

HAZARD MAPPING OF THE PHILIPPINES USING LIDAR (PHIL-LIDAR I)

LiDAR Surveys and Flood Mapping of Palandok River



University of the Philippines Training Center
for Applied Geodesy and Photogrammetry
Ateneo de Zamboanga University



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Hazard Mapping of the Philippines Using LIDAR (Phil-LIDAR 1)

LIDAR SURVEYS AND FLOOD MAPPING OF PALANDOK RIVER



University of the Philippines Training Center for Applied Geodesy and Photogrammetry
Ateneo de Zamboanga University

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CHAPTER 1: INTRODUCTION

1.1 Background of the Phil-LiDAR 1 Program

The University of the Philippines Training Center for Applied Geodesy and Photogrammetry (UP TCAGP) launched a research program entitled “Nationwide Hazard Mapping using LiDAR” or Phil-LiDAR 1 in 2014, supported by the Department of Science and Technology (DOST) Grant-in-Aid (GiA) Program. The program was primarily aimed at acquiring a national elevation and resource dataset at sufficient resolution to produce information necessary to support the different phases of disaster management. Particularly, it targeted to operationalize the development of flood hazard models that would produce updated and detailed flood hazard maps for the major river systems in the country.

The program also aimed to produce an up-to-date and detailed national elevation dataset suitable for 1:5,000 scale mapping, with 50 cm and 20 cm horizontal and vertical accuracies, respectively. These accuracies were achieved through the use of the state-of-the-art Light Detection and Ranging (LiDAR) airborne technology procured by the project through DOST. The methods applied in this report are thoroughly described in a separate publication titled *Flood Mapping of Rivers in the Philippines Using Airborne LiDAR: Methods* (Paringit et al., 2017) available separately.

The implementing partner university for the Phil-LiDAR 1 Program is the Ateneo de Zamboanga University (ADZU). ADZU is in charge of processing LiDAR data and conducting data validation reconnaissance, cross section, bathymetric survey, validation, river flow measurements, flood height and extent data gathering, flood modeling, and flood map generation for the 18 river basins in the Zamboanga Peninsula. The university is located in Zamboanga City in the province of Zamboanga Sibugay.

1.2 Overview of the Palandok River Basin

Overview

The Palandok River Basin, with a total land area of 227.04 sq. km., covers four (4) municipalities in Zamboanga del Norte, namely the municipalities of Bacungan, Godod, Kabalasan, Salug, and some parts of Bayog. According to the DENR River Basin Control Office (RBCO), the river basin has a drainage area of 203 km² and an estimated 152 cubic meter (MCM) annual run-off (RBCO, 2015).

Its main stem, Palandok River, one of the several major water bodies in Zamboanga del Norte, is part of the eighteen (18) river systems under the PHIL-LIDAR 1 Program partner HEI, Ateneo de Zamboanga University. Palandok River is one of the several major water bodies in Zamboanga del Norte. Its mouth is found between the municipalities of Salug and Leon B. Postigo, and thus, it is considered as the political boundary of the two. A total of 2,024 persons are residing in Brgy. Rizon in the Municipality of Bacungan, which is within the immediate vicinity of the river, according to the 2015 national census of PSA (Source: <http://www.islandsproperties.com/places/zambonor.htm>).

According to legends, the river is known as the reference point in tracing and location locating Zamboanga Peninsula. It was also known as a place where sailing boats and ships docked when trading and sailing along the Sulu Archipelago.



Figure 1. Up-streams of Palandok River

Scenic Sights and Destination

The municipality of Leon B. Postigo is gifted with several natural resources and sights, one of which is the impressive and preserved beauty of Palandok River. The upstream part of the river is possessed with currents which could be considered for outdoor activities like such as water rafting. Aside from this, the municipality is also known to the local tourist for its hot spring resort which is located in Sitio Mainit, just few kilometers away from the river.

Sunset view is also very popular in the province of Zamboanga del Norte. However, it is the municipality that is also widely known for its spectacular view of the sunset scene happening which can be seen in from the Sindangan Bay.



Figure 2. Sunset in Leon B. Postigo

Flood and other hazards

Up to date, the municipality of Leon B. Postigo has still not recorded any significant flood event which happened in the area. However, in 2010, the Mines and Geosciences Bureau (MGB) Region 9 has released the Geo-hazard Map of the municipality. It was indicated in their data that areas near the mouth of the river is are highly susceptible to flooding and the waters may reach more than 1 meter in height. The agency has also identified several areas of the municipality as which are prone to landslide and storm surge.

Moreover, just recently, on February 1, 2017, the Philippine Atmospheric, Geophysical and Astronomical Services Administration (PAGASA) issued a flood advisory for Polandok River and its tributaries due to the moderate to heavy rains brought by the presence of a trough of low pressure area affecting Mindanao as per NDRRMC report (Source: www.ndrrmc.gov.ph/attachments/article/3/ADVISORY_GFA_No.06-REGII,_No.05-REG_III,_No.01-REG_IX,_No.02-REG_X,_No.02-REG_XI,_No.02-CARAGA,_No.05-CAR,_No.01-ARMM.pdf)

In 2007, the Philippine Institute of Vulcanology and Seismology (PHILVOLCS) also identified the shorelines of the municipality as susceptible to Tsunami and have further indicated that at an 8 magnitude earthquake in the Sulu Trench, the wave of the water could reach up to 8.43 meters.

Just recently, The Polandok River Basin covers four (4) municipalities in Zamboanga del Norte; namely, the municipalities of Bacungan, Godod, Kabalasan, Salug, and some parts of Bayog. The DENR River Basin Control Office (RBCO) states that the Polandok River Basin has a drainage are of 203 km² and an estimated 152 cubic meter (MCM) annual run-off (RBCO, 2015).

Its main stem, Polandok River, is part of the eighteen (18) river systems under the PHIL-LIDAR 1 Program partner HEI, Ateneo de Zamboanga University.

According to the 2015 national census of PSA, a total of 2,024 persons are residing in Brgy. Rizon in the Municipality of Bacungan, which is within the immediate vicinity of the river. The economy of the province Zamboanga del Norte largely rests on agriculture particularly fishing, and mineral extraction (Source:). On February 1, 2017, the Philippine Atmospheric, Geophysical and Astronomical Services Administration (PAGASA) issued a flood advisory for Polandok River and its tributaries due to the moderate to heavy rains brought by the presence of a trough of low pressure area affecting Mindanao as per NDRRMC report (Source: www.ndrrmc.gov.ph/attachments/article/3/ADVISORY_GFA_No.06-REGII,_No.05-REG_III,_No.01-REG_IX,_No.02-REG_X,_No.02-REG_XI,_No.02-CARAGA,_No.05-CAR,_No.01-ARMM.pdf)

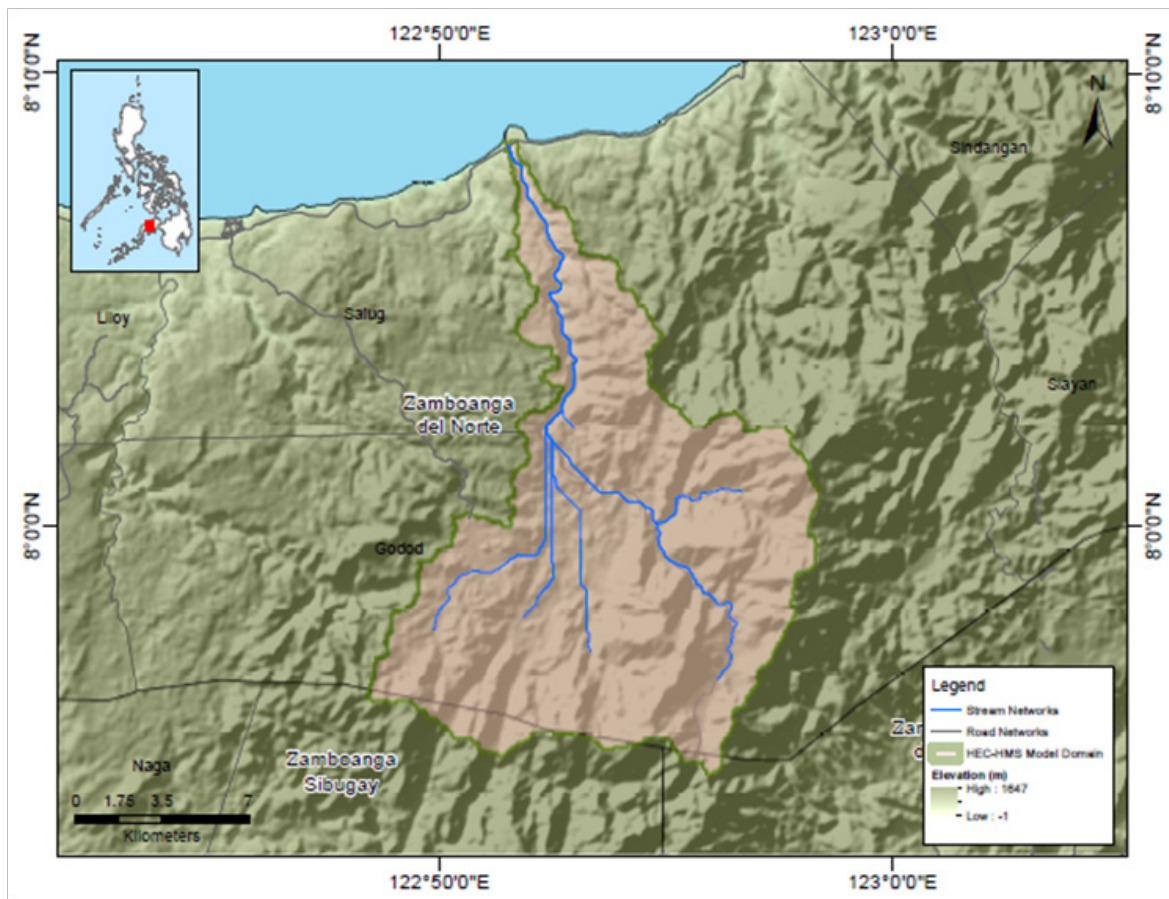


Figure 3. Map of Palandok River Basin (in brown)

CHAPTER 2: LIDAR ACQUISITION IN PALANDOK FLOODPLAIN

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The methods applied in this chapter were based on the DREAM methods manual (Sarmiento et al., 2014) and further enhanced and updated in Paringit et al. (2017).

