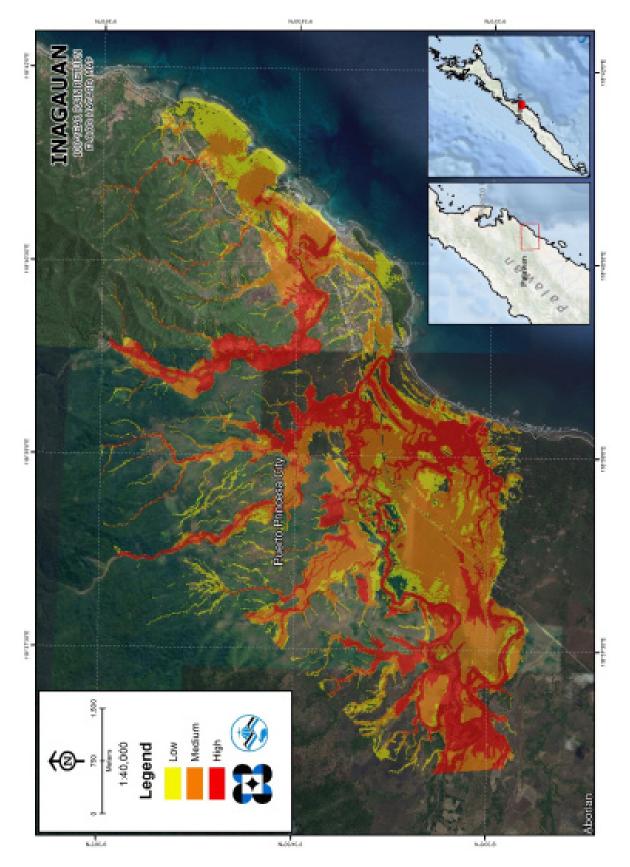


Figure 59. 100-year Flood Hazard Map for Inagauan Floodplain overlaid in Google Earth imagery

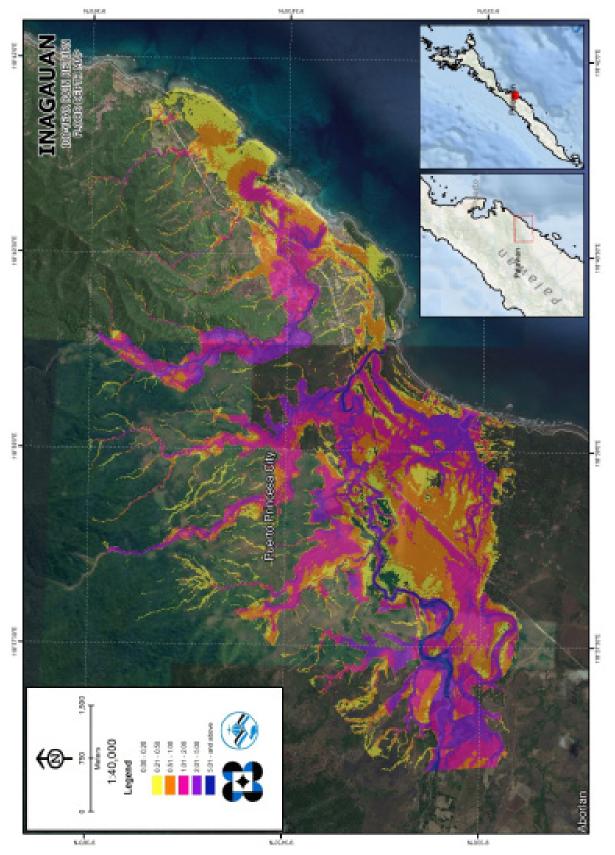


Figure 60. 100-year Flow Depth Map for Inagauan Floodplain overlaid in Google Earth imagery

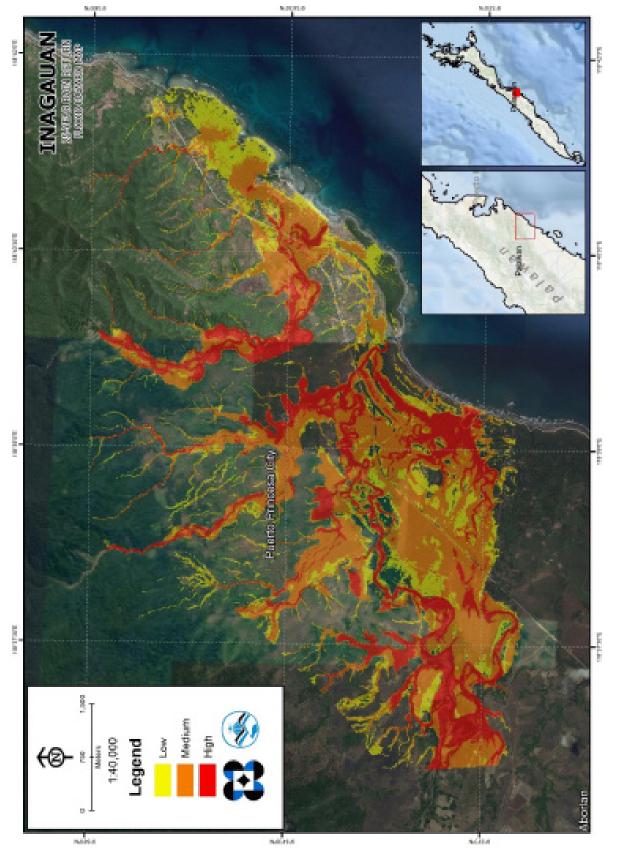
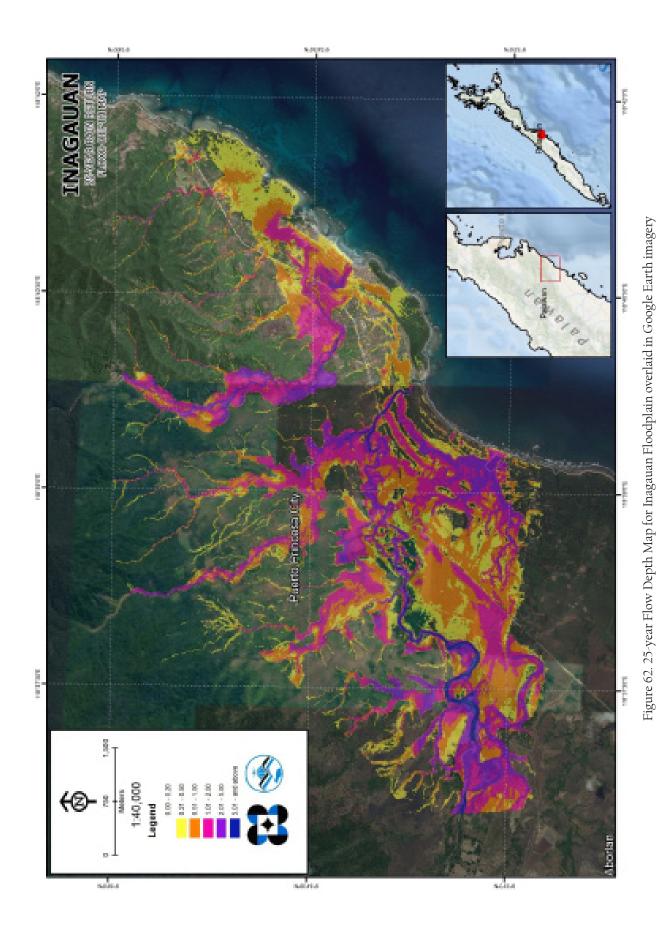


Figure 61. 25-year Flood Hazard Map for Inagauan Floodplain overlaid in Google Earth imagery



71

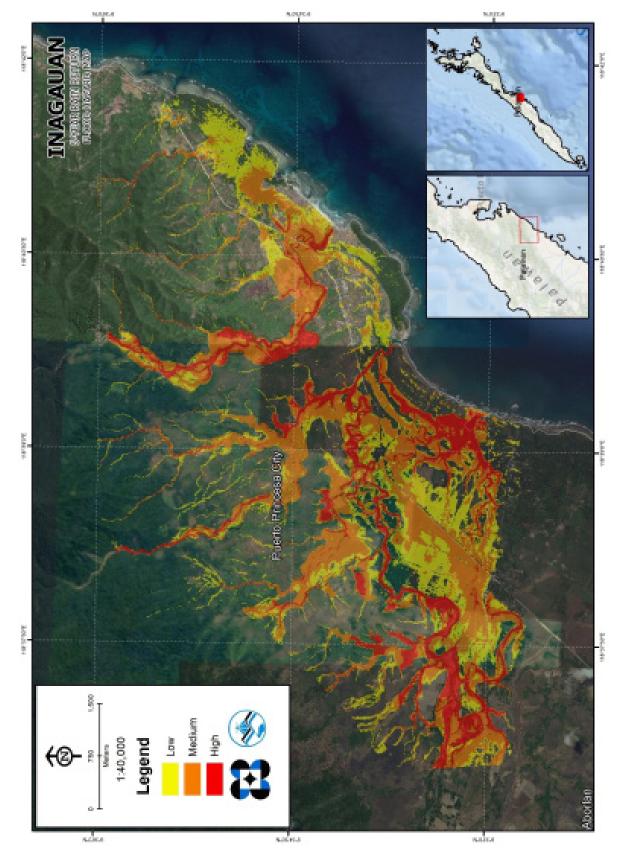


Figure 63. 5-year Flood Hazard Map for Inagauan Floodplain overlaid in Google Earth imagery

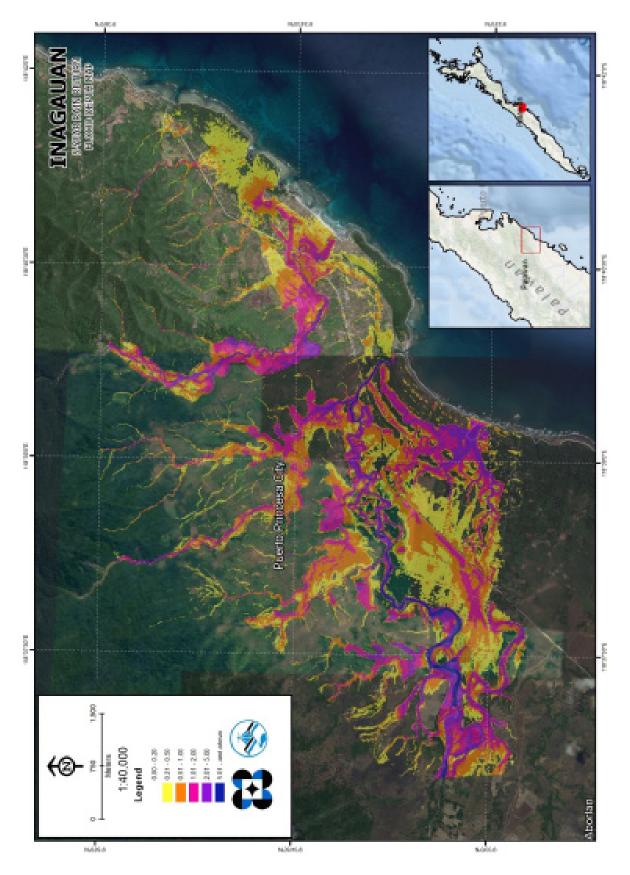


Figure 64. 5-year Flow Depth Map for Inagauan Floodplain overlaid in Google Earth imagery

# 5.9 Inventory of Areas Exposed to Flooding

Affected barangays in Inagauan river basin, grouped by municipality, are listed below. For the said basin, only one city consisting of three (3) barangays is expected to experience flooding when subjected to 5-yr rainfall return period.

For the 5-year return period, 1.08% of the municipality of Puerto Princesa City with an area of 2186.36 sq. km. will experience flood levels of less than 0.20 meters. 0.21% of the area will experience flood levels of 0.21 to 0.50 meters while 0.20%, 0.16%, 0.07%, and 0.01% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in Table 28 and shown in Figure 65 are the affected areas in square kilometers by flood depth per barangay.

Affected area (sq.km.)	Area of affected barangays in Puerto Princesa City (in sq. km)				
by flood depth (in m.)	Inagawan	Tagburos			
0.03-0.20	0.8	16.17	6.75		
0.21-0.50	0.48	3.07	1.01		
0.51-1.00	0.29	3.38	0.77		
1.01-2.00	0.18	2.85	0.52		
2.01-5.00	0.12	1.27	0.091		
> 5.00	0	0.21	0.00081		

Table 28. Affected Areas in Puerto Princesa City, Palawan during 5-Year Rainfall Return Period

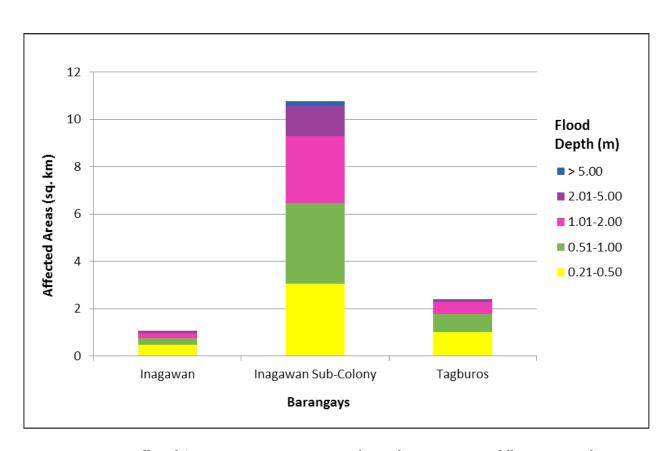


Figure 65. Affected Areas in Puerto Princesa City, Palawan during 5-Year Rainfall Return Period

For the 25-year return period, 0.98% of the municipality of Puerto Princesa City with an area of 2186.36 sq. km. will experience flood levels of less than 0.20 meters. 0.18% of the area will experience flood levels of 0.21 to 0.50 meters while 0.23%, 0.23%, 0.11%, and 0.01% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in Table 29 and shown in Figure 66 are the affected areas in square kilometers by flood depth per barangay.

Table 29. Affected Areas in Puerto Princesa City, Palawan during 25-Year Rainfall Return Period

Affected area (sq.km.)	Area of affected barangays in Puerto Princesa City (in sq. km)				
by flood depth (in m.)	Inagawan	Inagawan Sub-Colony	Tagburos		
0.03-0.20	0.53	14.54	6.34		
0.21-0.50	0.38	2.53	1.01		
0.51-1.00	0.53	3.66	0.84		
1.01-2.00	0.27	3.95	0.8		
2.01-5.00	0.16	2.02	0.15		
> 5.00	0	0.24	0.0016		

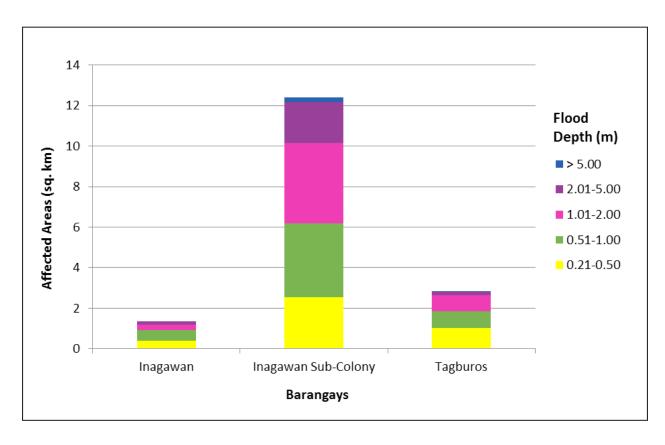


Figure 66. Affected Areas in Puerto Princesa City, Palawan during 25-Year Rainfall Return Period

For the 100-year return period, 0.93% of the municipality of Puerto Princesa City with an area of 2186.36 sq. km. will experience flood levels of less than 0.20 meters. 0.16% of the area will experience flood levels of 0.21 to 0.50 meters while 0.23%, 0.26%, 0.14%, and 0.01% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters,

Table 30. Affected Areas in Puerto Princesa City, Palawan during 100-Year Rainfall Return Period

Affected area (sq.km.)	Area of affected barangays in Puerto Princesa City (in sq. km)				
by flood depth (in m.)	Inagawan	Inagawan Sub-Colony	Tagburos		
0.03-0.20	0.42	13.73	6.15		
0.21-0.50	0.27	2.28	0.96		
0.51-1.00	0.64	3.46	0.91		
1.01-2.00	0.35	4.52	0.88		
2.01-5.00	0.2	2.69	0.23		
> 5.00	0	0.27	0.0034		

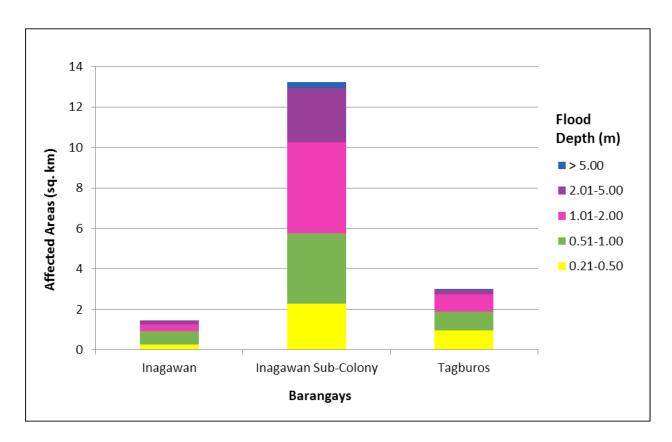


Figure 67. Affected Areas in Puerto Princesa City, Palawan during 100-Year Rainfall Return Period

Among the barangays in the municipality of Puerto Princesa City in Palawan, Inagawan Sub-Colony is projected to have the highest percentage of area that will experience flood levels at 1.23%. Meanwhile, Tagburos posted the second highest percentage of area that may be affected by flood depths at 0.42%.