2.1 Flight Plans

Plans were made to acquire LiDAR data within the delineated priority area for Palandok Floodplain in Zamboanga del Norte. These missions were planned for 12 lines that run for at most four and a half (4.5) hours including take-off, landing and turning time. The flight planning parameters for the LiDAR system is found in Table 1. Figure 4 shows the flight plan for Palandok Floodplain survey.

Table 1. Flight planning parameters for Pegasus LiDAR System.

Block Name	Flying Height (AGL)	Overlap (%)	Field of View (θ)	Pulse Repetition Frequency (PRF) (kHz)	Scan Frequency (Hz)	Average Speed (kts)	Average Turn Time (Minutes)
BLK73B	1000	20	50	200	30	130	5

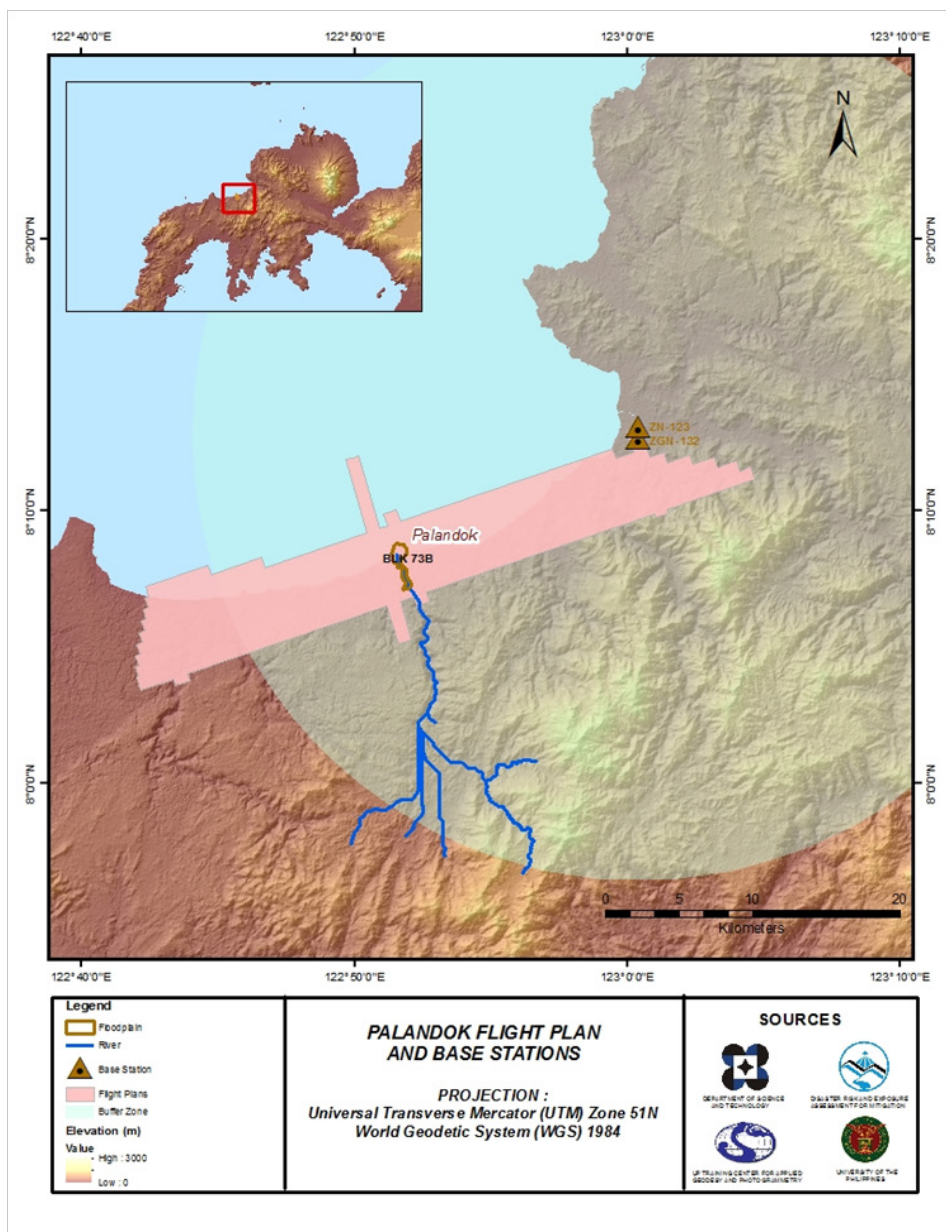


Figure 4. Flight plan and base stations used for Palandok Floodplain.

2.2 Ground Base Stations

The project team was able to recover one (1) NAMRIA ground control point ZGN-132 which is of second (2nd) order accuracy. The project team also recovered one (1) NAMRIA benchmark, ZN-123. The certification for the NAMRIA reference point is found in Annex 2 while the baseline processing report for the established control point is found in Annex 3. These were used as base stations during flight operations for the entire duration of the survey (October 8 to November 11, 2014). Base stations were observed using dual frequency GPS receivers: TRIMBLE SPS 852 and TRIMBLE SPS 882. Flight plans and location of base stations used during the aerial LiDAR acquisition in Palandok Floodplain are shown in Figure 4.

Figure 5 shows the recovered NAMRIA reference point within the area. In addition, Table 2 to Table 3 show the details about the following NAMRIA control stations, while Table 4 shows the list of lists all ground control points occupied during the acquisition together with the corresponding dates of utilization.



Figure 5. GPS set-up over ZGN-132 at Barangay Mandih, Zambonaga del Norte (a) and NAMRIA reference point ZGN-132 (b) as recovered by the field team.

Table 2. Details of the recovered NAMRIA horizontal control point ZGN-132 used as base station for the LiDAR acquisition.

Station Name	ZGN-132	
Order of Accuracy	2nd	
Relative Error (horizontal positioning)	1 in 50,000	
Geographic Coordinates, Philippine Reference of 1992 Datum (PRS 92)	Latitude Longitude Ellipsoidal Height	8° 12' 44.29460" North 123° 0' 19.12667" East 11.502 meters
Grid Coordinates, Philippine Transverse Mercator Zone 3 (PTM Zone 5 PRS 92)	Easting Northing	500,585.389 meters 908,029.029 meters
Geographic Coordinates, World Geodetic System 1984 Datum (WGS 84)	Latitude Longitude Ellipsoidal Height	8° 12' 40.63408" North 123° 0' 24.56923" East 75.58 meters
Grid Coordinates, Universal Transverse Mercator Zone 51 North (UTM 51N PRS 1992)	Easting Northing	500,751.70 meters 907,656.11 meters

Table 3. Details of the recovered NAMRIA horizontal control point ZN-123 used as base station for the LiDAR acquisition.

Station Name	ZN-123	
Order of Accuracy	1st	
Relative Error (horizontal positioning)	1 in 50,000	
Geographic Coordinates, Philippine Reference of 1992 Datum (PRS 92)	Latitude Longitude Ellipsoidal Height	8° 13' 8.18558" North 123° 00' 36053" East 10.101 meters
Geographic Coordinates, World Geodetic System 1984 Datum (WGS 84)	Latitude Longitude Ellipsoidal Height	8° 13' 4.52332" North 123° 00' 24.80249" East 74.166 meters
Grid Coordinates, Universal Transverse Mercator Zone 51 North (UTM 51N PRS 1992)	Easting Northing	500,758.82 meters 908,389.73 meters

Table 4. Ground Control Points used during LiDAR data acquisition

Date Surveyed	Flight Number	Mission Name	Ground Control Points
November 3, 2014	2157P	1BLK70C307A	ZGN-132, ZN-123

2.3 Flight Missions

One (1) mission was conducted to complete the LiDAR data acquisition in Palandok Floodplain, for a total of three hours and fifty-nine minutes (3+59) of flying time for RP-9122. The mission was acquired using the Pegasus LiDAR system. Table 5 shows the total area of actual coverage and the corresponding flying hours of the mission while Table 6 presents the actual parameters used during the LiDAR data acquisition.

Table 5. Flight mission for LiDAR data acquisition in Palandok Floodplain.

Date Surveyed	Flight Number	Flight Plan Area (km ²)	Surveyed Area (km ²)	Area Surveyed within the Floodplain (km ²)	Area Surveyed Outside the Floodplain (km ²)	No. of Images (Frames)	Flying Hours	
							Hr	Min
November 3, 2014	2157P	221.53	228.68	1.65	227.03	587	3	59
TOTAL		221.53	228.68	1.65	227.03	587	3	59

Table 6. Actual parameters used during LiDAR data acquisition

Flight Number	Flying Height (AGL)	Overlap (%)	Field of View (θ)	Pulse Repetition Frequency (PRF) (kHz)	Scan Frequency (Hz)	Average Speed (kts)	Average Turn Time (Minutes)
2157P	1100	20	50	200	30	130	5

2.4 Survey Coverage

Palandok floodplain is located along the province of Zamboanga del Norte with majority of the floodplain situated within the municipality of Salug and Bacungan. The list of municipalities and cities surveyed, with at least one (1) square kilometer coverage, is shown in Table 7. The actual coverage of the LiDAR acquisition for Palandok Floodplain is presented in Figure 63.

Table 7. List of municipalities and cities surveyed during Palandok Floodplain LiDAR survey.