#### 5.10 Flood Validation

In order to check and validate the extent of flooding in different river systems, there was a need to perform validation survey work. Field personnel gathered secondary data regarding flood occurrence in the area within the major river system in the Philippines.

From the Flood Depth Maps produced by Phil-LiDAR 1 Program, multiple points representing the different flood depths for different scenarios were identified for validation.

The validation personnel went to the specified points identified in a river basin and gathered data regarding the actual flood level in each location. Data gathering was done through a local DRRM office to obtain maps or situation reports about the past flooding events and through interviews with some residents who have knowledge of or have had experienced flooding in a particular area.

After which, the actual data from the field was compared to the simulated data to assess the accuracy of the Flood Depth Maps produced and to improve on what is needed. The points in the flood map versus its corresponding validation depths are shown in Figure 69.

The flood validation consisted of 86 points randomly selected all over the Inagauan flood plain. Comparing it with the flood depth map of the nearest storm event, the map has an RMSE value of 0.53m. Table 31 shows a contingency matrix of the comparison.

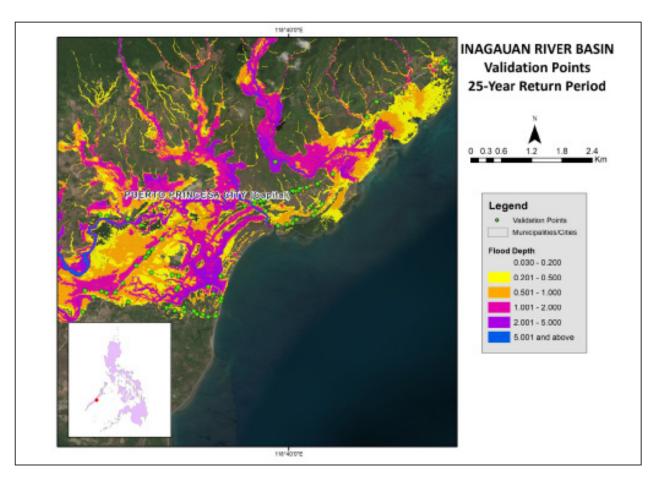


Figure 68. Validation points for 25-year Flood Depth Map of Inagauan Floodplain

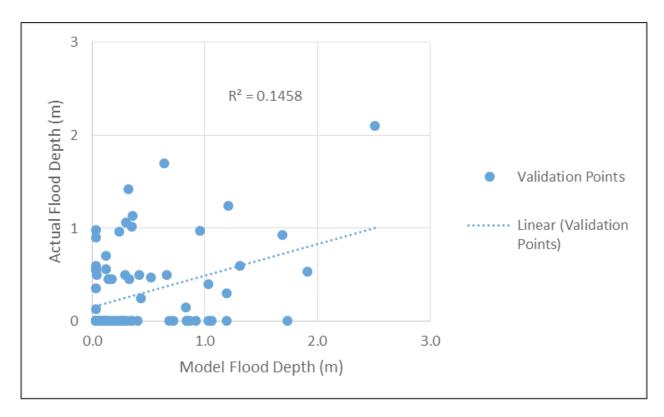


Figure 69. Flood map depth vs. actual flood depth

Table 31. Actual flood vs simulated flood depth at different levels in the Inagauan River Basin.

Actual Flood Depth		Modeled Flood Depth (m)						
(m)	0-0.20	0.21-0.50	0.51-1.00	1.01-2.00	2.01-5.00	> 5.00	Total	
0-0.20	34	10	7	4	0	0	55	
0.21-0.50	4	4	2	2	0	0	12	
0.51-1.00	7	1	1	3	0	0	12	
1.01-2.00	0	4	1	1	0	0	6	
2.01-5.00	0	0	0	0	1	0	1	
> 5.00	0	0	0	0	0	0	0	
Total	45	19	11	10	1	0	86	

The overall accuracy generated by the flood model is estimated at 47.67% with 41 points correctly matching the actual flood depths. In addition, there were 19 points estimated one level above and below the correct flood depths while there were 20 points and 4 points estimated two levels above and below, and three or more levels above and below the correct flood. A total of 4 points were overestimated while a total of 28 points were underestimated in the modelled flood depths of Inagauan. Table 32 depicts the summary of the Accuracy Assessment in the Inagauan River Basin Survey.

Table 32. Summary of Accuracy Assessment in the Inagauan River Basin Survey

	No. of Points	%
Correct	41	47.67
Overestimated	28	32.56
Underestimated	17	19.77
Total	86	100.00

#### REFERENCES

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Paringit, E.C., Balicanta, L.P., Ang, M.C., Lagmay, A.F., Sarmiento, C. 2017, Flood Mapping of Rivers in the Philippines Using Airborne LiDAR: Methods. Quezon City, Philippines: UP Training Center for Applied Geodesy and Photogrammetry

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UP TCAGP 2016. Acceptance and Evaluation of Synthetic Aperture Radar Digital Surface Model (SAR DSM) and Ground Control Points (GCP). Quezon City, Philippines: UP Training Center for Applied Geodesy and Photogrammetry

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# Annex 1. Optech Technical Specification of the Sensor

# 1. AQUARIUS SENSOR

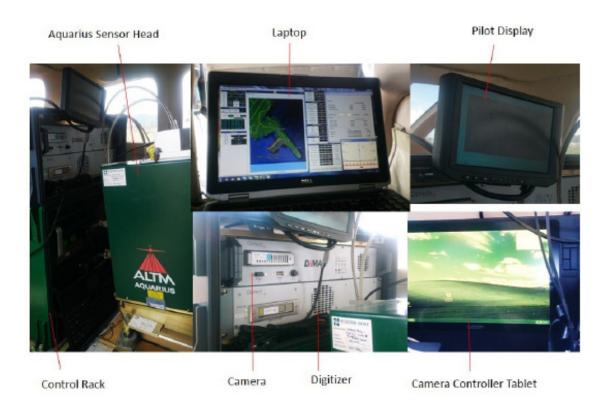


Figure A-1.1 Aquarius Sensor

# 2. PARAMETERS AND SPECIFICATIONS OF THE AQUARIUS SENSOR

Table A-1.1 Parameters and Specifications of the Aquarius Sensor

Parameter	Specification
Operational altitude	300-600 m AGL
Laser pulse repetition rate	33, 50. 70 kHz
Scan rate	0-70 Hz
Scan half-angle	0 to ± 25 °
Laser footprint on water surface	30-60 cm
Depth range	0 to > 10 m (for k < 0.1/m)
Topographic mode	
Operational altitude	300-2500
Range Capture	Up to 4 range measurements, including 1st, 2nd, 3rd, and last returns
Intensity capture	12-bit dynamic measurement range
Position and orientation system	POS AVTM 510 (OEM) includes embedded 72-channel GNSS receiver (GPS and GLONASS)
Data Storage	Ruggedized removable SSD hard disk (SATA III)
Power	28 V, 900 W, 35 A
Image capture	5 MP interline camera (standard); 60 MP full frame (optional)
Full waveform capture	12-bit Optech IWD-2 Intelligent Waveform Digitizer (optional)
Dimensions and weight	Sensor:250 x 430 x 320 mm; 30 kg; Control rack: 591 x 485 x 578 mm; 53 kg
Operating temperature	0-35°C
Relative humidity	0-95% no-condensing
Relative humidity	0-95% non-condensing

# 3. PEGASUS SENSOR



Figure A-1.2 Pegasus Sensor

#### 4. PARAMETERS AND SPECIFICATIONS OF THE PEGASUS SENSOR

Table A-1.2 Parameters and Specifications of the Pegasus Sensor

Parameter	Specification
Operational envelope (1,2,3,4)	150-5000 m AGL, nominal
Laser wavelength	1064 nm
Horizontal accuracy (2)	1/5,500 x altitude, 1σ
Elevation accuracy (2)	< 5-20 cm, 1σ
Effective laser repetition rate	Programmable, 100-500 kHz
Position and orientation system	POS AV ™AP50 (OEM)
Scan width (FOV)	Programmable, 0-75 °
Scan frequency (5)	Programmable, 0-140 Hz (effective)
Sensor scan product	800 maximum
Beam divergence	0.25 mrad (1/e)
Roll compensation	Programmable, ±37° (FOV dependent)
Vertical target separation distance	<0.7 m
Range capture	Up to 4 range measurements, including 1st, 2nd, 3rd, and last returns
Intensity capture	Up to 4 intensity returns for each pulse, including last (12 bit)
Image capture	5 MP interline camera (standard); 60 MP full frame (optional)
Full waveform capture	12-bit Optech IWD-2 Intelligent Waveform Digitizer
Data storage	Removable solid state disk SSD (SATA II)
Power requirements	28 V, 800 W, 30 A
Dimensions and weight	Sensor: 630 x 540 x 450 mm; 65 kg;
	Control rack: 650 x 590 x 490 mm; 46 kg
Operating Temperature	-10°C to +35°C
Relative humidity	0-95% non-condensing

<sup>1.</sup> Target reflectivity ≥20%

<sup>2.</sup> Dependent on selected operational parameters using nominal FOV of up to 40° in standard atmospheric conditions with 24-km visibility

<sup>3.</sup> Angle of incidence ≤20°

<sup>4.</sup> Target size ≥ laser footprint5 Dependent on system configuration

### Annex 2. NAMRIA Certificates of Reference Points Used

#### 1. MRW-18



December 11, 2015

#### CERTIFICATION

#### To whom it may concern:

This is to certify that according to the records on file in this office, the requested survey information is as follows -

	Province: OCC	IDENTAL MINDORO			
	Station N	lame: MRW-18			
	Orde	r: 2nd			
Island: LUZON Municipality: MAGSAYSAY	Barangay. MSL Elevi PRS				
Latitude: 12° 18' 45.39463"	Longitude	121° 8' 36.92441"	Ellipsoid	al Hgt:	21.29500 m
	WG	\$84 Coordinates			
Latitude: 12° 18' 40.53383"	Longitude	121° 8' 42.01469"	Ellipsoid	al Hgt:	71.37500 m
	PTM/F	PRS92 Coordinates			
Northing: 1381517.851 m.	Easting:	515618.524 m.	Zone:	3	
	UTM/	PRS92 Coordinates			
Northing: 1,361,734.74	Easting:	298,113.89	Zone:	51	

Location Description

#### MRW-18

From Municipality of Magsaysay, located in front of statue of President Ramon Magsaysay, inside the Municipal Compound, about 40 m SE of Municipal Bidg. of Magsaysay. Station is located in Municipality of Magsaysay, Occ. Mindoro, Mark is the head of a 4 in, copper nail flushed in a cement block embedded in the ground with inscriptions, "MRW-18, 2007, NAMRIA".

 Requesting Party:
 UP DREAM

 Purpose:
 Reference

 OR Number:
 8088861 I

 T.N.:
 2015-4114

RUEL DM. BELEN, MNSA Director, Mapping And Geodesy Branch





AMARIAN (FFICES: Main | Lawten-Ruema, Fiet Banishaio, 1634 Taguig City, Philippines: Tel. No.: 1632) 819-4531 in 41 Branch: 421 Banaca St. San Hissins, 1618 Blanks, Philippines, Tol. No. (632) 241-3404 to 95

www.samria.gov.ph

ISO 9001: 2008 CERTIFIED FOR HIAPPING AND GEOGRATIAL INFORMATION IMMINGEMENT

Figure A-2.1 MRW-18

#### 2. MRW-22



March 04, 2014

#### CERTIFICATION

#### To whom it may concern:

This is to certify that according to the records on file in this office, the requested survey information is as follows -

	Province: OCCI	DENTAL MINDORO			
	Station N	ame: MRW-22			
Island: LUZON Municipality: CALINTAAN	Order	2nd	Baranga	y: TAN	/AG
	PRSS	22 Coordinates			
Latitude: 12° 31' 36.76881"	Longitude:	120° 59" 13.46492"	Ellipsoid	al Hgt:	35.12700 m.
	WGS	84 Coordinates			
Latitude: 12° 31' 31.84278"	Longitude:	120° 59' 18.53734"	Ellipsoid	al Hgt:	84.27100 m.
	PTN	f Coordinates			
Northing: 1385214.96 m.	Easting:	498595.125 m.	Zone:	3	
	UTN	f Coordinates			
Northing: 1,385,563.72	Easting:	281,265.62	Zone:	51	

#### Location Description

#### MRW-22

From Abra de Ilog to San Jose, along Nat'l Road, approx. 9 Km. from Calintsan Town Proper, located Lumintao Bridge at Brgy. Tanyag, Sitio Marilao, Calintsan, Occ. Mindoro. Station is located at the N end of the catwalk of Lumintao Bridge. Mark is the head of a 4 in. copper nail flushed in a cement block embedded in the ground with inscriptions, "MRW-22, 2007, NAMRIA".

Requesting Party: UP-DREAM Pupose: Reference OR Number: 8795470 A T.N.: 2014-446

RUEL DM. BELEN, MNSA Director Mapping And Geodesy Branch

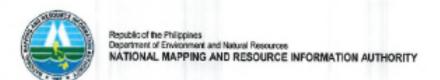




MAMRIA OFFICES:

Main : Lawton Avenue, Fort Bonilado, 1634 Taguig City, Philippines Tel. Ro.; (632) 810-4631 to 41 Branch : 431 Barroot St. Son Missles, 1618 Manile, Philippines, Tel. So. (632) 241-3474 to 58 www.normine.gov.ph

#### 3. MRE-56



October 28, 2015

#### CERTIFICATION

#### To whom it may concern:

This is to certify that according to the records on file in this office, the requested survey information is as follows -

	Province: ORIENTAL MINDORO		
	Station Name: MRE-56		
	Order: 2nd		
Island: LUZON	Barangay:		
Municipality: MANSALAY	MSL Elevation: PRS92 Coordinates		
Latitude: 12* 31' 25.76362"	Longitude: 121° 26' 25.21109"	Ellipsoidal Hgt:	7.87000 m.
	WGS84 Coordinates		
Latitude: 12° 31' 20.87629"	Longitude: 121° 26' 30.28143"	Ellipsoidal Hgt:	58.13600 m.
	PTM / PRS92 Coordinates		
Northing: 1384916.657 m.	Easting: 547857.861 m.	Zone: 3	
	UTM / PRS92 Coordinates		
Northing: 1,384,892.31	Easting: 330,530.08	Zone: 51	

Location Description

#### MIRE-56

From Calapan City to Bulalacco, along Nat1 Road approx. 4 Km. from Roxas Proper is an intersection of Roxas, Mansalay, Bongabong Road, turn left, approx. 14 Km. travet, right side of Nat1 Road located Mun. Hall of Mansalay, Oriental Mindoro, in front of Mansalay Hospital. Station is located in comer wall of Mun. Park in front of Mun. Hall. Mark is the head of a 4 in. copper nail flushed in a cement block embedded in the ground with inscrittions, "MRE-56, 2007, NAMRIA".