Province	Municipality/City	Area of Municipality/City (km ²)	Total Area Surveyed (km ²)	Percentage of Area Surveyed =(Total Area covered/ Area of Municipality)*100
Zamboanga del Norte	Salug	157.23	50.23	31.94%
	Sindangan	295.62	61.55	20.82%
	Liloy	123.94	23.48	18.95%
	Bacungan	372.86	34.93	9.37%
	Siayan	461.46	5.57	1.21%
Total		1411.11	175.76	12.46%

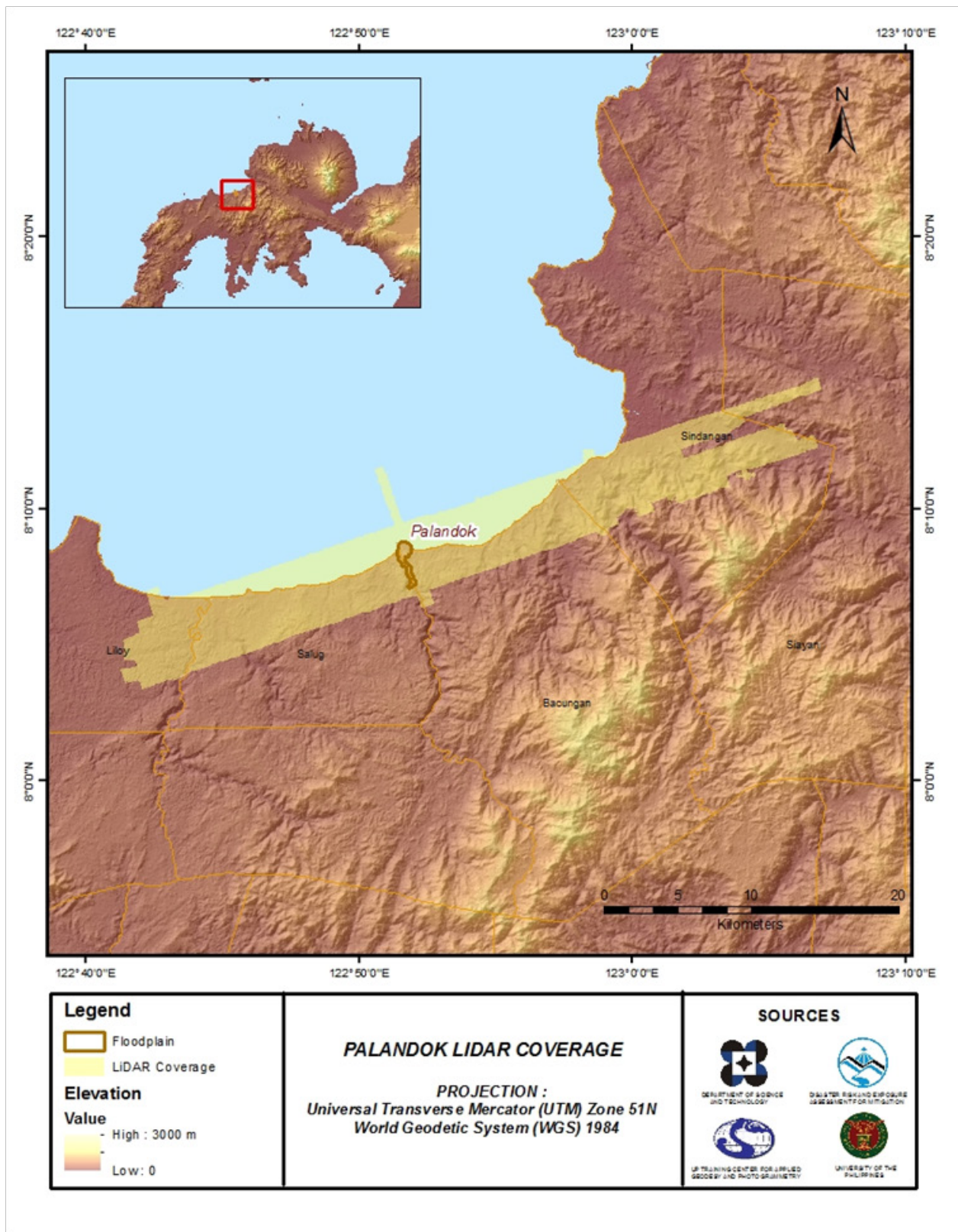


Figure 6. Actual LiDAR survey coverage for Palandok Floodplain.

CHAPTER 3: LIDAR DATA PROCESSING FOR BATU PALANDOK FLOODPLAIN

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The methods applied in this chapter were based on the DREAM methods manual (Ang, et al., 2014) and further enhanced and updated in Paringit, et al. (2017).

3.1 Overview of the LIDAR Data Pre-Processing

The data transmitted by the Data Acquisition Component were checked for completeness based on the list of raw files required to proceed with the pre-processing of the LiDAR data. Upon acceptance of the LiDAR field data, georeferencing of the flight trajectory was done to obtain the exact location of the LiDAR sensor when the laser was shot.

Point cloud georectification was performed to incorporate correct position and orientation for each point acquired. The georectified LiDAR point clouds were subject for quality checking to ensure that the required accuracies of the program, which are the minimum point density, vertical and horizontal accuracies, are met. The point clouds were then classified into various classes before generating Digital Elevation Models such as Digital Terrain Model and Digital Surface Model.

Using the elevation of points gathered in the field, the LiDAR-derived digital models were calibrated. Portions of the river that are barely penetrated by the LiDAR system were replaced by the actual river geometry measured from the field by the Data Validation and Bathymetry Component. LiDAR acquired temporally were then mosaicked to completely cover the target river systems in the Philippines. Orthorectification of images acquired simultaneously with the LiDAR data was done through the help of the georectified point clouds and the metadata containing the time the image was captured.

These processes are summarized in the flowchart shown in Figure 7.

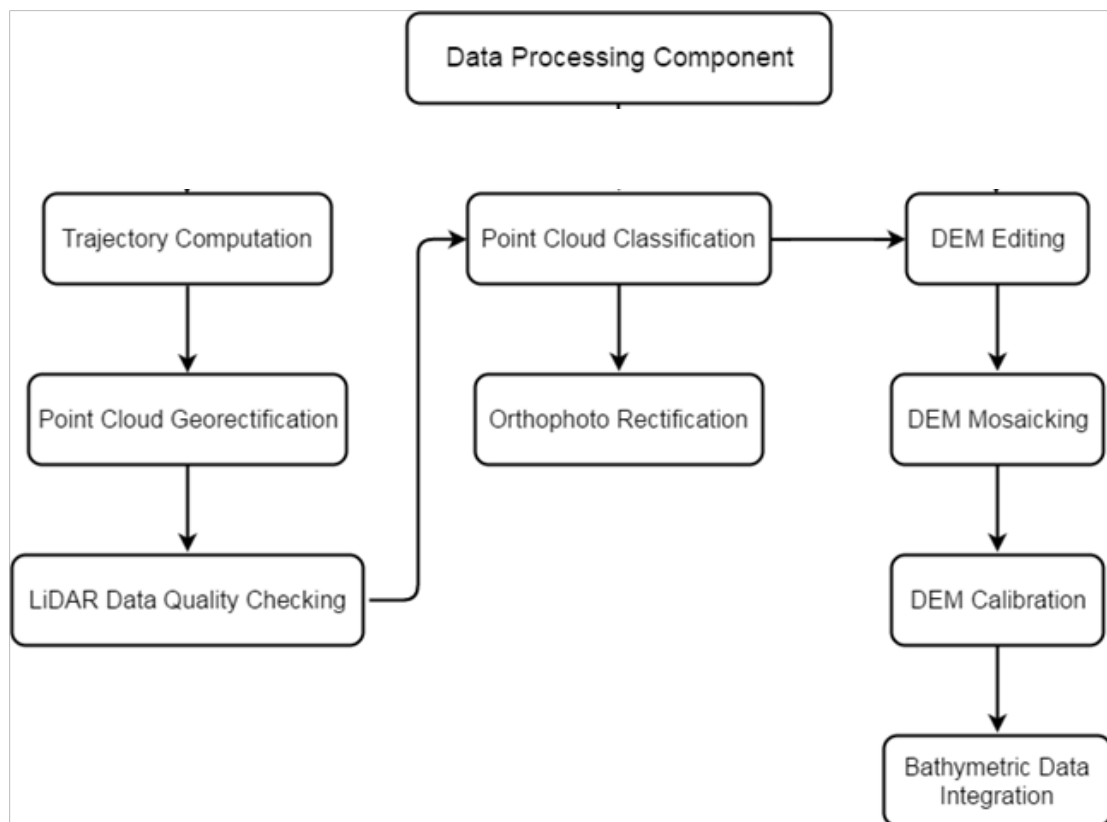


Figure 7. Schematic Diagram for Data Pre-Processing Component

3.2. Transmittal of Acquired LiDAR Data

Data transfer sheets for all the LiDAR missions for Palandok floodplain can be found in Annex 5. The mission flown during the survey conducted on November 2014 used the Airborne LiDAR Terrain Mapper (ALTM™ Optech Inc.) Pegasus system over Salug and Bacungan, Zamboanga del Norte. The Data Acquisition Component (DAC) transferred a total of 25.1 Gigabytes of Range data, 240 Megabytes of POS data, 35.5 Megabytes of GPS base station data, and 40.6 Gigabytes of raw image data to the data server on November 3, 2014. The Data Pre-processing Component (DPPC) verified the completeness of the transferred data. The whole dataset for Palandok was fully transferred on November 14, 2014, as indicated on the Data Transfer Sheets for Palandok Floodplain.

3.3 Trajectory Computation

The Smoothed Performance Metric parameters of the computed trajectory for flight 2157P, one of the Palandokflights, which is the North, East, and Down position RMSE values are shown in Figure 8. The x-axis corresponds to the time of flight, which is measured by the number of seconds from the midnight of the start of the GPS week, which on that week fell on November 3, 2014 00:00AM. The y-axis is the RMSE value for that particular position.

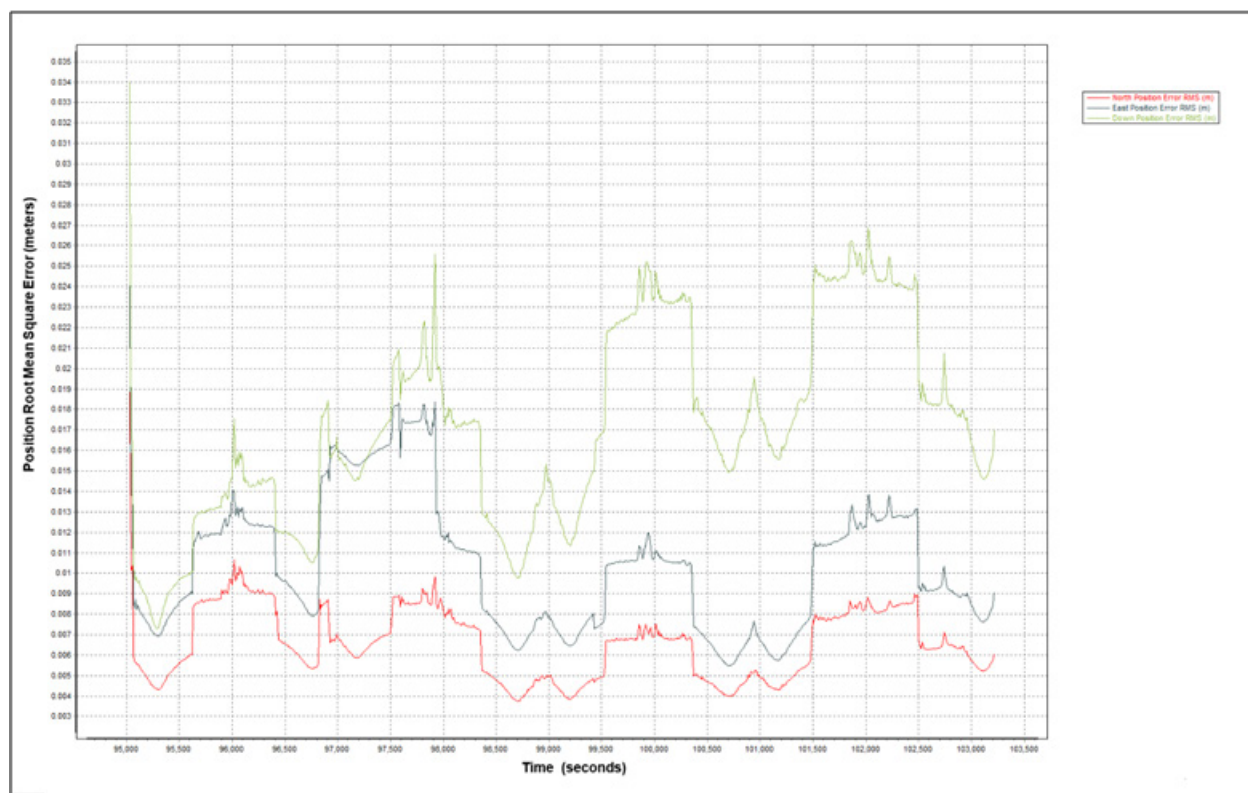


Figure 8. Smoothed Performance Metric Parameters of Palandok Flight 2157P.

The time of flight was from 95000 seconds to 103500 seconds, which corresponds to morning of November 3, 2014. The initial spike that is seen on the data corresponds to the time that the aircraft was getting into position to start the acquisition, and the POS system starts computing for the position and orientation of the aircraft. Redundant measurements from the POS system quickly minimized the RMSE value of the positions. The periodic increase in RMSE values from an otherwise smoothly curving RMSE values correspond to the turn-around period of the aircraft, when the aircraft makes a turn to start a new flight line. Figure 9 shows that the North position RMSE peaks at 1.10 centimeters, the East position RMSE peaks at 1.90 centimeters, and the Down position RMSE peaks at 2.70 centimeters, which are within the prescribed accuracies described in the methodology.

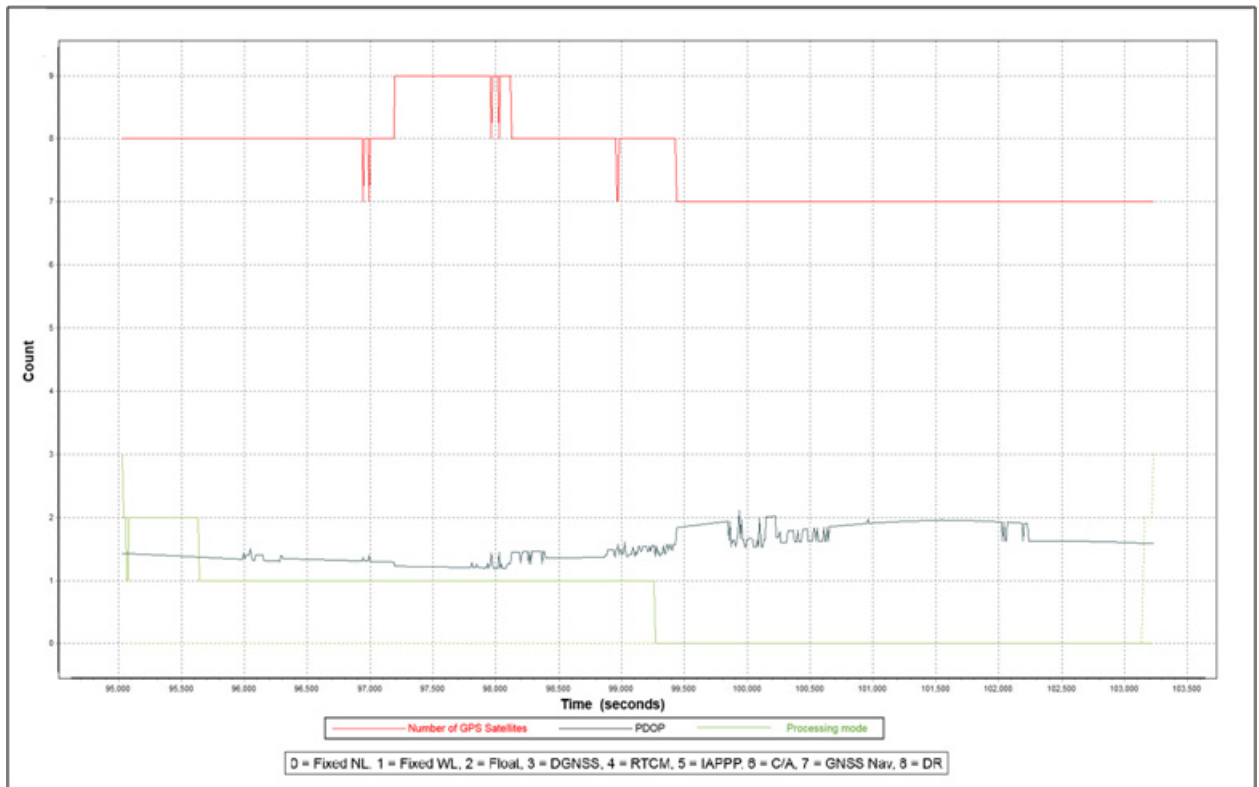


Figure 9. Solution Status Parameters of Palandok Flight 2157P.

The Solution Status parameters of flight 2157P, one of the Palandok flights, which are the number of GPS satellites, Positional Dilution of Precision (PDOP), and the GPS processing mode used, are shown in Figure 9. The graphs indicate that the number of satellites during the acquisition did not go down to 6. Majority of the time, the number of satellites tracked was between 7 and 9. The PDOP value also did not go above the value of 3, which indicates optimal GPS geometry. The processing mode stayed at the value of 0 for majority of the survey with some peaks up to 2 attributed to the turns performed by the aircraft. The value of 0 corresponds to a Fixed, Narrow-Lane mode, which is the optimum carrier-cycle integer ambiguity resolution technique available for POSPAC MMS. All of the parameters adhered to the accuracy requirements for optimal trajectory solutions, as indicated in the methodology. The computed best estimated trajectory for all Palandok flights is shown in Figure 101.

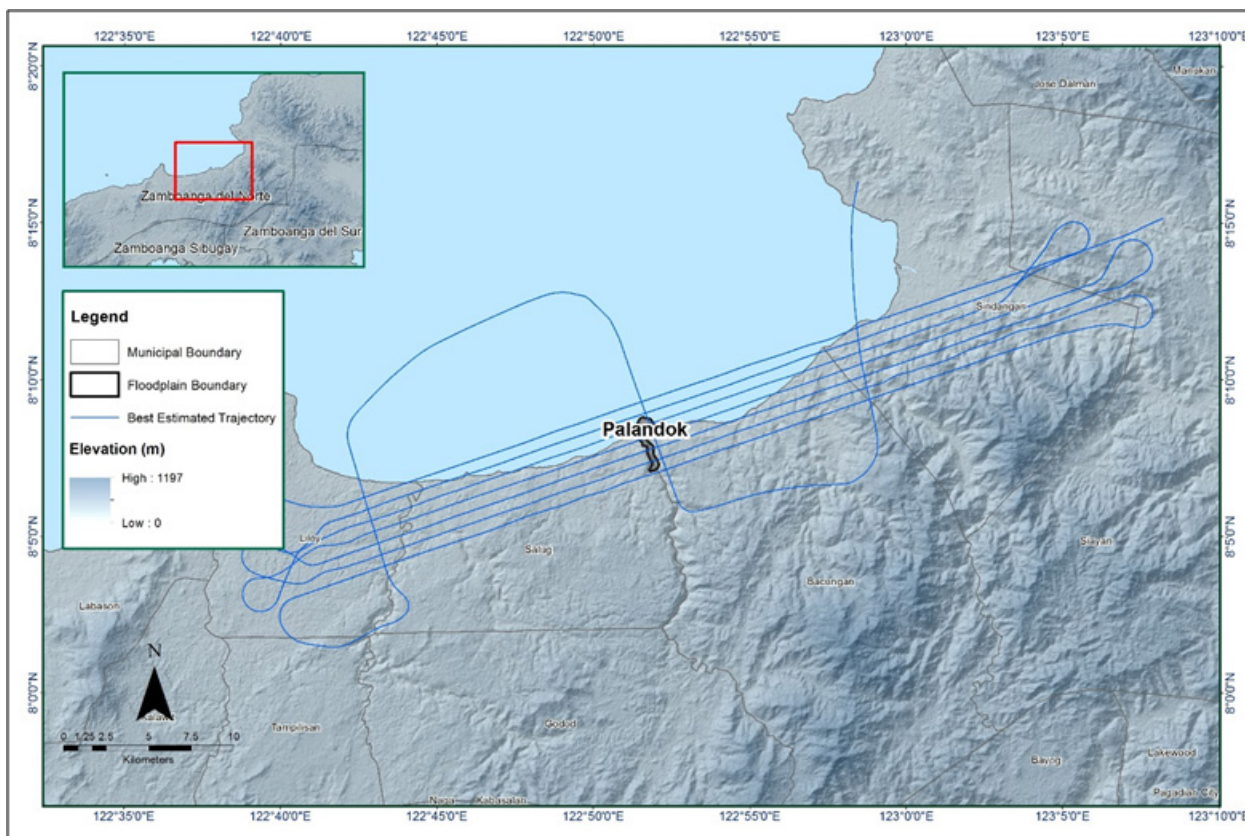


Figure 10. Best Estimated Trajectory for Palandok floodplain.

3.4 LiDAR Point Cloud Computation

The produced LAS data contains 10 flight lines, with each flight line containing two channels, since the Pegasus system contains two channels. The summary of the self-calibration results obtained from LiDAR processing in LiDAR Mapping Suite (LMS) software for all flights over Palandok floodplain are given in Table B-18.