Requesting Party: ENGR. CHRISTOPHER CRUZ

Purpose: Reference OR Number: 8088472 I T.N.: 2015-3523

> RUEL DM. BELEN, MNSA Director Mapping And Geodesy Branch





SAMEDA OFFICES:

Man: Lanton Vencius, For Bondacis, "Bill Taguag City, Philogones Tell No. (602) 2913-4609 to 91 Brand 1-87 Bernaco St. Sannecous, 1010 Manua, Philogones, Tell No. (602) 2913-546416 St. Avew. n.e. merilla...gov..ph.

ISD 9001: 2008 CERTIFIED FOR INAPPING AND CEOSPATIAL INFORMATION INVANCEMENT

Figure A-2.3 MRE-56

#### 4. MRW-4203



March 25, 2014

#### CERTIFICATION

#### To whom it may concern:

This is to certify that according to the records on file in this office, the requested survey information is as follows -

		Province: OCCI	DENTAL MINDORO			
		Station Na	me: MRW-4203			
		Order	: 3rd			
Island: LUZ Municipality	ZON SAN JOSE			Barangay	MAP	AYA
		PRS	92 Coordinates			
Latitude: 1	12" 21' 24.45294"	Longitude:	121° 7' 26.92407"	Ellipsoida	l Hgt:	7.40100 m.
		WGS	84 Coordinates			
Latitude: 1	12° 21' 19.57973"	Longitude:	121° 7' 32.01059"	Ellipsoida	l Hgt	57.32000 m
		PTN	I Coordinates			
Northing: 1	366404.003 m.	Easting:	513501.246 m.	Zone:	3	
		UTI	f Coordinates			
Northing:	1,366,637.32	Easting:	296,032.79	Zone:	51	

#### Location Description

#### MRW-4203

From San Jose Town Proper to Brgy, Mapaya, approx. 7.8 Km. travel to reach brgy, hall. The station is located inside the compound of brgy, plaza, beside the gate post, left side fronting brgy, hall about 40 m NE of brgy, hall, 200 m NW of post Km. post 228, along Naf1 Road, 7 Km. to San Jose. Station is located in Brgy, Mapaya, San Jose, Occ., Mindoro, Mark is the head of a 4 in. copper nail flushed in a cement block embedded in the ground with inscriptions, "MRW-4203, 2007, NAMRIA".

Requesting Party: UP DREAM
Pupose: Reference
OR Number: 8795829 A
T.N.: 2014-643

RUEY DM. BEL'EN, MNSA Director, Mapping And Geodesy Branch





HAMRIN COTICES:

Nich : Landon Avenue, Fort Breshein, 1934 Fagnig City, Philippines — Tel. No.: (500) 810-4001 to 41 Barriot : 401 Barriot : 400 Barriot : 4

ISO 9001: 2000 CERTIFIED FOR MAPPING AND GEOSPATIAL INFORMATION MANAGEMENT

Figure A-2.4 MRW-4203

#### 5. MRW-4205



March 04, 2014

#### CERTIFICATION

#### To whom it may concern:

This is to certify that according to the records on file in this office, the requested survey information is as follows -

			/ Coordinates			
Latitude: 12	2° 26" 3.44072"	Longitude:	121° 2" 51.70789"	Ellipsoid	al Hgt	62.09500 m.
		was	84 Coordinates			
Latitude: 12	2° 26" 8.33964"	Longitude:	121° 2' 46.62783"	Elipsoid	al Hgt	12.56900 m.
meno-puny.	01110000	PRSS	92 Coordinates			
Island: LUZON Municipality: SAN JOSE		Order	. oru	Baranga	y: CEN	TRAL
		Station Na Order	me: MRW-4205			

#### Location Description

#### MRW-4205

From Abra de liog to San Jose, along Nat'l Road, approx. 10 Km, travel from San Jose Town Proper, 70 m E of Km, post 247 located Mabuhay Home Based ECCD Center for Health and Nutrition Bidg, located at Brgy. Central, Sitio Mabuhay, San Jose, Occ., Mindoro. Station is located beside fence, 2.0 m SW of Sitio Mabuhay Home Based ECCD Center of Health and Nutrition Post, 40 m NE of Nat'l Road, 70 m E of Km. post 247. Mark is the head of a 4 in. copper nail flushed in a cement block embedded in the ground with inscriptions, "MRW-4205, 2007, NAMRIA".

Requesting Party: UP-DREAM Pupose: Reference OR Number: 8795470 A T.N.: 2014-448

RUEL DM. BELEN, MNSA Director, Mapping And Geodesy Branch





MARKER OFFICES.

Hain: Lawton Evenue, Fort Bootinoto, 1634 Topoig City, Philippines: Tel. Soc. (632) 810-4831 to 41 Brench: 421 Borroot St. Son Risoles, 1818 Hande, Philippines, Tel. No. (632) 241-3414 to 16 www.normine.gov.ph

Figure A-2.5 MRW-4205

# Annex 3. Baseline Processing Report of Reference Points Used

Project information		Coordinate System	m	
Name:	C:\Users\qwerty\D ocuments\S usiness	Name:	UTM	
	Center - HCEUmrw18-mrw18a.vce	Datum:	PRS 92	
Size:	156 KB	Zone:	51 North (128E)	
Modified:	12/21/2015 2:56:44 PM (UTC:8)	Geold:	EGMPH	
Time zone:	Taipei Standard Time	Vertical datum:	E-STITE II	
Reference number:		ventical daton.		
Description:				

# Baseline Processing Report

#### **Processing Summary**

Observation	From	To	Solution Type	H. Prec. (Moter)	V. Prec. (Meter)	Geodetic Az.	Ellipsoid Dist. (Meter)	ΔHeight (Meter)
MRW-18 MRW- 18a (B1)	MRW-18	MRW-18a	Fixed	0.001	0.002	312"48"54"	6.566	0.551

#### Acceptance Summary

Processed	Passed	Flag	<b>*</b>	Fail	-
1	1	0		0	

#### MRW-18 - MRW-18a (6:38:43 AM-10:41:45 AM) (S1)

Baseline observation:	MRW-18 MRW-18a (B1)
Processed:	12/21/2015 3:00:47 PM
Solution type:	Fixed
Frequency used:	Dual Frequency (L1, L2)
Horizontal precision:	0.001 m
Vertical precision:	0.002 m
RMS:	0.000 m
Maximum PDOP:	2.035
Ephemeris used:	Broadcast
Antenna model:	NGS Absolute
Processing start time:	12/12/2015 6:38:51 AM (Local: UTC+8hr)
Processing stop time:	12/12/2015 10:41:45 AM (Local: UTC+8hr)
Processing duration:	04:02:54
Processing interval:	1 second

1

Figure A-3.1 Baseline Processing Report - A

#### Vector Components (Mark to Mark)

From:	MRW-18	MRW-18					
Grid		Local		Global			
Easting	298113.895 m	Latitude	N12"18'45.39463"	Latitude	N12"18'40.53383"		
Northing	1361734.745 m	Longitude	E121'08'38.92444"	Longitude	E121'08'42.01469"		
Elevation	20.797 m	Height	21.295 m	Height	71.375 m		

Ta:	MRW-18a					
Grid		Local		Global		
Easting	298109.109 m	Latitude	N12"18'45.53886"	Latitude	N1211840,679041	
Northing	1361739.241 m	Longitude	E121'08'36.76504"	Longitude	E121'08'41.85529"	
Elevation	21.348 m	Height	21.845 m	Height	71.926 m	

Vector						
ΔEasting	-4.786 m	NS Fwd Azimuth	312'48'54"	ΔX	4.336 m	
ΔNorthing	4.496 m	Ellipsoid Dist.	6.566 m	ΔY	2.137 m	
ΔElevation	0.551 m	ΔHeight	0.551 m	ΔZ	4.477 m	

#### Standard Errors

Vector errors:						
σ ΔE asting	0.000 m	σ NS fwd Azimuth	0'00'11"	σ ΔΧ	0.000 m	
σ ΔNorthing	0.000 m	σ Ellipsoid Dist.	0.000 m	σ ΔΥ	0.001 m	
σ ΔE levation	0.001 m	σ ΔHeight	0.001 m	σ ΔΖ	0.000 m	

#### Aposteriori Covariance Matrix (Meter\*)

	х	Υ	Z
x	0.0000002319		
Υ	-0.0000003031	0.0000009420	
z	-0.0000000578	0.0000001601	0.0000001302

Figure A-3.2 Baseline Processing Report - B

# Annex 4. The LiDAR Survey Team Composition

Figure A-4.1 LiDAR Survey Team Composition

Data Acquisition Component Sub -Team	Designation	Name	Agency / Affiliation
PHIL-LIDAR 1	Program Leader	ENRICO C. PARINGIT, D.ENG	UP-TCAGP
Data Acquisition Component Leader	Data Component Project Leader – I	ENGR. CZAR JAKIRI SARMIENTO	UP-TCAGP
	Chief Science Research Specialist (CSRS)	ENGR. CHRISTOPHER CRUZ	UP-TCAGP
Survey Supervisor	Supervising Science	LOVELY GRACIA ACUÑA	UP-TCAGP
	Research Specialist (Supervising SRS)	LOVELYN ASUNCION	UP-TCAGP

# FIELD TEAM

	Senior Science Research Specialist (SSRS)	PAULINE JOANNE ARCEO	UP-TACGP
	Research Associate (RA)	PATRICIA YSABEL ALCANTARA	UP-TCAGP
LiDAR Operation	RA	ENGR. LARAH KRISELLE PARAGAS	UP-TCAGP
	RA	ENGR. MILLIE SHANE REYES	UP-TCAGP
	RA	GRACE SINADJAN	UP-TCAGP
Ground Survey,	RA	ENGR. FRANK NICOLAS ILEJAY	UP-TCAGP
Data Download and Transfer	RA	GRACE SINADJAN	UP-TCAGP
	Airborne Security	SSG. ERIC CACANINDIN	PHILIPPINE AIR FORCE (PAF)
	/ moonie deddiney	SSG. BENJAMIN CARBOLLEDO	PAF
LiDAR Operation		CAPT. JEFFREY JEREMY ALAAR	ASIAN AEROSPACE CORPORATION (AAC)
	Pilot	CAPT. JACKSON JAVIER	AAC
		CAPT. SHERWIN ALFONSO III	AAC
		CAPT. JUSTINE JOYA	AAC

# Annex 5. Data Transfer Sheet For Inagauan Floodplain

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Figure A-5.1 Data Transfer Sheet for Inagauan Floodplain - A

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1BL/CSSMODIS42A	popula	98	100	2.00	65	13	133	679		22	9	9	20105	2	Z-EACTIVA DATA
18LK25GJ3428	position	90	0	272	22	4.83	35	273	2	2	100	94	26128	2	CIENCINAN DATA
(BLK29GH0A3A	propert	92	252	2	9	14.3	2	553	2	10.5	101	1908	200700	2	Distriction DATA
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12L/20MORS34SA	product.	199	5	15	100	12.9	9	5.0	2	7.00	169	180	13/140	-	Z-SEACTIVET SHCA
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Figure A-5.1 Data Transfer Sheet for Inagauan Floodplain - B

# Annex 6. Flight Logs

.. Flight Log for 3BLK29A59B Mission

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7 Pilot JAld ring 3 Co-Mat . Libraries 98.				
10 Date: 2) 28/14 12 Auton of Departure IN mark City	Produce):	2 Ampert of Aminal	LE Ampert of Aerival (Airport, City/Province):	
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19 Weacher				
20 Remarks:				
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Figure A-6.1 Flight Log for Mission 3BLK29A59B

Flight Log for 3BLK29C60A Mission

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CALLER ADVONOC CALIFORNIACE	1 LIDAR Operators (C. Forcander   RALTM Model: AGUA   3 Mission No mes 3 & LK356.)	SA 4 Type: VFR	a Aircaill Type: Ces nea 12064	6 Arroaft Identifications
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7	12 Minuted of Departure (Meson, Chyfriceines).  Macock, person	12 Airport of Arriva	12 Airport of Arriva (Airport, OttyProvince):	
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Figure A-6.2 Flight Log for Mission 3BLK29C60A

Flight Log for 3BLK29AS60B Mission

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17 Landing:	7 Pilot: Jayler 10 Date: _ / // /	8 Co-Pilot: / Alg ar	9 Route: Airport, Gty/Province):	12 Airport of Arrival	(Airport, City/Province):		107
Stand Solutions:  Stand Solutions:  Acquisition Flight Approach by Acquisition Flight Certified by Acquisition Flight Approach Signature over Printed Name (PAP Representative)  Signature over Printed Name (PAP Representative)	13 Engine On: 1438	14 Engine Off:	15 Total Engine Time:	16 Take off:	17 Landing:	18 Total Flight Time:	
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Figure A-6.3 Flight Log for Mission 3BLK29AS60B

Flight Log for 3BLK29N61A Mission

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14 Engl	Missin			12 Airport of Arrival (Airport, Cty/Province):		
20 Remarks:	Micen		16 Take off:	17 Landing:	18 Total Flight Time:	
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		n completed	-			
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Figure A-6.4 Flight Log for Mission 3BLK29N61A

Flight Log for 3BLK29B61B Mission

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No.	D 2 ALTM Model: 4 GUA	1 LIDAR Operator: LASUNGOD 2 ALTM Model 46UA 3 Mission Name: 36L 296618 4 Type: VFR	14/8 4Type: VFR	5 Aircraft Type: CesnnaT206H	6 Aircraft Identification:	
10 Date: 3/2/1/4	12 Airport of Departure (Airport, Gty/Province):	(Airport, Gty/Province):	12 Airport of Arrival	12 Airport of Arrival (Airport, Gty/Province):		
	14 Engine Off:	15 Total Engine Time:	16 Take off:	17 Landing:	18 Total Flight Time:	
19 Weather						
20 Remarks:						
	Completed	4 lines in	Ara B			
21 Booklams and Collisions						
ALTOGORIES BITG SOLUTIONS						
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Figure A-6.5 Flight Log for Mission 3BLK29B61B

Flight Log for 3BLK29BS62A Mission

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Figure A-6.6 Flight Log for Mission 3BLK29BS62A

Flight Log for 1BLK29NQRS345A Mission

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7 Mot CS AUTOMO 80	0 8	12 Airport of Departure (Airport, City/Province): 12 Airport of Arriva (Airport, City/Province): 13 Airport of Arriva (Airport, City/Province): 14 Airport of Arriva (Airport, City/Province): 15 Airport of Airport of Airport, City/Province): 15 Airport of Ai	UVAD KIZA	My Minort	Man Says ou	6 Aircraft Identification: 9/22
19 Weather	Clothdy	15 Total Engine Time. 2+47	16 Take off: 08/8	17 Landing:	l'he:	18 Total Flight Time: 27-37
20 Flight Classification 20a Billable 20 Forty Flight 20 Forty Flight 30 System Test Flight 40 Calibration Flight 50 Calibration Flight	20.b Non Billable  O Altcraft Test Flight  O AAC Admin Flight  O Others:	20.c Others  O LiDAR System Maintenance O Aircraft Maintenance O Phil-LiDAR Admin Activities	-	Survayed	Survayed Buk 29 N, Q, R&S.	ak & S.
22 Problems and Solutions O Weather Problem O Aircraft Problem O Pilot Problem O Others:						
Acquisition Flight Approved by PROLINE PROCOCO Signature over Printed Name (Cost Uner Propresentative)	Acquisition Fight Certified by Cartified by Cartified by Cartified by Significant over Princed Name (PAF Representative)		Pitot-in-Command  ( A H N N S)  Signature over Printed Name		Lidar Operator PRIMIQUE TRANCO Sprinture over Printed Name	Aircraft Wechanicy Technician N. RC. C. C. C. Signature over Printed Name

Figure A-6.7 Flight Log for Mission 1BLK29NQRS345A

Flight Log for 1BLK29R346A Mission

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# Annex 7. Flight Status

FLIGHT STATUS REPORT Inagauan FLOODPLAIN February 28-March 3, 2014; December 11-12, 2015

Table A-7.1 Flight Status Report

FLIGHT NO.	AREA	MISSION	OPERATOR	DATE FLOWN	REMARKS
1158A	BLK29A	3BLK29A59B	L. ASUNCION	28-Feb-14	Covered 10 lines.
1160A	BLK29C	3BLK29C60A	L. PARAGAS	01-Mar-14	Mission completed.
1162A	BLK29A & 29D	3BLK29AS+DV60B	L. ASUNCION	01-Mar-14	Mission completed. Continuation of BLK29A and covered voids in BLK29D.
1164A	BLK29N & 29B	3BLK29N+B61A	L. PARAGAS	02-Mar-14	Mission completed. Covered lines 10 and 11 of BLK29B.
1166A	BLK29B	3BLK29B61B	L. ASUNCION	02-Mar-14	Covered gap in line 10 from the morning flight.
1168A	BLK29B, 29A, 29D, 29C & 29K	3BLK29BS+ AB+DB+CV+ KV62B	L. PARAGAS	03-Mar-14	Mission completed.
3078P	BLK 29N, 29Q, 29R & 29S.	1BLK29NQRS345A	P. ARCEO	11-Dec-15	Surveyed BLK 29N, Q, R & S.
3082P	BLK 29R	1BLK29R346A	G. SINADJAN	12-Dec-15	Surveyed BLK29R.

#### LAS/SWATH BOUNDARIES PER MISSION FLIGHT

FLIGHT LOG NO. 1158A AREA: BLK29A MISSION NAME: 3BLK29A59B

PARAMETERS: Alt: 550 Scan Freq: 40 kHz Scan Angle: 18 deg



Figure A-7.1 Swath for Flight No. 1158A

FLIGHT LOG NO. 1160A AREA: BLK29C MISSION NAME: 3BLK29C60A

PARAMETERS: Alt: 650 Scan Freq: 40 kHz Scan Angle: 18 deg

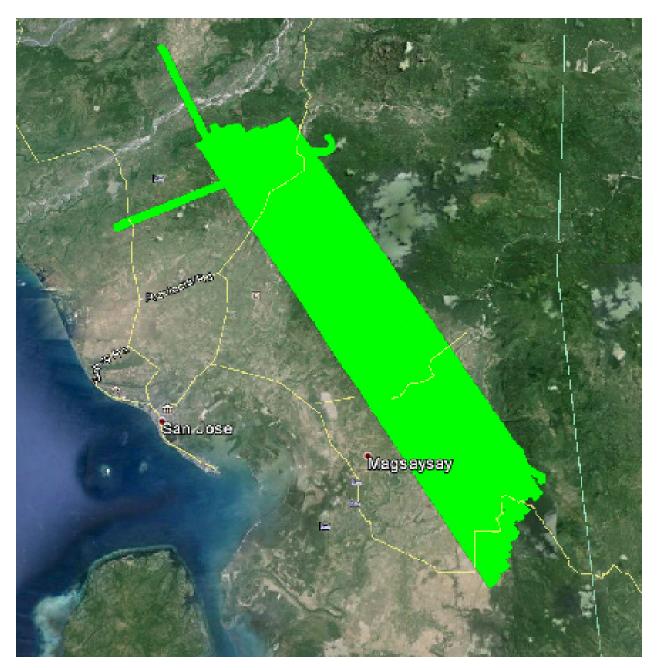


Figure A-7.2 Swath for Flight No. 1160A

FLIGHT LOG NO. 1162A

AREA: BLK29A AND BLK29D MISSION NAME: 3BLK29AS+DV60B

PARAMETERS: Alt: 600 Scan Freq: 40 kHz Scan Angle: 18 deg



Figure A-7.3 Swath for Flight No. 1162A

FLIGHT LOG NO. 1164A

AREA: BLK29N AND BLK29B MISSION NAME: 3BLK29N+B61A

PARAMETERS: Alt: 600 Scan Freq: 40 kHz Scan Angle: 18 deg

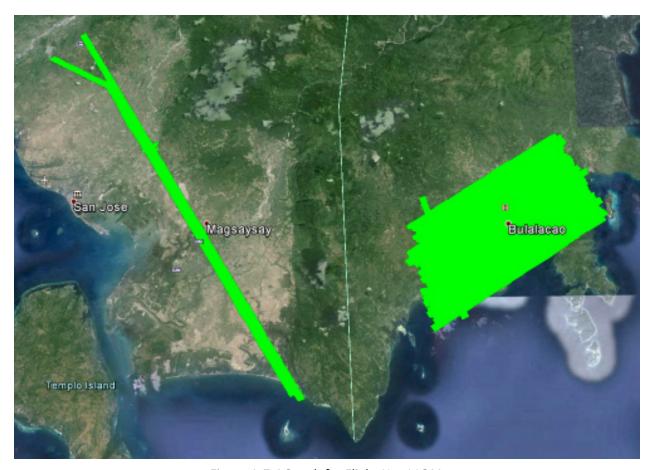


Figure A-7.4 Swath for Flight No. 1164A

FLIGHT LOG NO. 1166A AREA: BLK29B MISSION NAME: 3BLK29B61B

PARAMETERS: Alt: 600 Scan Freq: 40 kHz Scan Angle: 18 deg



Figure A-7.5 Swath for Flight No. 1166A

FLIGHT LOG NO. 1168A

AREA: BLK29B, BLK29A, BLK29D, BLK29C AND BLK29K

MISSION NAME: 3BLK29BS+AB+DB+CV+KV62B

PARAMETERS: Alt: 550 Scan Freq: 40 kHz Scan Angle: 18 deg

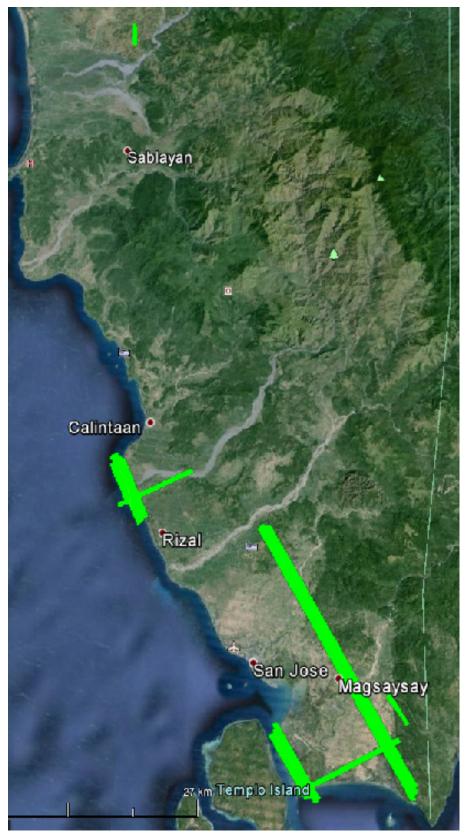


Figure A-7.6 Swath for Flight No. 1168A

FLIGHT NO.: 3078P

AREA: BLK29N, BLK29Q, BLK29R & BLK29S

MISSION NAME: 1BLK29NQRS345A

PARAMETERS: Alt: 850 m Scan Freq: 32 Scan Angle: 25

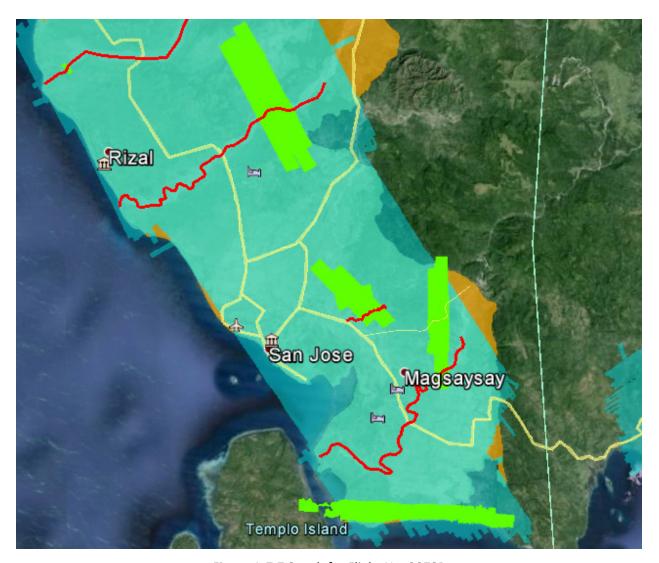


Figure A-7.7 Swath for Flight No. 3078P

FLIGHT NO.: 3082P AREA: BLK29R

MISSION NAME: 1BLK29R346A

PARAMETERS: Alt: 1100 m Scan Freq: 30 Scan Angle: 25



Figure A-7.8 Swath for Flight No. 3082P

# **Annex 8. Mission Summary Reports**

Table A-8.1 Mission Summary Report for Blk 29A

Table A-0.1 Mission Summary	The port for Bik 23A
Flight Area	Davao Oriental
Mission Name	Blk29A
Inclusive Flights	1158A. 1162A, 1168A
Range data size	29.98 GB
POS	677 MB
Image	72.6 GB
Transfer date	04/23/2014
Solution Status	
Number of Satellites (>6)	No
PDOP (<3)	Yes
Baseline Length (<30km)	Yes
Processing Mode (<=1)	No
Smoothed Performance Metrics (in cm)	
RMSE for North Position (<4.0 cm)	1.2
RMSE for East Position (<4.0 cm)	2.1
RMSE for Down Position (<8.0 cm)	4.4
NWSE TO BOWN TOSITION (NO.0 CIT)	7.7
Boresight correction stdev (<0.001deg)	0.000443
IMU attitude correction stdev (<0.001deg)	0.002081
GPS position stdev (<0.01m)	0.0294
Minimum % overlap (>25)	43.28%
Ave point cloud density per sq.m. (>2.0)	2.76
Elevation difference between strips (<0.20 m)	Yes
, , , , , , , , , , , , , , , , , , ,	
Number of 1km x 1km blocks	194
Maximum Height	165.41 m
Minimum Height	43.10 m
Classification (# of points)	
Ground	104,722,532
Low vegetation	130,224,088
Medium vegetation	60,206,940
High vegetation	16,625,237
Building	4,886,963
שמווטוווצ	4,000,505
Orthophoto	Yes
Processed by	Engr. Carlyn Ann Ibañez, Engr. Harmond Santos, Engr. Gladys Mae Apat

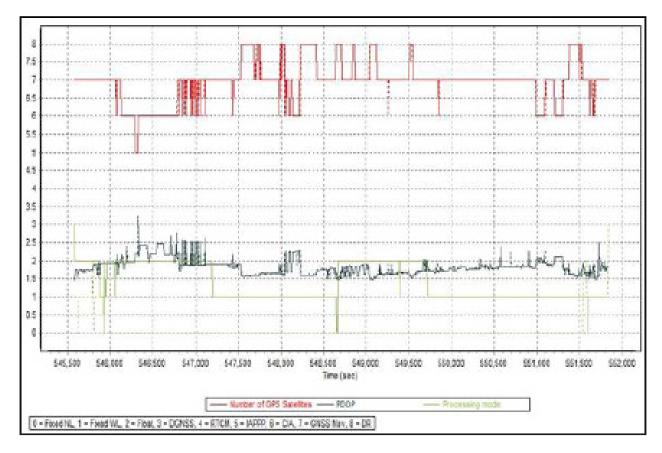


Figure A-8.1. Solution Status

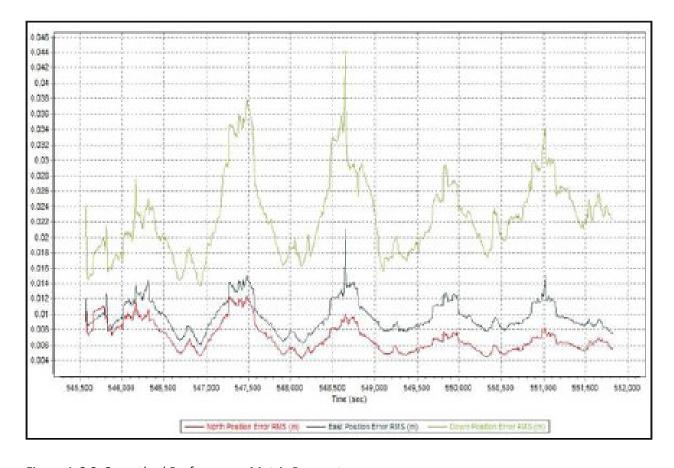


Figure A-8.2. Smoothed Performance Metric Parameters

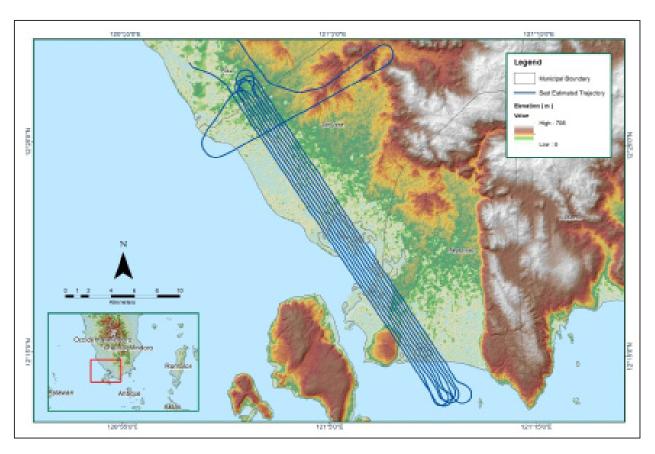


Figure A-8.3. Best Estimated Trajectory

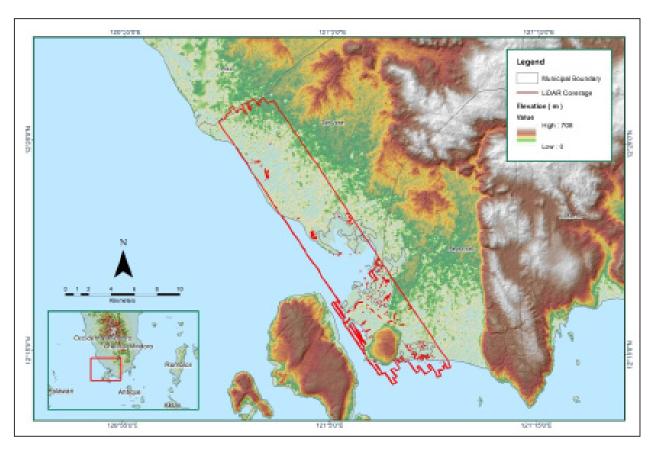


Figure A-8.4. Coverage of LiDAR data

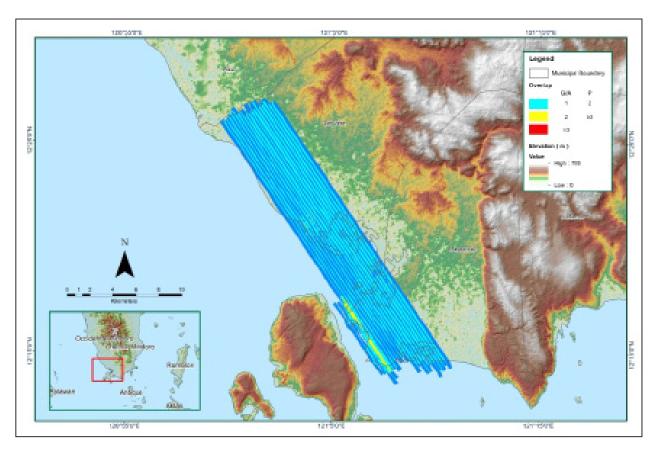


Figure A-8.5. Image of data overlap

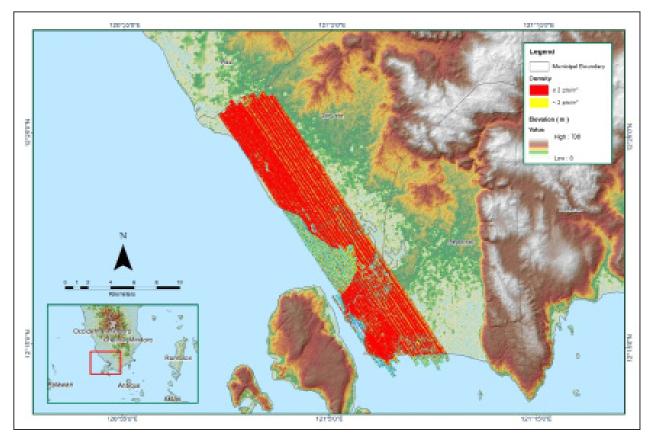


Figure A-8.6. Density map of merged LiDAR data

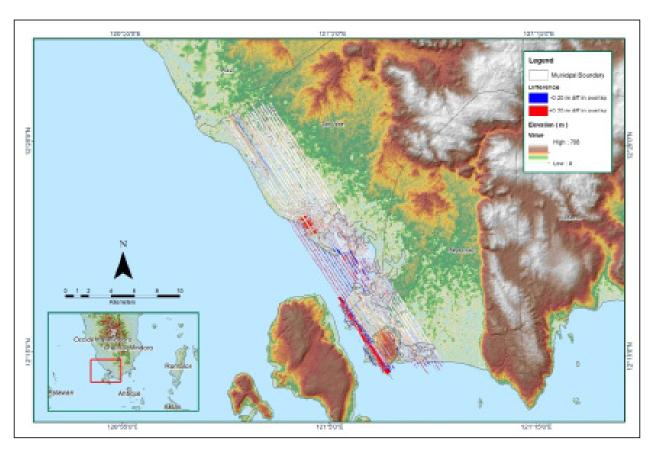


Figure A-8.7. Elevation difference between flight lines

Table A-8.2 Mission Summary Report for Blk 29B

Flight Area	Davao Oriental
Mission Name	Blk29B
Inclusive Flights	1164A. 1166A, 1168A
Range data size	38.4 GB
POS	791 MB
Image	22.5 GB
Transfer date	04/23/2014
Solution Status	
Number of Satellites (>6)	Yes
PDOP (<3)	Yes
Baseline Length (<30km)	No
Processing Mode (<=1)	Yes
Smoothed Performance Metrics (in cm)	
RMSE for North Position (<4.0 cm)	1.3
RMSE for East Position (<4.0 cm)	1.6
RMSE for Down Position (<8.0 cm)	3.8
,	
Boresight correction stdev (<0.001deg)	0.000629
IMU attitude correction stdev (<0.001deg)	0.002510
GPS position stdev (<0.01m)	0.0158
Minimum % overlap (>25)	43.01%
Ave point cloud density per sq.m. (>2.0)	3.09
Elevation difference between strips (<0.20 m)	Yes
Number of 1km x 1km blocks	197
Maximum Height	258.38 m
Minimum Height	46.77 m
Willimum Height	40.77 111
Classification (# of points)	
Ground	115,311,089
Low vegetation	138,979,099
Medium vegetation	95,318,939
High vegetation	30,819,969
Building	2,104,153
Orthophoto	Yes
Processed by	Ma. Victoria Rejuso, Engr. Harmond Santos, Engr. John Dill Macapagal