Table 8. Self-Calibration Results values for Palandok flights.

Parameter	Computed Value
Boresight Correction stdev	(<0.001degrees) 0.000185
IMU Attitude Correction Roll and Pitch Corrections stdev	(<0.001degrees) 0.000500
GPS Position Z-correction stdev	(<0.01meters) 0.0089

The optimum accuracy is obtained for all Palandok flights based on the computed standard deviations of the corrections of the orientation parameters. Standard deviation values for individual blocks are available in the Annex 8.

3.5 LiDAR Data Quality Checking

The boundary of the processed LiDAR data on top of a SAR Elevation Data over Palandok Floodplain is shown in Figure B-511. The map shows gaps in the LiDAR coverage that are attributed to cloud coverage.

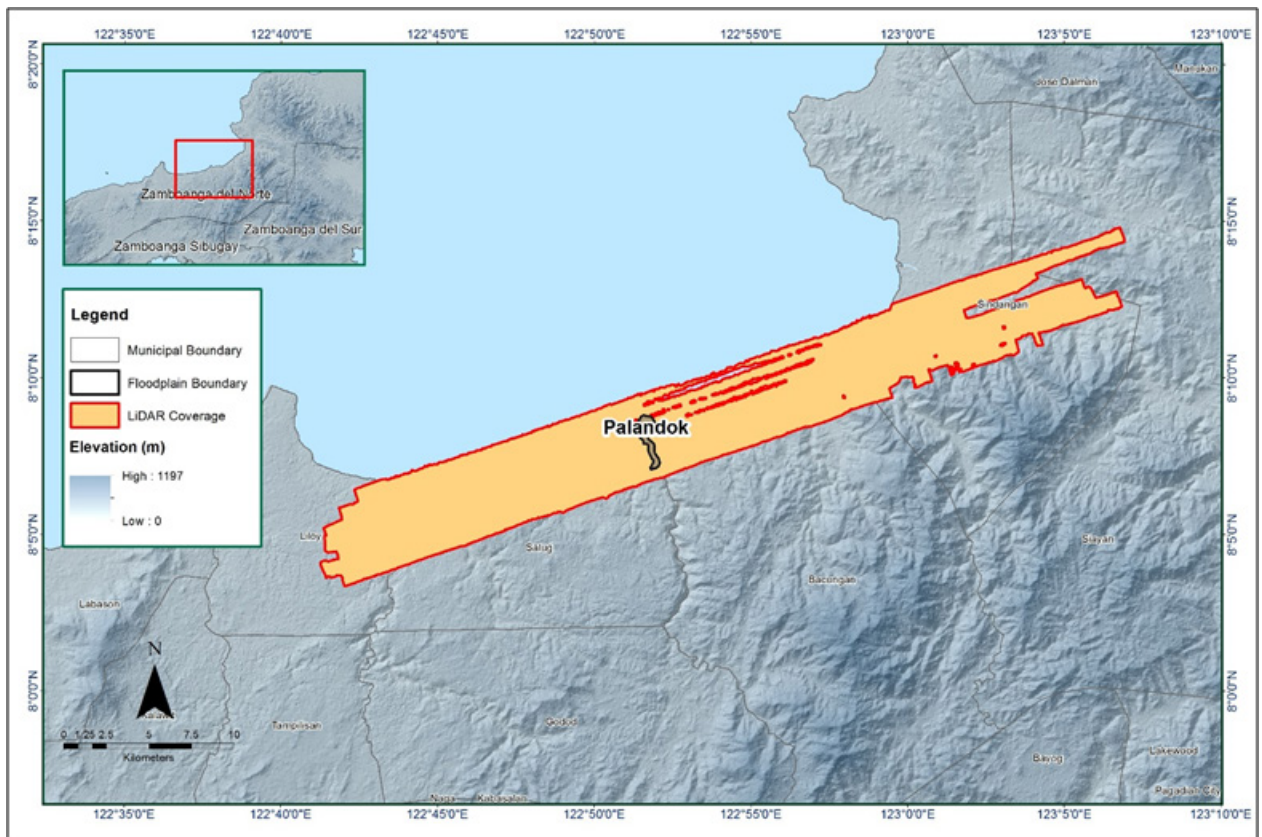


Figure 11. Boundary of the processed LiDAR data over Palandok Floodplain

The total area covered by the Palandok missions is 214.34 sq.km comprised of one (1) flight acquisition grouped and merged into one (1) block as shown in Table B-29.

Table 9. List of LiDAR blocks for Palandok floodplain.

LiDAR Blocks	Flight Numbers	Area (sq. km)
Dipolog_Bl73B	2157P	214.34
TOTAL		214.34 sq.km

The overlap data for the merged LiDAR blocks, showing the number of channels that pass through a particular location is shown in Figure B-612. Since the Pegasus system employs two channels, an average value of 2 (blue) for areas where there is limited overlap, and a value of 3 (yellow) or more (red) for areas with three or more overlapping flight lines are expected.

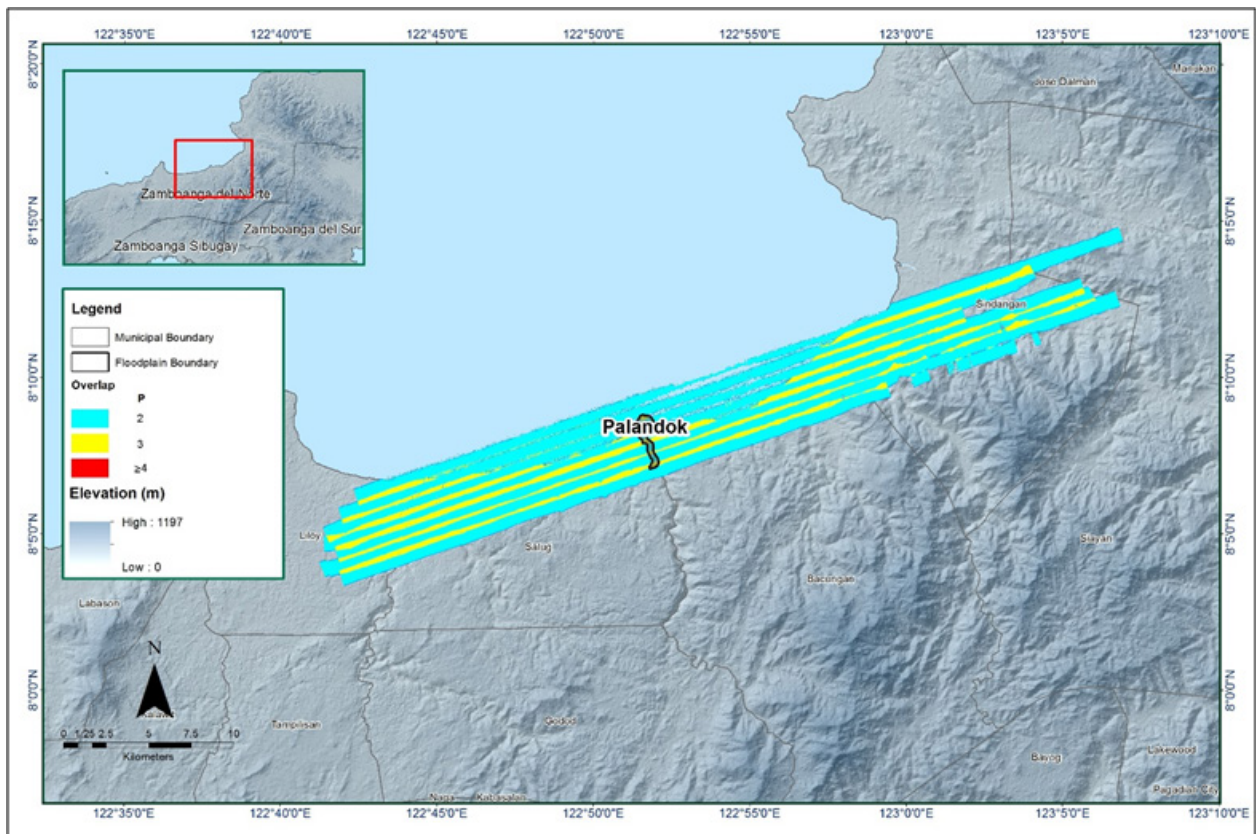


Figure 12. Image of data overlap for Palandok floodplain.

The overlap statistics per block for the Palandok Floodplain can be found in Annex B-1. One pixel corresponds to 25.0 square meters on the ground. For this area, the percent overlap computed between flight lines is 24.96.

The pulse density map for the merged LiDAR data, with the red parts showing the portions of the data that satisfy the 2 points per square meter criterion is shown in Figure B-713. It was determined that all LiDAR data for Palandok Floodplain satisfy the point density requirement, and the average density for the entire survey area is 3.15 points per square meter.