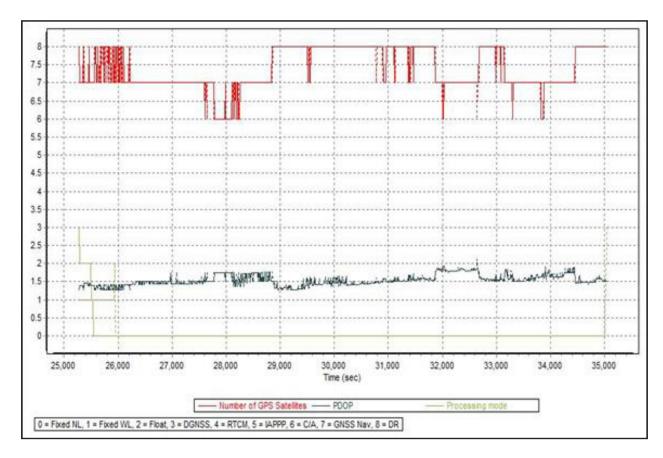


Figure A-8.8. Solution Status

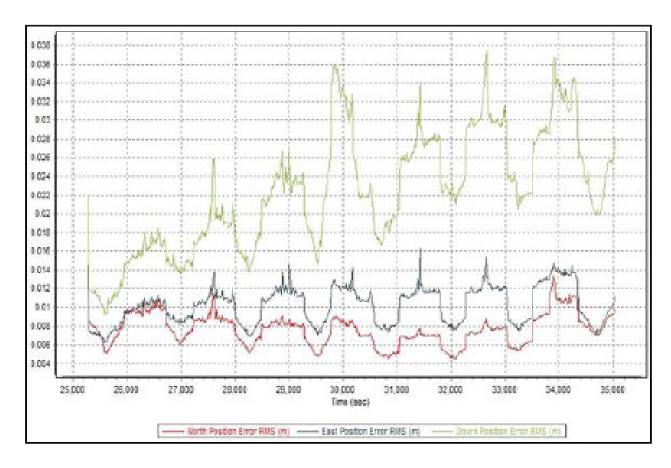


Figure A-8.9. Smoothed Performance Metric Parameters

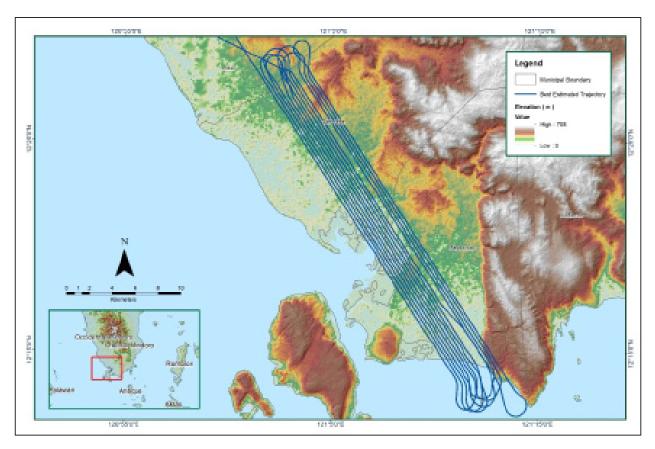


Figure A-8.10. Best Estimated Trajectory

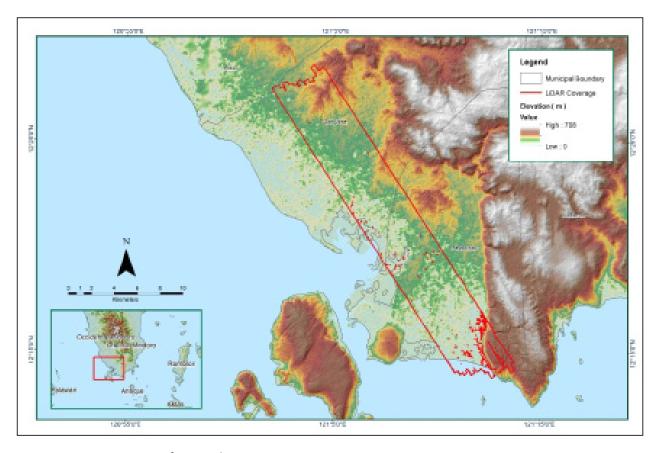


Figure A-8.11. Coverage of LiDAR data

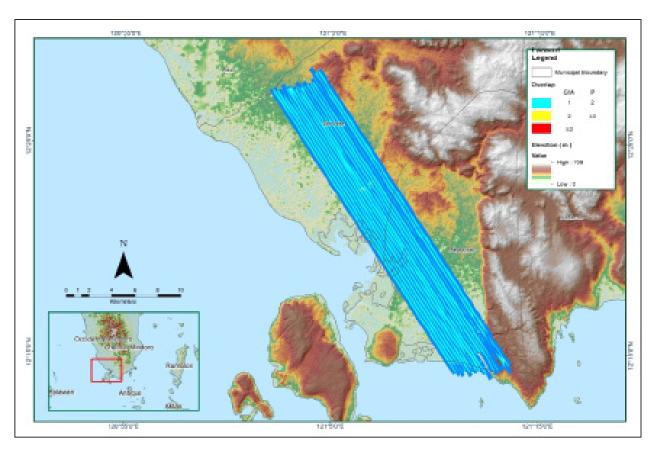


Figure A-8.12. Image of data overlap

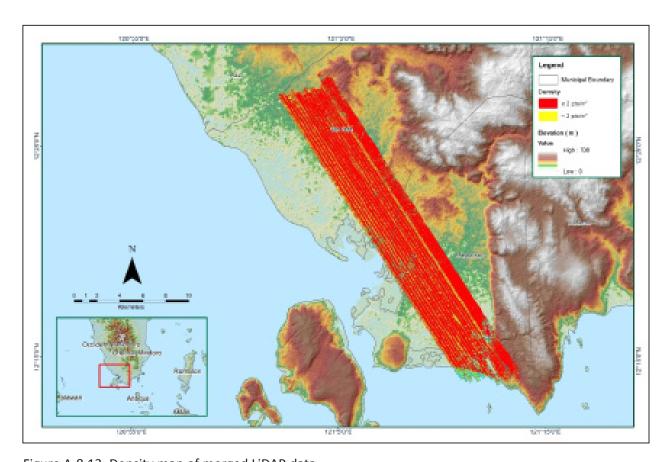


Figure A-8.13. Density map of merged LiDAR data

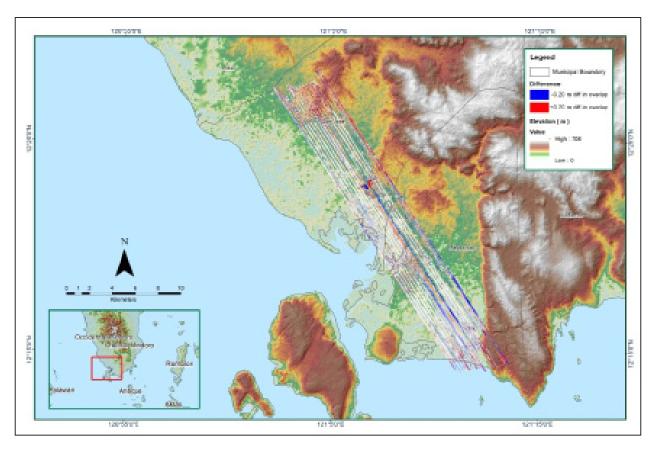


Figure A-8.14. Elevation difference between flight lines

Table A-8.3 Mission Summary Report for Blk 29C

Flight Area	Davao Oriental
Mission Name	Blk29C
Inclusive Flights	1160A
Range data size	14.1 GB
POS	268 MB
Image	13.5 GB
Transfer date	04/23/2014
Solution Status	
	No
Number of Satellites (>6)	No
PDOP (<3)	No
Baseline Length (<30km)	Yes
Processing Mode (<=1)	Yes
Smoothed Performance Metrics (in cm)	
RMSE for North Position (<4.0 cm)	4.8
RMSE for East Position (<4.0 cm)	2.2
RMSE for Down Position (<8.0 cm)	5.1
Boresight correction stdev (<0.001deg)	0.000373
IMU attitude correction stdev (<0.001deg)	0.001768
GPS position stdev (<0.01m)	0.0032
	0.0002
Minimum % overlap (>25)	44.16%
Ave point cloud density per sq.m. (>2.0)	3.59
Elevation difference between strips (<0.20 m)	Yes
Number of 1km x 1km blocks	143
Maximum Height	481.99 m
Minimum Height	51.54 m
Classification (# of points)	
Ground	109,156,938
Low vegetation	80,757,959
Medium vegetation	73,247,510
High vegetation	71,877,948
Building	1,281,773
Orthophoto	Yes
Processed by	Engr. Irish Cortez, Engr. Harmond Santos, Ailyn Biñas