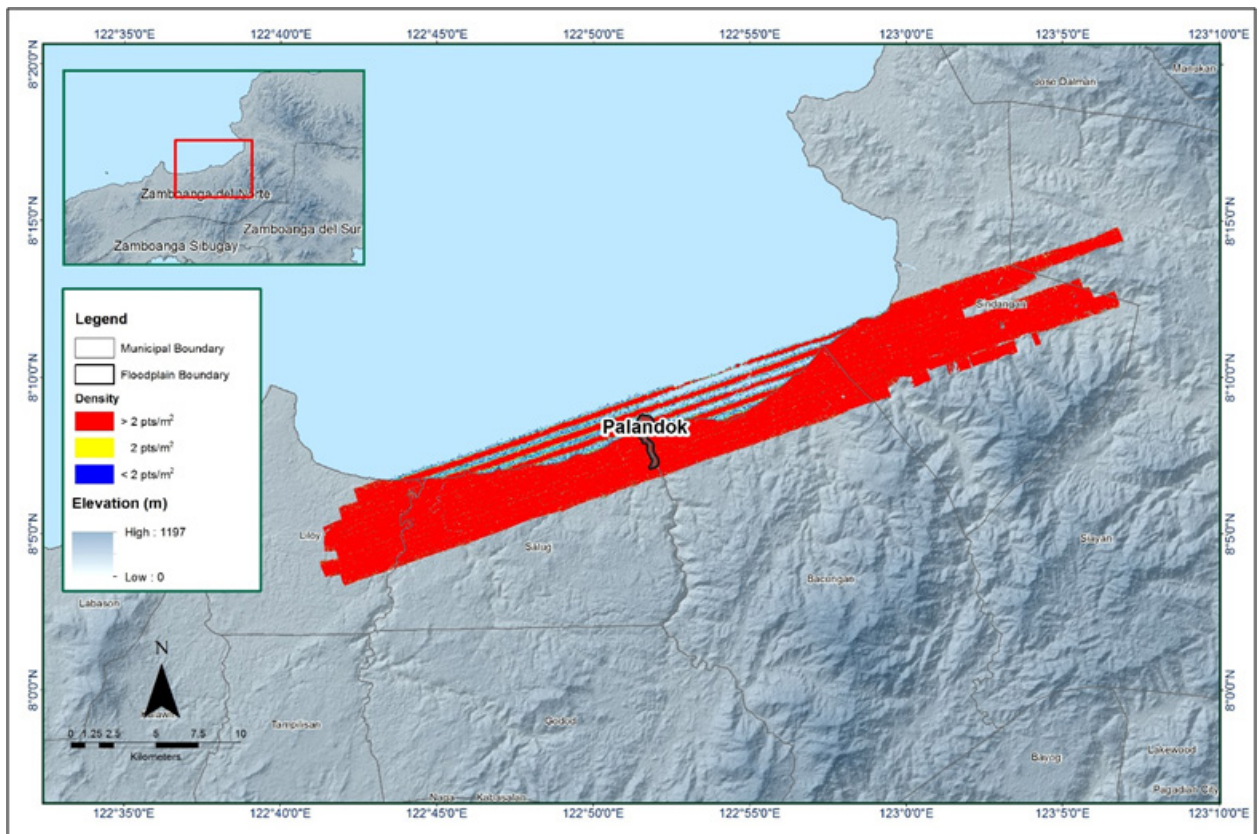


Figure 13. Pulse dDensity map of merged LiDAR data for Palandok floodplain.

The elevation difference between overlaps of adjacent flight lines is shown in Figure B-814. The default color range is from blue to red, where bright blue areas correspond to portions where elevations of a previous flight line, identified by its acquisition time, are higher by more than 0.20m relative to elevations of its adjacent flight line. Bright red areas indicate portions where elevations of a previous flight line are lower by more than 0.20m relative to elevations of its adjacent flight line. Areas with bright red or bright blue need to be investigated further using Quick Terrain Modeler software.

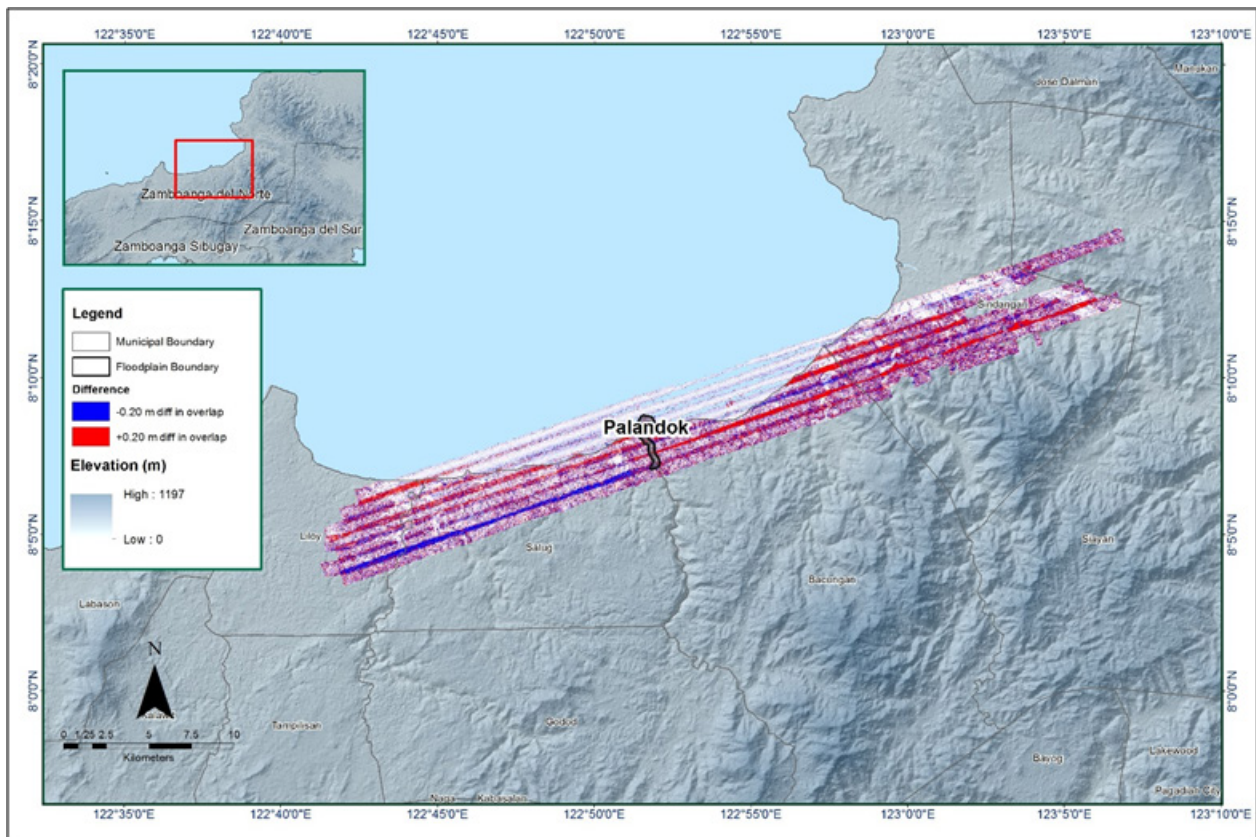


Figure 14. Elevation difference map between flight lines for Palandok floodplain.

A screen capture of the processed LAS data from a Palandok flight 2157P loaded in QT Modeler is shown in Figure B-915. The upper left image shows the elevations of the points from two overlapping flight strips traversed by the profile, illustrated by a dashed red line. The x-axis corresponds to the length of the profile. It is evident that there are differences in elevation, but the differences do not exceed the 20-centimeter mark. This profiling was repeated until the quality of the LiDAR data becomes satisfactory. No reprocessing was done for this LiDAR dataset.

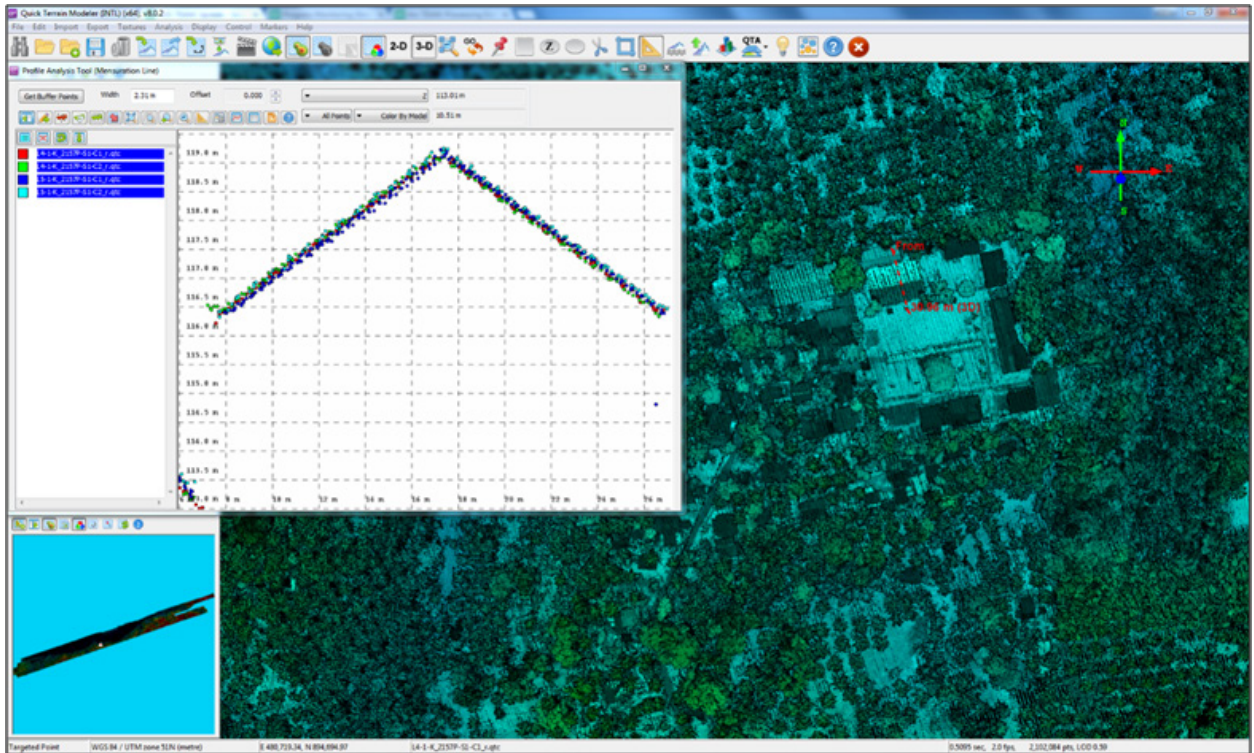


Figure 15. Quality checking for a Palandok flight 2157P using the Profile Tool of QT Modeler.

3.6 LiDAR Point Cloud Classification and Rasterization

Table 10. Table B- 3. Palandok classification results in TerraScan.

Pertinent Class	Total Number of Points
Ground	221,447,994
Low Vegetation	167,724,982
Medium Vegetation	312,332,327
High Vegetation	222,386,814
Building	4,523,324

The tile system that TerraScan employed for the LiDAR data and the final classification image for a block in Palandok Floodplain floodplain is shown in Figure B-1016. A total of 306 1km by 1km tiles were produced. The number of points classified to the pertinent categories is illustrated in Table B-310. The point cloud has a maximum and minimum height of 537.22 meters and 62.35 meters, respectively.