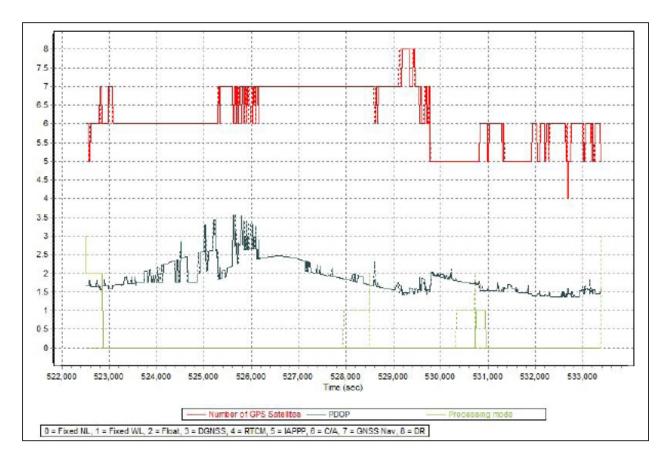


Figure A-8.15. Solution Status

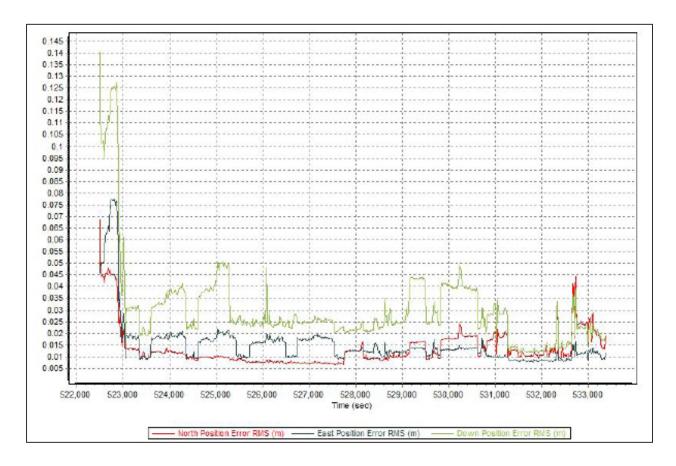


Figure A-8.16. Smoothed Performance Metric Parameters

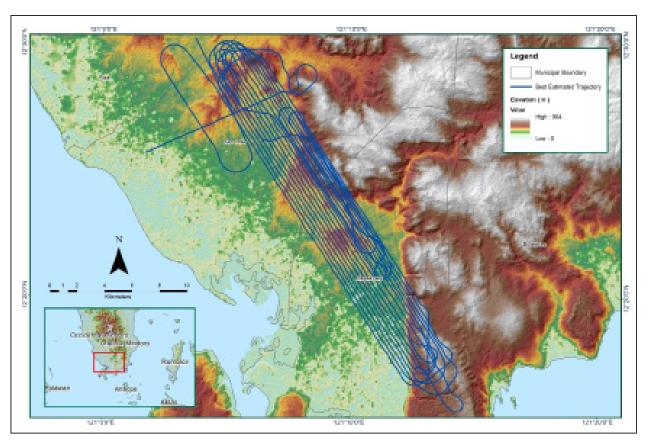


Figure A-8.17. Best Estimated Trajectory

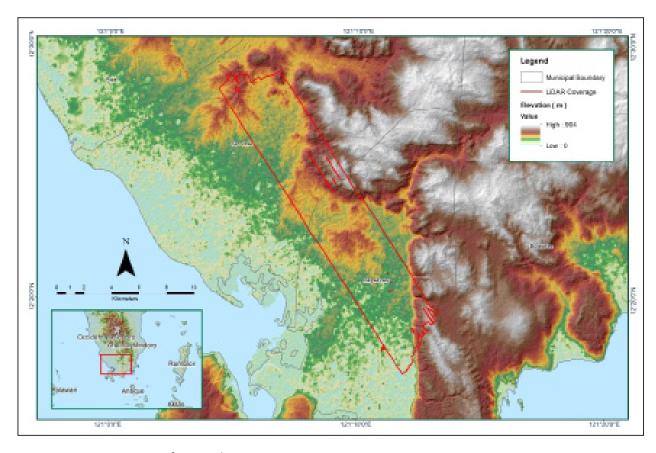


Figure A-8.18. Coverage of LiDAR data

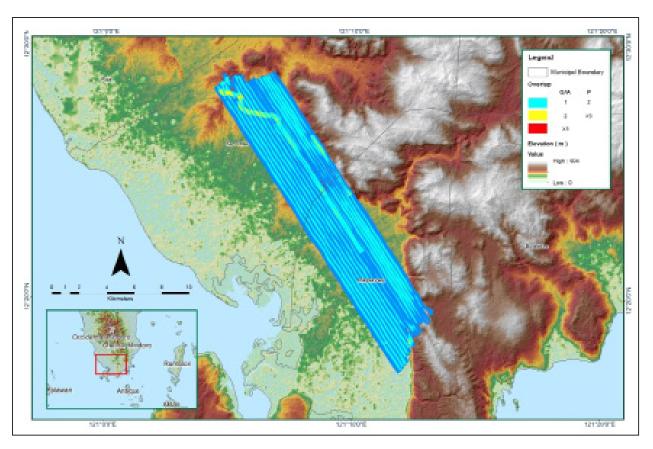


Figure A-8.19. Image of data overlap

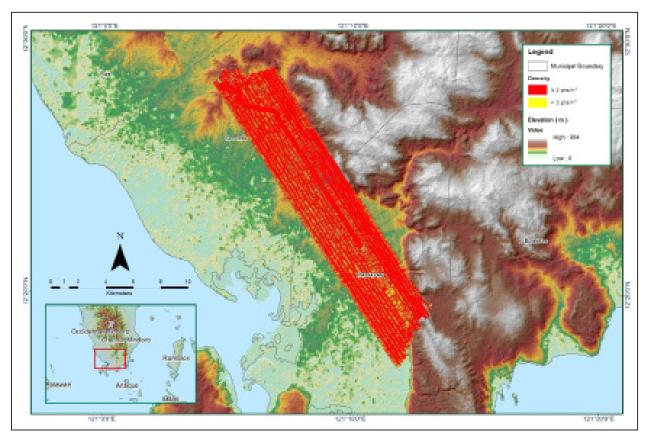


Figure A-8.20. Density map of merged LiDAR data

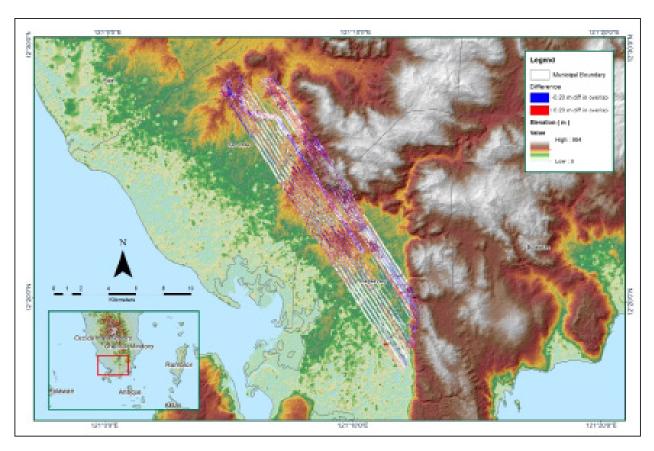


Figure A-8.21. Elevation difference between flight lines

Table A-8.4 Mission Summary Report for Blk 29A\_additional

Table A-6.4 Mission Summary Nepo	_	
Flight Area	Davao Oriental	
Mission Name	Blk29A_additional	
Inclusive Flights	3078P	
Range data size	6.2GB	
POS	167MB	
Image	12.9MB	
Transfer date	January 15, 2016	
Solution Status		
Number of Satellites (>6)	Yes	
PDOP (<3)	Yes	
Baseline Length (<30km)	No	
Processing Mode (<=1)	No	
Smoothed Performance Metrics (in cm)		
RMSE for North Position (<4.0 cm)	1.01	
RMSE for East Position (<4.0 cm)	1.16	
	4.25	
RMSE for Down Position (<8.0 cm)	4.25	
Boresight correction stdev (<0.001deg)	0.359804	
IMU attitude correction stdev (<0.001deg)	0.083211	
GPS position stdev (<0.01m)	0.0024	
Minimum % overlap (>25)		
Ave point cloud density per sq.m. (>2.0)		
Elevation difference between strips (<0.20 m)	Yes	
Lievation americane seeween strips (10.20 iii)	1.65	
Number of 1km x 1km blocks	40	
Maximum Height	231.12 m	
Minimum Height	50.50 m	
Classification (# of points)		
Ground	15,453,565	
Low vegetation	10,246,556	
Medium vegetation	13,004,794	
High vegetation	17,341,456	
Building	271,742	
building	2/1,/42	
Orthophoto	Yes	
Processed by	Engr. Abigail Ching, Engr. Harmond Santos, Engr. Melissa Fernandez	

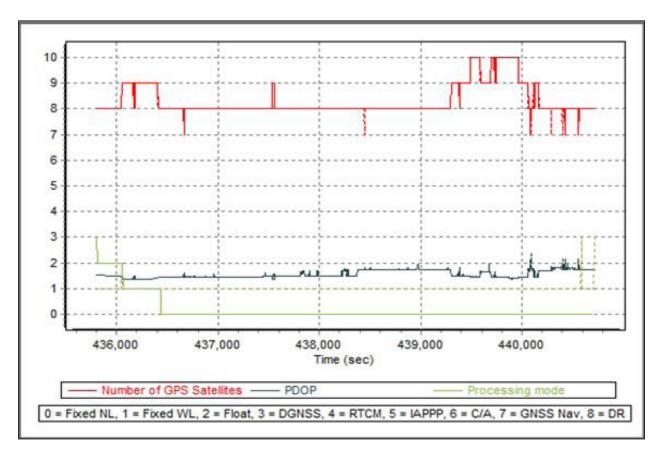


Figure A-8.22. Solution Status

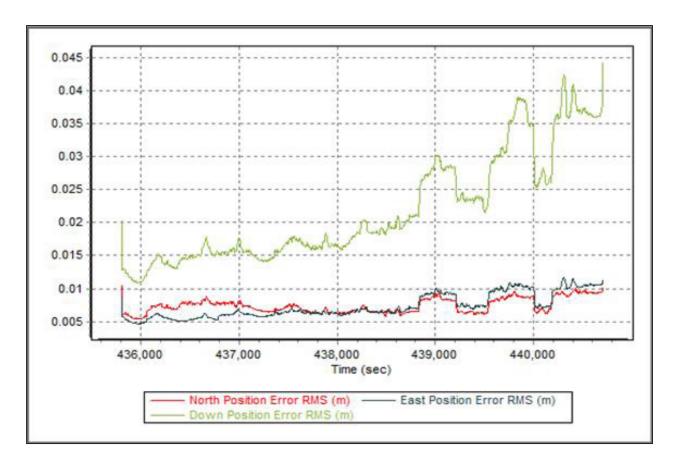


Figure A-8.23. Smoothed Performance Metric Parameters

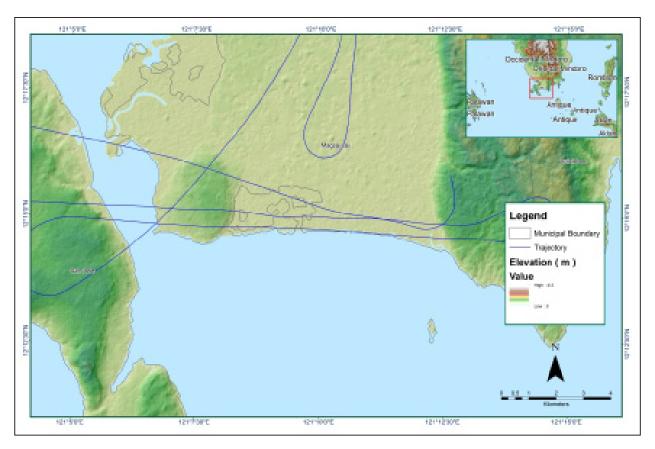


Figure A-8.24. Best Estimated Trajectory

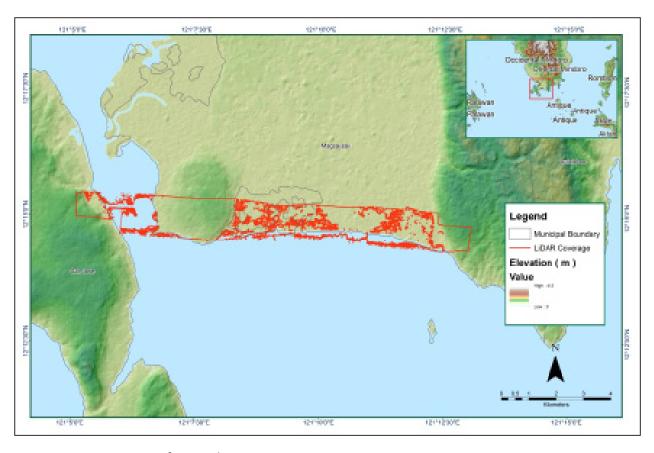


Figure A-8.25. Coverage of LiDAR data

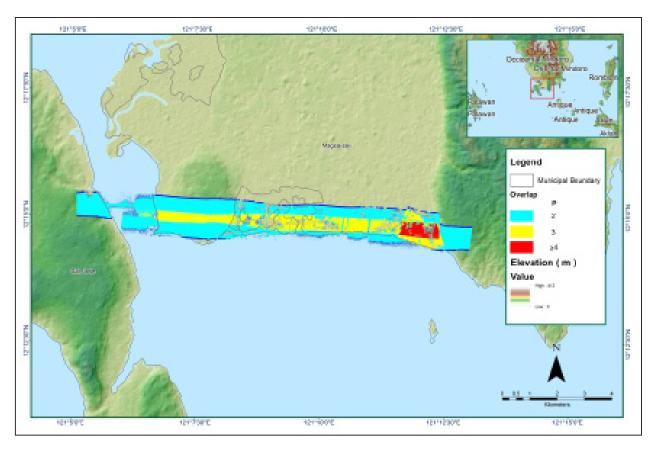


Figure A-8.26. Image of data overlap

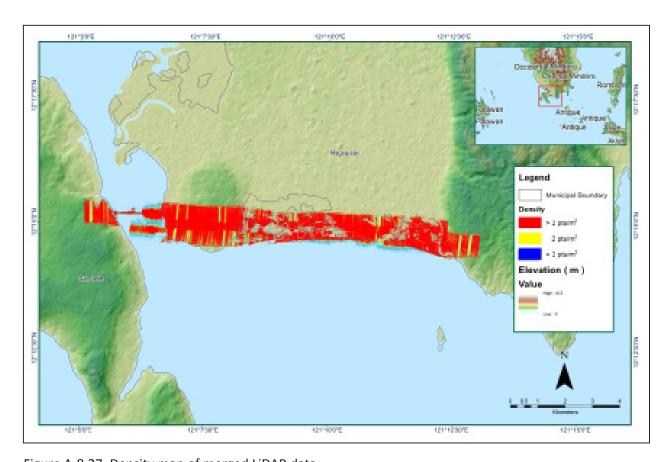


Figure A-8.27. Density map of merged LiDAR data

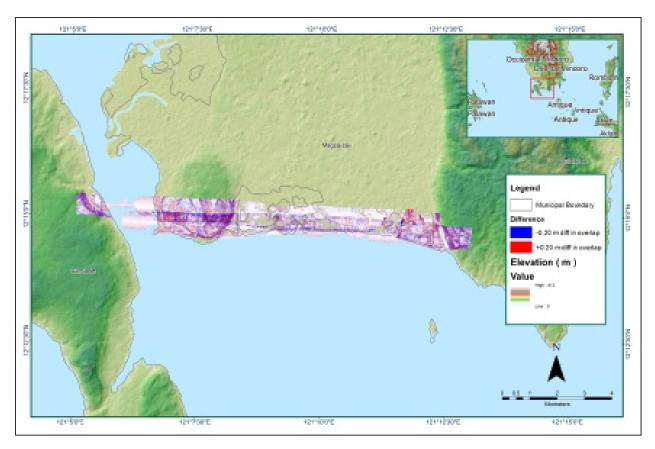


Figure A-8.28. Elevation difference between flight lines

Table A-8.5 Mission Summary Report for Blk 29B\_additional

Flight Area	Davao Oriental
Mission Name	
Inclusive Flights	Blk29B_additional 3078P
Range data size	6.2GB
Base data size	7.02MB
POS	167MB
	12.9MB
Image Transfer date	January 15, 2016
Transier date	January 13, 2010
Solution Status	
Number of Satellites (>6)	Yes
PDOP (<3)	Yes
Baseline Length (<30km)	No
Processing Mode (<=1)	No
Trocessing Mode (<-1)	NO NO
Smoothed Performance Metrics (in cm)	
RMSE for North Position (<4.0 cm)	1.01
RMSE for East Position (<4.0 cm)	1.16
RMSE for Down Position (<8.0 cm)	4.25
Boresight correction stdev (<0.001deg)	0.359804
IMU attitude correction stdev (<0.001deg)	0.083211
GPS position stdev (<0.01m)	0.0024
Minimum % overlap (>25)	17.73%
Ave point cloud density per sq.m. (>2.0)	1.81
Elevation difference between strips (<0.20 m)	Yes
Number of 1km x 1km blocks	18
Maximum Height	322.24 m
Minimum Height	53.37 m
Classification (# of points)	
Ground	13,137,914
Low vegetation	6,256,653
Medium vegetation	4,588,390
High vegetation	8,091,371
Building	187,119
Orthophoto	Yes
Processed by	Engr. Abigail Ching, Engr. Harmond Santos, Engr. Melissa Fernandez

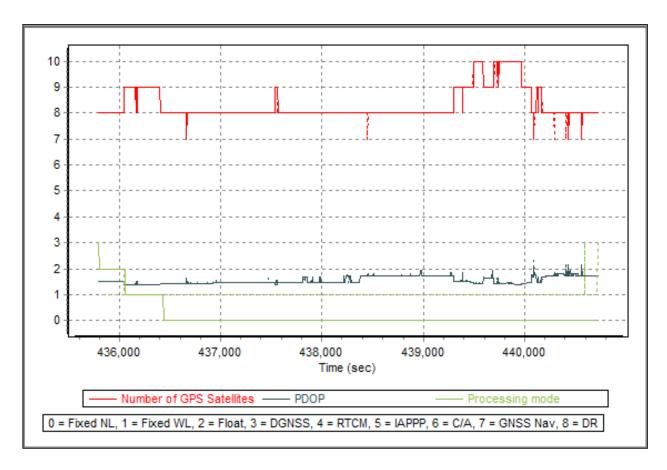


Figure A-8.29. Solution Status

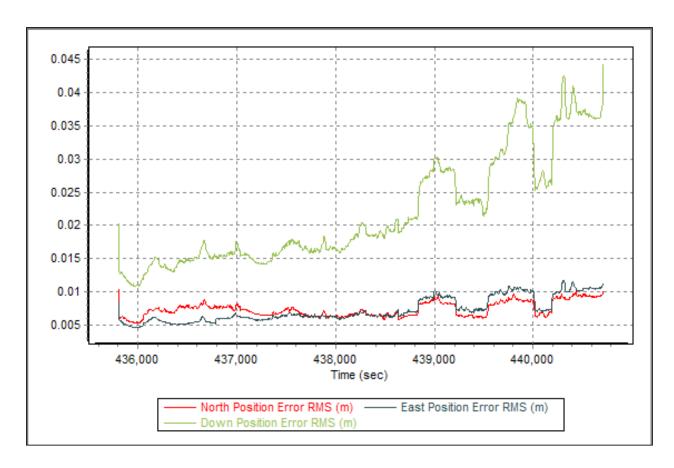


Figure A-8.30. Smoothed Performance Metric Parameters



Figure A-8.31. Best Estimated Trajectory

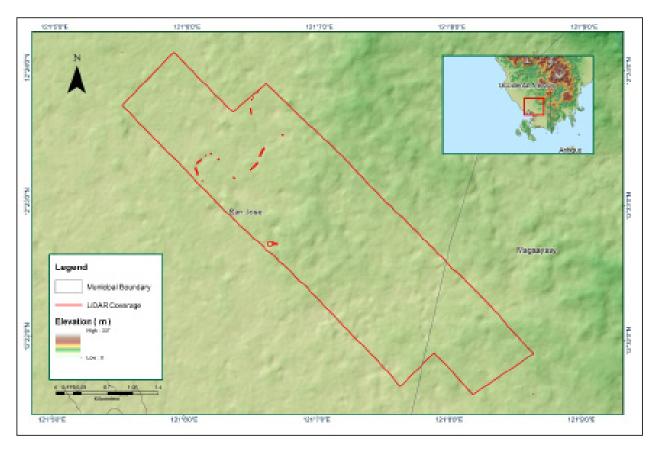


Figure A-8.32. Coverage of LiDAR data

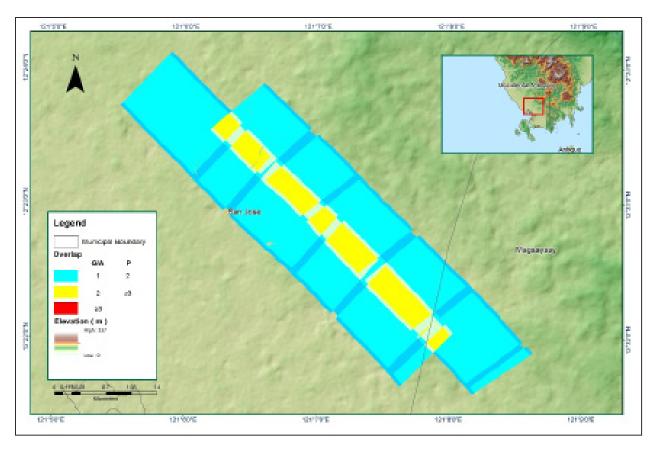


Figure A-8.33. Image of data overlap

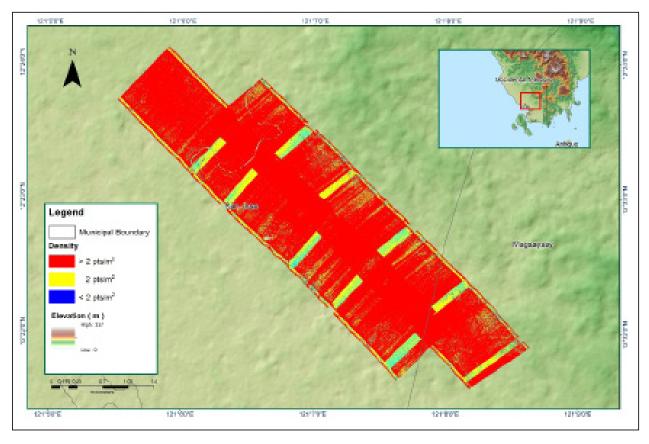


Figure A-8.34. Density map of merged LiDAR data

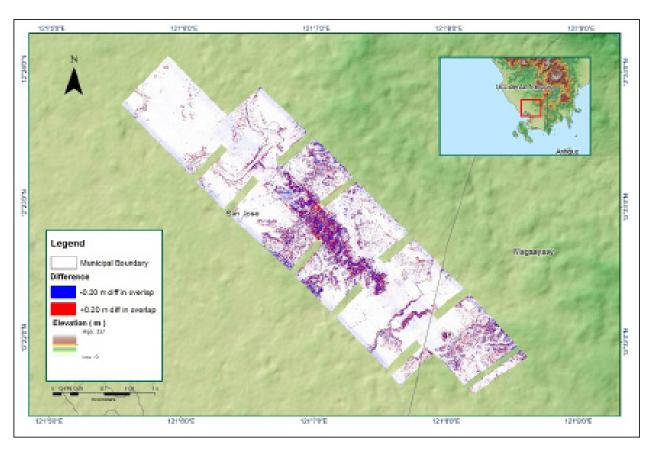


Figure A-8.35. Elevation difference between flight lines

Table A-8.6 Mission Summary Report for Blk 29C\_additional

Flight Area	Davao Oriental
Mission Name	Blk29C_additional
Inclusive Flights	3078P, 3082P
Range data size	15.42GB
POS	341MB
Image	26MB
Transfer date	January 15, 2016
Solution Status	
Number of Satellites (>6)	Yes
PDOP (<3)	Yes
Baseline Length (<30km)	Yes
Processing Mode (<=1)	Yes
Smoothed Performance Metrics (in cm)	
RMSE for North Position (<4.0 cm)	0.79
RMSE for East Position (<4.0 cm)	0.78
RMSE for Down Position (<8.0 cm)	1.67
Boresight correction stdev (<0.001deg)	0.359804
IMU attitude correction stdev (<0.001deg)	0.083211
GPS position stdev (<0.01m)	0.0024
Minimum % overlap (>25)	40.31
Ave point cloud density per sq.m. (>2.0)	2.10
Elevation difference between strips (<0.20 m)	Yes
Number of 1km x 1km blocks	108
Maximum Height	521.55 m
Minimum Height	51.07 m
Classification (# of points)	
Ground	132,379,761
Low vegetation	125,752,184
Medium vegetation	199,077,351
High vegetation	599,574,573
Building	15,255,571
Orthophoto	Yes
Processed by	Engr. Abigail Ching, Engr. Harmond Santos, Kathryn Claudyn Zarate

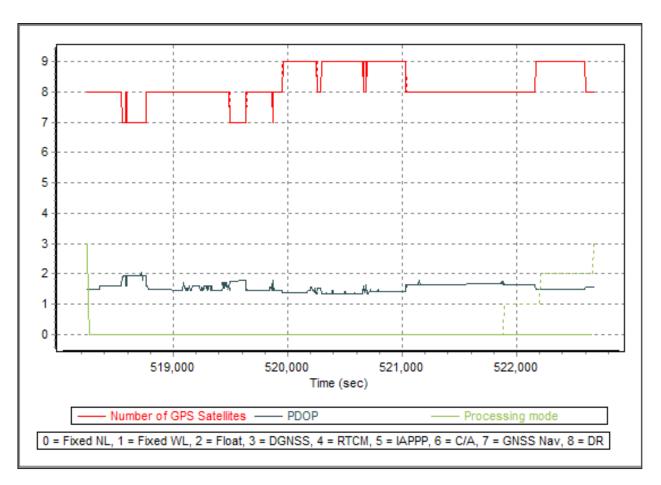


Figure A-8.36. Solution Status

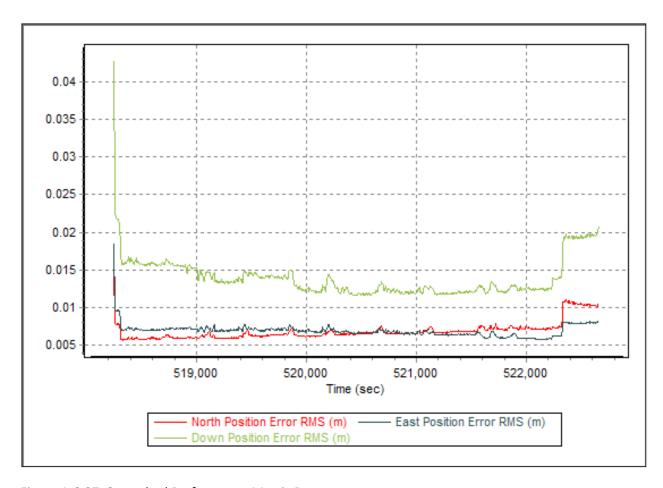


Figure A-8.37. Smoothed Performance Metric Parameters

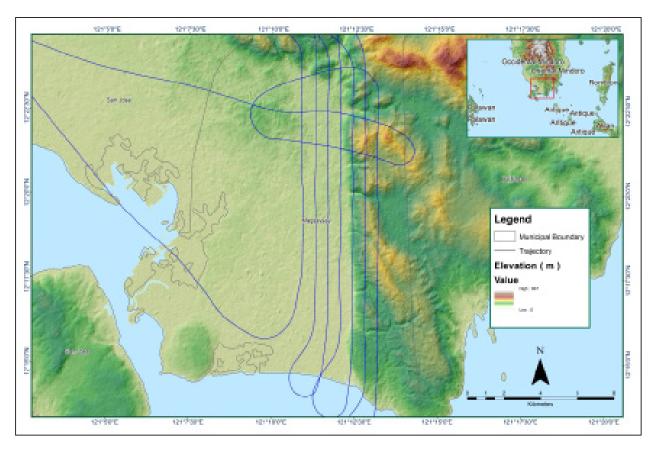


Figure A-8.38. Best Estimated Trajectory

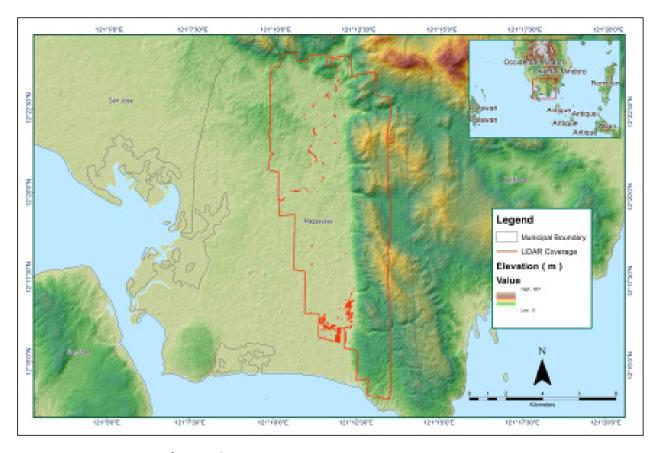


Figure A-8.39. Coverage of LiDAR data

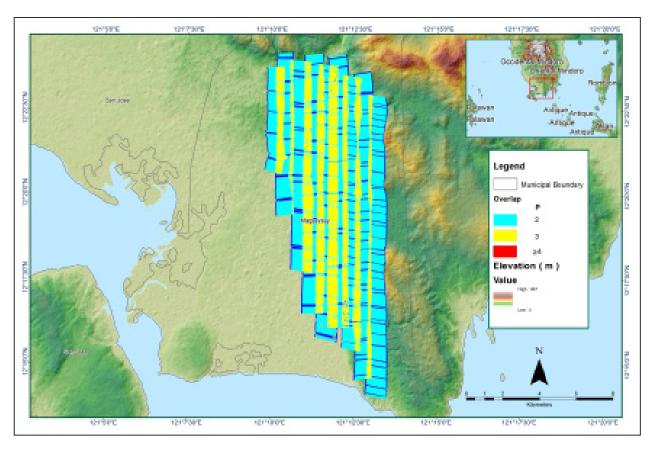


Figure A-8.40. Image of data overlap

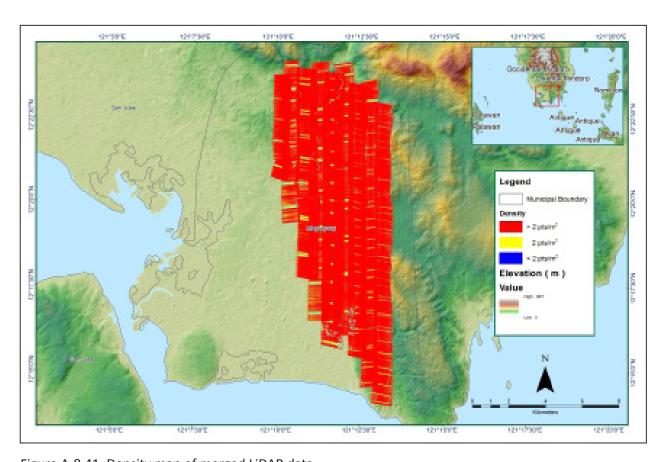


Figure A-8.41. Density map of merged LiDAR data

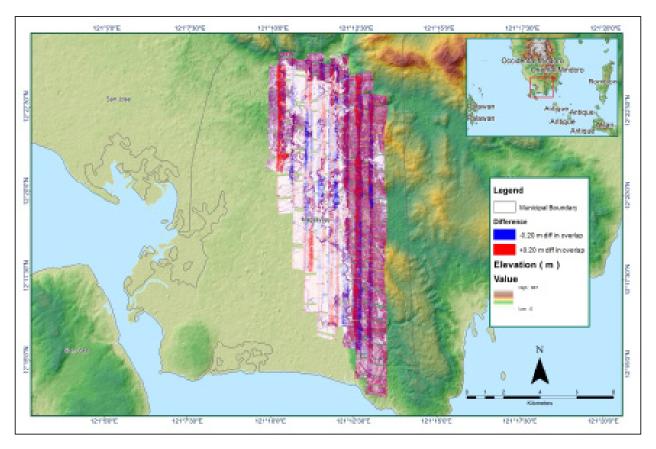


Figure A-8.42. Elevation difference between flight lines

Annex 9. Inagauan Model Basin Parameters

Table A-9.1 Inagauan Model Basin Parameters

	SCS Cur	SCS Curve Number Loss		Clark Unit Hydrograph Transform	graph Transform			
Sub-basin	Initial Abstraction (mm)	Curve	Impervious (%)	Time of Concentration (HR)	Storage Coefficient (HR)	Initial Discharge (M3/S)	Recession Constant	Ratio to Peak
W1000	0.1458	68.331	0.0	0.1253	60.304	0.27230	1	0.5000
W1010	0.1294	68.331	0.0	0.11321	80.099	0.10504	1	0.5000
W1040	20.272	61.1743	0.0	1.6278	39.849	0.31144	1	0.5000
W1050	0.1964	66	0.0	0.54572	345.89	0.29530	1	0.5
W520	0.5432	92.532	0.0	0.42315	60.121	0.23650	1	0.5
W530	1.2222	92.532	0.0	0.92922	88.414	0.34808	1	0.5
W540	0.472	66	0.0	0.65115	138.48	0.68559	1	0.5
W550	0.5432	92.532	0.0	3.1593	89.84	0.44300	1	0.5
W260	0.5432	94.421	0.0	1.3939	102.05	0.35383	1	0.5
W570	0.816	96.885	0.0	2.8	174.69	0.75110	1	0.5
W580	0.5432	92.532	0.0	1.8204	116.46	0.28501	1	0.5
W590	0.5432	93.802	0.0	2.9157	124.93	0.54392	1	0.5
W600	1.2086	96.965	0.0	0.64102	92.767	0.36002	1	0.5
W610	0.5352	97.234	0.0	2.2193	141.25	0.72187	1	0.5
W620	0.5076	92.826	0.0	1.3338	127.79	0.78438	1	0.5
W630	0.5432	97.208	0.0	0.80972	76.951	0.22689	1	0.5
W640	2.334	68.331	0.0	1.4139	138.78	0.61288	1	0.5
W650	0.5432	62.946	0.0	0.17336	37.838	0.0169690	1	0.5
W660	0.5432	62.946	0.0	0.64138	94.004	0.10105	1	0.5
W670	0.7986	92.532	0.0	0.094735	22.522	.000663181	1	0.4802
W680	0.5432	92.532	0.0	0.86797	84.578	0.43717	1	0.5
069M	0.5432	66	0.0	0.68016	97.427	0.26251	1	0.5

	SCS Cur	SCS Curve Number Loss	S	Clark Unit Hydrograph Transform	raph Transform			
Sub-basin	Initial Abstraction (mm)	Curve	Impervious (%)	Time of Concentration (HR)	Storage Coefficient (HR)	Initial Discharge (M3/S)	<b>Recession</b> <b>Constant</b>	Ratio to Peak
W700	0.4816	66	0.0	1.0836	155.19	0.62682	1	0.5
W710	0.3454	66	0.0	0.21805	45.486	0.18063	1	0.5
W720	0.4316	66	0.0	0.38721	84.853	0.35507	1	0.5
W730	1.2222	94.809	0.0	2.1907	139.43	0.65987	Τ	0.5
W740	0.5106	64.992	0.0	0.24921	79.963	0.21583	1	0.5
W750	0.5432	62.946	0.0	0.42364	62.246	0.0578862	1	0.5
M760	0.3502	66	0.0	0.63613	90.826	0.17837	τ	0.5
W770	0.3946	66	0.0	0.37577	120.98	0.58995	1	0.5
W780	0.5432	66	0.0	0.09213	66.172	0.0154847	1	0.5
W790	0.5368	66	0.0	1.9798	189.93	0.55494	1	0.5
W800	1.1276	65.607	0.0	0.70668	102.24	0.26565	1	0.5
W810	0.5602	91.079	0.0	1.1138	159.5	0.95204	1	0.5
W820	1.2222	62.946	0.0	0.4252	93.245	0.30553	1	0.5
W830	1.1412	65.211	0.0	0.34601	75.888	0.25639	1	0.5
W840	2.75	42.821	0.0	0.2935	95.942	0.0912979	1	0.5
W850	0.4842	66	0.0	0.71624	69.124	0.36562	1	0.5
W860	1.1498	67.648	0.0	0.10129	74.201	0.28254	1	0.5
W870	0.7656	88.905	0.0	0.3824	125.18	0.52965	1	0.5
W880	0.5432	66	0.0	0.54271	116.51	0.25141	1	0.5
W890	0.7986	66	0.0	2.5252	105.07	0.27817	1	0.5
006M	0.9754	66	0.0	0.22251	158.37	0.79070	1	0.5
W910	0.403	66	0.0	0.14794	47.649	0.32253	1	0.5
W920	0.2732	66	0.0	0.07321	15.632	0.24728	1	0.2222
W930	0.3444	66	0.0	0.25243	287.16	0.19742	1	0.5
W940	0.5432	66	0.0	0.14816	163.49	0.37988	1	0.5

	no sos cni	SCS Curve Number Loss		Clark Unit Hydrograph Transform	raph Transform			
Sub-basin	Initial Abstraction (mm)	Curve Number	Impervious (%)	Time of Concentration (HR)	Storage Coefficient (HR)	Initial Discharge (M3/S)	<b>Recession</b> <b>Constant</b>	Ratio to Peak
W950	0.4394	66	0.0	0.15159	32.425	0.0517649	1	0.4917
096M	0.3936	66	0.0	0.14121	100.67	0.48522	1	0.5
W970	0.6004	66	0.0	0.11528	24.538	0.37268	1	0.3333
086M	0.8932	66	0.0	2.1041	456.64	1.2000	1	0.5