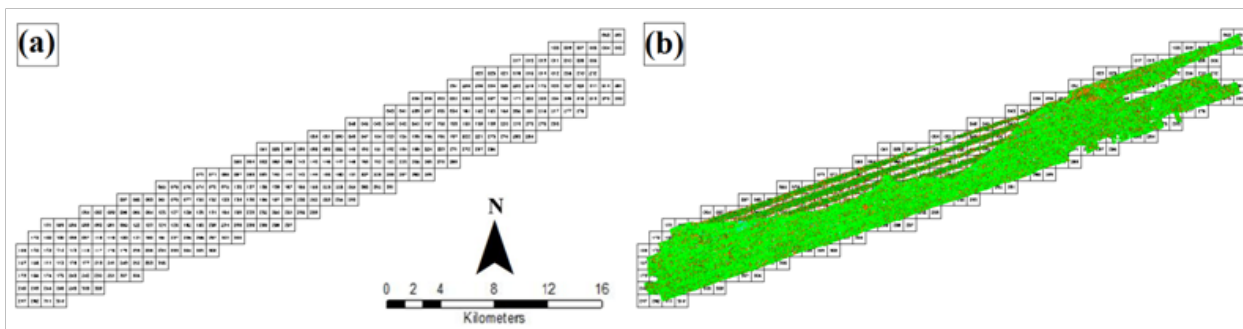


Figure 16. Tiles for Palandok floodplain (a) and classification results (b) in TerraScan.

An isometric view of an area before and after running the classification routines is shown in Figure B-1117. The ground points are in orange, the vegetation is in different shades of green, and the buildings are in cyan. It can be seen that residential structures adjacent or even below canopy are classified correctly, due to the density of the LiDAR data.

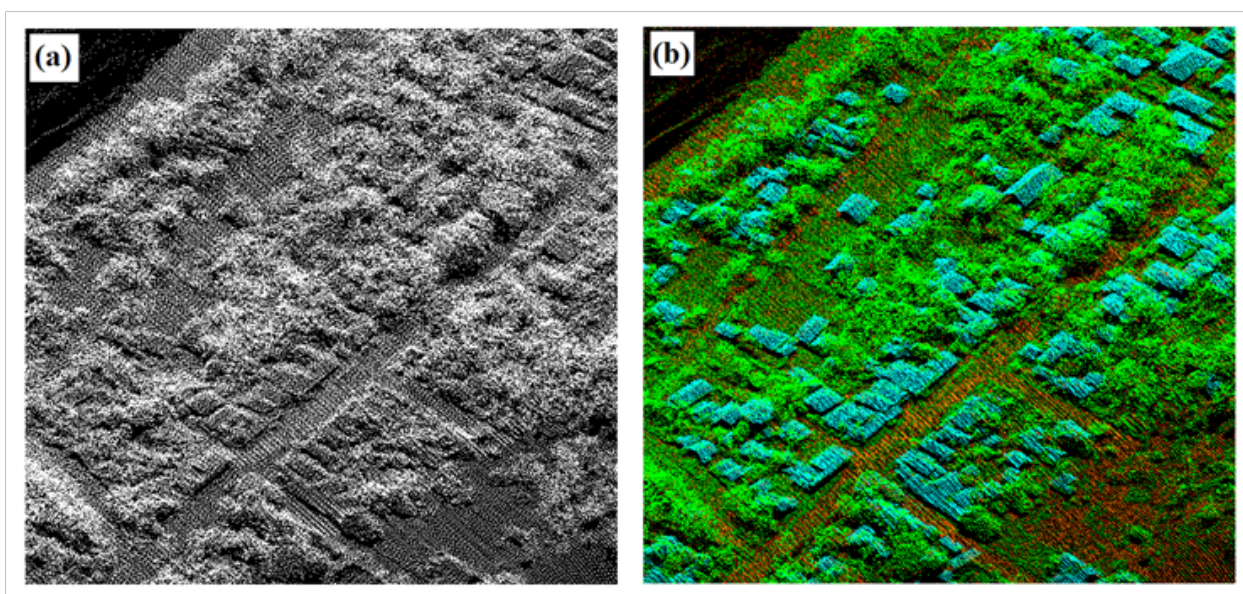


Figure 17. Point cloud before (a) and after (b) classification.

The production of last return (V_ASCII) and the secondary (T_ASCII) DTM, first (S_ASCII) and last (D_ASCII) return DSM of the area in top view display are shown in Figure B-1218. It shows that DTMs are the representation of the bare earth while on the DSMs, all features are present such as buildings and vegetation.

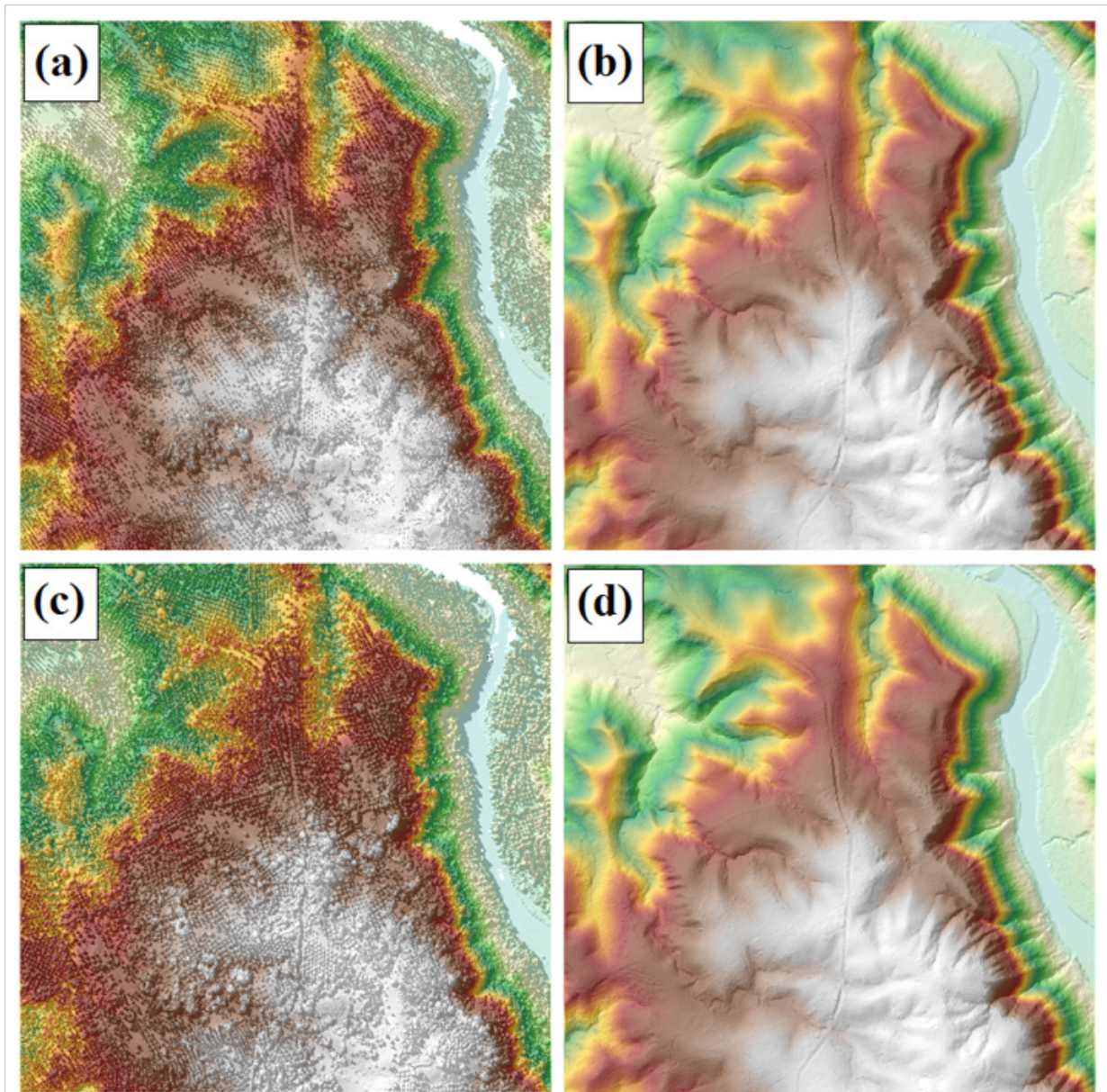


Figure 18. The production of last return DSM (a) and DTM (b), first return DSM (c) and secondary DTM (d) in some portion of Palandok Floodplain.

3.7 LiDAR Image Processing and Orthophotograph Rectification

The 297 1km by 1km tiles area covered by Palandok floodplain is shown in Figure B-1319. After tie point selection to fix photo misalignments, color points were added to smoothen out visual inconsistencies along the seamlines where photos overlap. The Palandok Floodplain floodplain has a total of 223.00 sq.km orthophotograph coverage comprised of 570 images. A zoomed in version of sample orthophotographs named in reference to its tile number is shown in Figure B-1420.

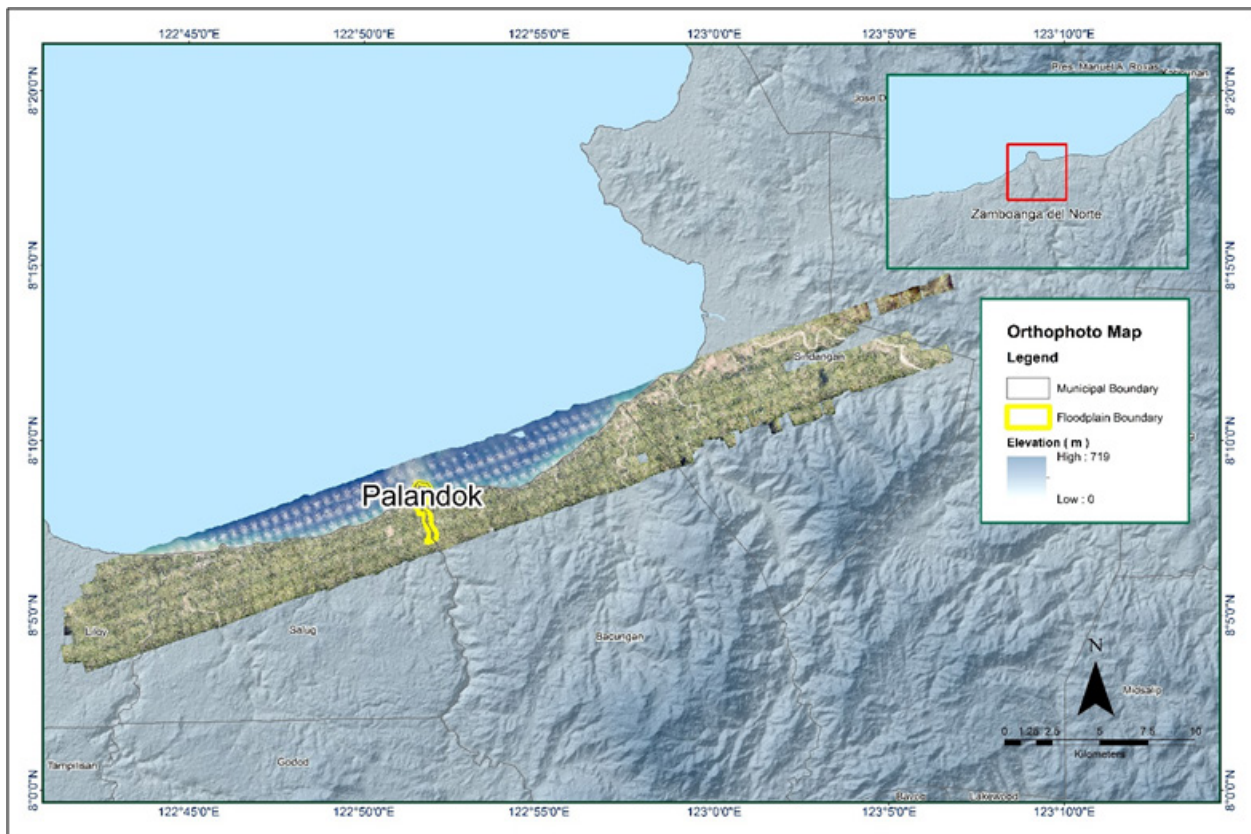


Figure 19. Palandok Floodplain floodplain with available orthophotographs.