# Annex 10. Inagauan Model Reach Parameters

Table A-10.1 Inagauan Model Reach Parameters

Reach			Muskingum Cunge Channel Routing	l Routing			
Number	Time Step Method	Length (m)	Slope	Manning's n	Shape	Width	Side Slope
R1060	Automatic Fixed Interval	8193.6	0.0046050	0.0669292	Trapezoid	40	1
R130	Automatic Fixed Interval	56.569	0.0141509	0.5452	Trapezoid	40	1
R140	Automatic Fixed Interval	1393.3	0.0067208	0.36539	Trapezoid	40	1
R150	Automatic Fixed Interval	5701.9	0.0133876	0.54531	Trapezoid	40	1
R160	Automatic Fixed Interval	3543.3	0.0141264	0.2448	Trapezoid	40	1
R190	Automatic Fixed Interval	2426.2	0.0077829	0.35627	Trapezoid	40	1
R220	Automatic Fixed Interval	3349.1	0.0091600	0.16238	Trapezoid	40	1
R230	Automatic Fixed Interval	2646.3	0.0061734	0.24238	Trapezoid	40	1
R240	Automatic Fixed Interval	2658.4	0.0065554	0.15835	Trapezoid	40	1
R250	Automatic Fixed Interval	1400.5	0.0110834	0.16076	Trapezoid	40	1
R260	Automatic Fixed Interval	118.28	0.0273725	0.15836	Trapezoid	40	1
R280	Automatic Fixed Interval	2263.1	0.0027761	0.23221	Trapezoid	40	1
R310	Automatic Fixed Interval	3547.5	0.0048115	0.23274	Trapezoid	40	1
R320	Automatic Fixed Interval	1758.5	0.0086590	0.10772	Trapezoid	40	1

Reach			Muskingum Cunge Channel Routing	el Routing			
Number	Time Step Method	Length (m)	Slope	Manning's n	Shape	Width	Side Slope
R330	Automatic Fixed Interval	5530.8	0.0156066	0.16029	Trapezoid	40	1
R350	Automatic Fixed Interval	3248.8	0.0051373	0.23636	Trapezoid	40	1
R410	Automatic Fixed Interval	1738.2	0.0017449	0.10244	Trapezoid	40	1
R420	Automatic Fixed Interval	7732.6	0.0042001	0.15442	Trapezoid	40	1
R440	Automatic Fixed Interval	69:506	0.0042001	0.10453	Trapezoid	40	1
R450	Automatic Fixed Interval	4671.6	0.0305024	0.0696889	Trapezoid	40	1
R460	Automatic Fixed Interval	4474.2	0.0016455	0.0696889	Trapezoid	40	1
R500	Automatic Fixed Interval	1767.8	0.0046050	0.0696889	Trapezoid	40	1
R510	Automatic Fixed Interval	9161.4	0.0046050	0.04	Trapezoid	40	1
R70	Automatic Fixed Interval	4911.4	0.0201613	0.54495	Trapezoid	40	1
R80	Automatic Fixed Interval	7061.7	0.0330236	0.36499	Trapezoid	40	1
R90	Automatic Fixed Interval	683.55	0.0141509	0.54533	Trapezoid	40	1

# Annex 11. Inagauan Field Validation Data

Table A-11.1 Inagauan Field Validation Data

Point	Validation (	Coordinates	Model	Validation	Error	Event/Date	Rain Return /
Number	Lat	Long	Var (m)	Points (m)	LITOI	Lvent/ Date	Scenario
1	12.2677593	121.139577	0.052	0.00	-0.05		25-Year
2	12.26808239	121.1359749	0.096	0.00	-0.10		25-Year
3	12.26811462	121.1353353	0.042	0.00	-0.04		25-Year
4	12.26817205	121.1348946	0.056	0.00	-0.06		25-Year
5	12.2683501	121.1396667	0.178	0.00	-0.18		25-Year
6	12.26855913	121.139096	0.067	0.00	-0.07		25-Year
7	12.26867158	121.1358332	0.056	0.00	-0.06		25-Year
8	12.2687375	121.1369316	0.097	0.00	-0.10		25-Year
9	12.26890997	121.1370532	0.06	0.00	-0.06		25-Year
10	12.26898759	121.136709	0.072	0.00	-0.07		25-Year
11	12.26906449	121.1348371	0.078	0.00	-0.08		25-Year
12	12.26918658	121.1388668	0.119	0.00	-0.12		25-Year
13	12.26918873	121.1377438	0.138	0.00	-0.14		25-Year
14	12.26942876	121.1364042	0.162	0.00	-0.16		25-Year
15	12.26947415	121.1384567	0.247	0.00	-0.25		25-Year
16	12.26948678	121.1375942	0.278	0.00	-0.28		25-Year
17	12.26983985	121.1342523	0.202	0.00	-0.20		25-Year
18	12.26990337	121.1332585	0.074	0.00	-0.07		25-Year
19	12.27053581	121.1343376	0.293	0.00	-0.29		25-Year
20	12.27085968	121.1369329	0.081	0.30	0.22		25-Year
21	12.27101084	121.1333797	0.036	0.00	-0.04	Ruby / Dec. 2014	25-Year
22	12.27107064	121.1403548	0.035	0.60	0.57		25-Year
23	12.27104627	121.1365413	0.268	0.60	0.33	Ruby / Dec. 2014	25-Year
24	12.27130712	121.1420051	0.056	0.60	0.54	Ruby / Dec. 2014	25-Year
25	12.27148638	121.1399971	0.067	0.90	0.83	Ruby / Dec. 2014	25-Year
26	12.27155569	121.140664	0.032	0.60	0.57	Ruby / Dec. 2014	25-Year
27	12.27181344	121.1391873	0.03	0.60	0.57	Ruby / Dec. 2014	25-Year
28	12.27181687	121.1327829	0.061	0.00	-0.06	Ruby / Dec. 2014	25-Year
29	12.27199171	121.1365596	0.094	0.60	0.51		25-Year
30	12.27237603	121.1318774	0.077	0.00	-0.08	Ruby / Dec. 2014	25-Year
31	12.27389698	121.1320439	0.03	0.50	0.47		25-Year
32	12.27420119	121.1246051	0.216	1.00	0.78	Ruby / Dec. 2014	25-Year

Point	Validation (	Coordinates	Model	Validation	Error	Event/Date	Rain Return /
Number	Lat	Long	Var (m)	Points (m)	EIIOI	Event/Date	Scenario
33	12.280456	121.140142	0.03	0.40	0.37	Ruby / Dec. 2014	25-Year
34	12.280598	121.140319	0.03	0.35	0.32	Ondoy / Sept. 2009	25-Year
35	12.2815	121.1406	0.199	0.24	0.04	Yolanda / Nov. 2013	25-Year
36	12.2824	121.141125	0.03	0.42	0.39	Glenda / July, 2014	25-Year
37	12.2826	121.1413	0.031	0.77	0.74	Nona / Dec. 2015	25-Year
38	12.28291	121.14136	0.268	0.50	0.23	Yolanda / Nov. 2013	25-Year
39	12.283027	121.142358	0.03	0.30	0.27	Yolanda / Nov. 2013	25-Year
40	12.28324598	121.1163547	0.03	0.30	0.27	Yolanda / Nov. 2013	25-Year
41	12.28343378	121.1168896	0.03	0.00	-0.03	Ruby / Dec. 2014	25-Year
42	12.28369153	121.1183253	0.03	0.00	-0.03		25-Year
43	12.28385491	121.1165601	0.03	0.30	0.27		25-Year
44	12.283971	121.115485	0.037	0.30	0.26	Ruby / Dec. 2014	25-Year
45	12.28404456	121.1149475	0.03	0.00	-0.03	Ruby / Dec. 2014	25-Year
46	12.2841044	121.1177292	0.056	0.30	0.244		25-Year
47	12.28436415	121.1161775	0.03	0.60	0.57	Ruby / Dec. 2014	25-Year
48	12.28454404	121.1174499	0.034	0.00	-0.034	Ruby / Dec. 2014	25-Year
49	12.28454459	121.1145975	0.075	0.00	-0.075		25-Year
50	12.28473556	121.1142957	0.054	0.00	-0.054		25-Year
51	12.28501068	121.1148755	0.038	0.00	-0.038		25-Year
52	12.28521985	121.1413016	0.03	0.90	0.87		25-Year
53	12.2852526	121.1149556	0.044	0.00	-0.044	Frank / June, 2008	25-Year
54	12.28541714	121.1146135	0.03	0.60	0.57		25-Year
55	12.28570751	121.1145189	0.049	0.70	0.651	Ruby / Dec. 2014	25-Year
56	12.28584209	121.1142662	0.031	1.00	0.969	Ruby / Dec. 2014	25-Year
57	12.28588312	121.1161351	0.048	0.30	0.252	Ruby / Dec. 2014	25-Year
58	12.28590879	121.1146586	0.03	0.90	0.87	Ruby / Dec. 2014	25-Year
59	12.28605087	121.1137754	0.054	1.00	0.946	Ruby / Dec. 2014	25-Year

Point	Validation (	Coordinates	Model	Validation	Funan	Frent/Data	Rain
Number	Lat	Long	Var (m)	Points (m)	Error	Event/Date	Return / Scenario
60	12.28619179	121.1143164	0.037	1.00	0.963	Ruby / Dec. 2014	25-Year
61	12.286413	121.113646	0.03	0.70	0.67	Ruby / Dec. 2014	25-Year
62	12.28662187	121.1137243	0.03	1.00	0.97	Yolanda / Nov. 2013	25-Year
63	12.28713632	121.14006	0.03	0.00	-0.03	Ruby / Dec. 2014	25-Year
64	12.28713758	121.1152344	0.03	0.00	-0.03		25-Year
65	12.2874	121.1118	0.03	0.40	0.37		25-Year
66	12.287451	121.106806	0.123	1.30	1.177	Yolanda / Nov. 2013	25-Year
67	12.287451	121.106806	0.123	0.56	0.437	Yolanda / Nov. 2013	25-Year
68	12.287712	121.110859	0.033	0.10	0.067	Ruby / Dec. 2014	25-Year
69	12.2877	121.1076	0.149	0.77	0.621	Ruby / Dec. 2014	25-Year
70	12.288088	121.107628	0.343	1.00	0.657	Glenda / July, 2014	25-Year
71	12.288137	121.109273	0.259	0.85	0.591	Ruby / Dec. 2014	25-Year
72	12.2881785	121.1140012	0.03	0.00	-0.03	Aug. 2015	25-Year
73	12.288228	121.108309	0.363	0.95	0.587		25-Year
74	12.28826257	121.1078895	0.286	1.30	1.014	Yolanda / Nov. 2013	25-Year
75	12.28846631	121.1084624	0.296	1.30	1.004	Ruby / Dec. 2014	25-Year
76	12.288522	121.1095252	0.111	1.30	1.189	Ruby / Dec. 2014	25-Year
77	12.2888	121.1101	0.03	0.7	0.67	Ruby / Dec. 2014	25-Year
78	12.29604087	121.1485593	1.32	0.9	-0.42	Undang / 1984	25-Year
79	12.29769703	121.1368933	0.03	0	-0.03	Mario / Sept. 2014	25-Year
80	12.29790457	121.1465242	0.03	0	-0.03		25-Year
81	12.30016443	121.1466252	0.031	0	-0.031		25-Year
82	12.30080732	121.1466362	0.03	0	-0.03		25-Year
83	12.30216259	121.1493133	0.037	0	-0.037		25-Year
84	12.30300969	121.151692	0.858	0.9	0.042		25-Year
85	12.30471438	121.1514492	0.03	0	-0.03	Mario / Sept. 2014	25-Year
86	12.30694932	121.1511363	0.03	0	-0.03		25-Year
87	12.30704819	121.138254	0.03	0	-0.03		25-Year
88	12.30837591	121.1497059	0.03	0	-0.03		25-Year

Point	Validation (	Coordinates	Model	Validation	Error	Event/Date	Rain Return /
Number	Lat	Long	Var (m)	Points (m)	EIIOI	Event/ Date	Scenario
89	12.30836425	121.1429956	0.03	0	-0.03		25-Year
90	12.3083822	121.1356902	0.03	0	-0.03		25-Year
91	12.30873982	121.1426897	0.036	0	-0.036		25-Year
92	12.30887093	121.1437328	0.031	0	-0.031		25-Year
93	12.30920466	121.1340459	0.03	0	-0.03		25-Year
94	12.30931966	121.1494876	0.031	0	-0.031		25-Year
95	12.30972792	121.1433471	0.031	0	-0.031		25-Year
96	12.30992565	121.147753	0.055	0	-0.055		25-Year
97	12.31063205	121.1464339	0.03	0	-0.03		25-Year
98	12.31175295	121.144631	0.03	0	-0.03		25-Year
99	12.31194925	121.1460993	0.03	0	-0.03		25-Year
100	12.31682036	121.1464441	0.03	0	-0.03		25-Year
101	12.31818137	121.1463399	0.03	0	-0.03		25-Year
102	12.31878966	121.1488838	0.03	0	-0.03		25-Year
103	12.31892575	121.1478403	0.03	0	-0.03		25-Year
104	12.31937536	121.1462643	0.03	0	-0.03		25-Year
105	12.31981115	121.1493826	0.03	0	-0.03		25-Year
106	12.3212637	121.1460699	0.031	0	-0.031		25-Year
107	12.32141748	121.1501692	0.03	0	-0.03		25-Year
108	12.3231649	121.1455786	0.03	0	-0.03		25-Year
109	12.32450852	121.1466509	0.03	0	-0.03		25-Year
110	12.32641608	121.1455303	0.031	0	-0.031		25-Year
111	12.32794347	121.1446483	0.03	0	-0.03		25-Year
112	12.33484516	121.1403739	0.631	0	-0.631		25-Year
113	12.33541404	121.1384496	0.03	0	-0.03		25-Year
114	12.33617703	121.1377899	0.066	0	-0.066		25-Year
115	12.337552	121.17735	2.251	0.6	-1.651		25-Year
116	12.3375274	121.137022	0.03	0	-0.03	Yolanda / Nov. 2013	25-Year
117	12.337885	121.17717	1.372	0.5	-0.872		25-Year
118	12.3382249	121.1352883	0.03	0	-0.03	Undang / 1984	25-Year
119	12.33915982	121.1346933	0.078	0	-0.078		25-Year
120	12.339481	121.17681	0.03	0	-0.03		25-Year
121	12.33981626	121.1342148	0.03	0	-0.03		25-Year
122	12.34130204	121.1338624	0.035	0	-0.035		25-Year
123	12.34160143	121.155423	0.03	0	-0.03		25-Year
124	12.342589	121.133367	0.036	0	-0.036		25-Year
125	12.343543	121.17839	0.03	0	-0.03		25-Year
126	12.34345954	121.133481	0.146	0	-0.146		25-Year
127	12.344377	121.1331039	0.03	0	-0.03		25-Year
128	12.34475837	121.1326007	0.089	0	-0.089		25-Year

Point	Validation (	Coordinates	Model	Validation	Гииои	Frent/Data	Rain
Number	Lat	Long	Var (m)	Points (m)	Error	Event/Date	Return / Scenario
129	12.34692781	121.133061	0.288	0	-0.288		25-Year
130	12.34747038	121.1319442	0.607	0	-0.607		25-Year
131	12.34827659	121.1310979	0.467	0	-0.467		25-Year
132	12.35144908	121.1298895	0.052	0	-0.052		25-Year
133	12.3522015	121.1301683	0.284	0	-0.284		25-Year
134	12.35250501	121.1270408	0.03	0	-0.03		25-Year
135	12.35412972	121.1264919	0.031	0	-0.031		25-Year
136	12.35536653	121.1252275	0.03	0	-0.03		25-Year

## Annex 12. Phil-LiDAR 1 UPLB Team Composition

### **Project Leader**

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Ms. Sandra Samantela (CHE, UPLB)
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