Figure 20. Sample orthophotograph tiles for Palandok Floodplainfloodplain.

3.8 DEM Editing and Hydro-Correction

One (1) mission block was processed for Palandok Floodplain flood plain which is Dipolog_Bl73B. Table B-411 shows the name and corresponding area of each block in square kilometers.

Table 11. LiDAR blocks with its corresponding area.

LiDAR Blocks	Area (sq.km)
Dipolog_Bl73B	214.34
TOTAL	214.34 sq.km

Portions of DTM before and after manual editing are shown in Figure B-1521. The portion of the mountain (Figure B-15a21a) has been removed during classification process and has to be retrieved to complete the surface (Figure B-15b21b) to allow the correct flow of water. The bridge (Figure B-15c21c) is also considered to be an impedance to the flow of water along the river and has to be removed (Figure B-15d21d) in order to hydrologically correct the river.

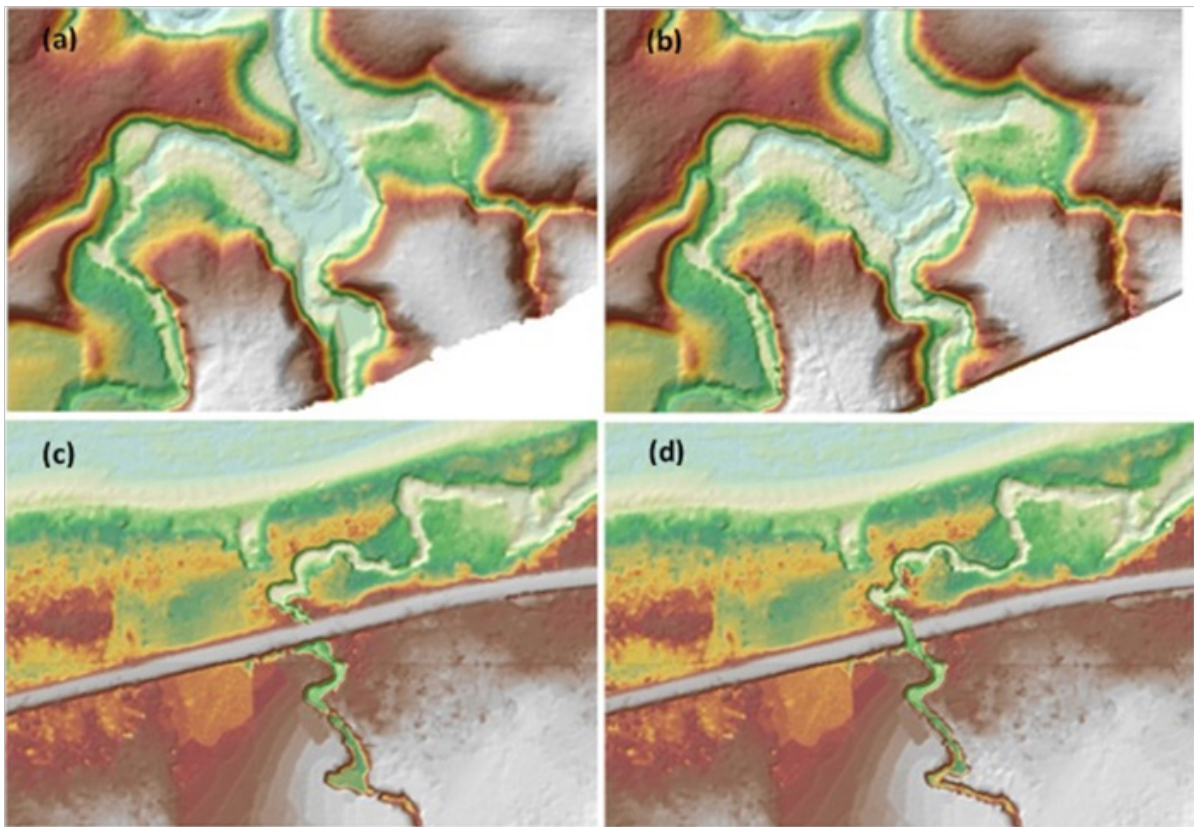


Figure 21. Portions in the DTM of Palandok floodplain – a cut portion of the mountain before (a) and after (b) data retrieval; a bridge before (c) and after (d) manual editing.

3.9 Mosaicking of Blocks

Dipolog_Bl73B fully covered the Palandok floodplain. Thus, no mosaicking of blocks was performed. Moreover, Dipolog_Bl73B was used as reference in mosaicking adjacent LiDAR blocks of Dipolog. Table B-512 shows the shift values applied to the LiDAR block covering Palandok floodplain.

Table 12. Shift Values of each LiDAR Block of Palandok floodplain.

Mission Blocks	Shift Values (meters)		
	x	y	z
Dipolog_Bl73B	0.00	0.00	0.00

The LiDAR DTM for Palandok Floodplain floodplain is shown in Figure B-1622. It can be seen that the entire Palandok Floodplain floodplain is 100% covered by LiDAR data.

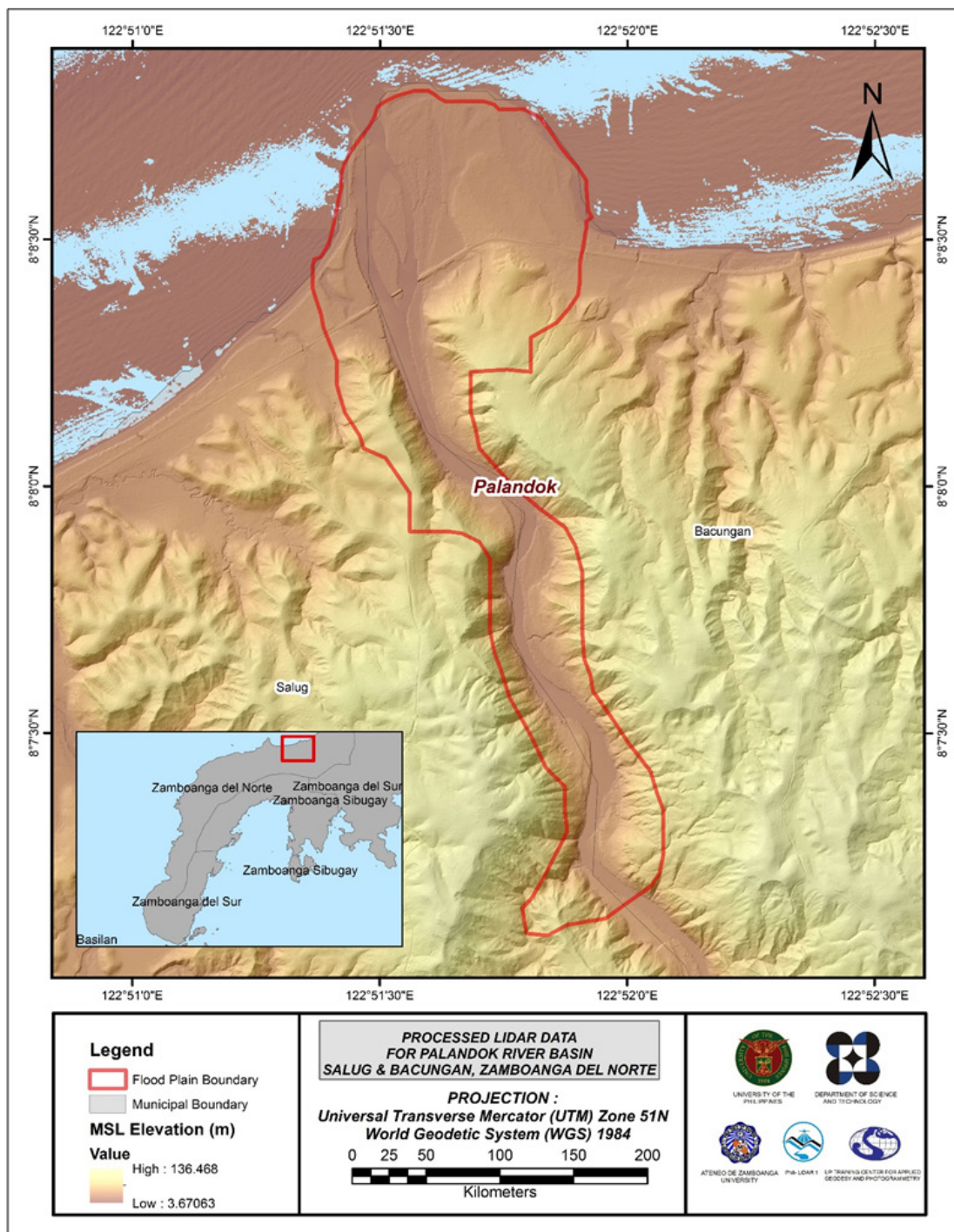


Figure 22. Map of Processed LiDAR Data for Palandok Floodplain Flood Plain.

3.10 Calibration and Validation of Mosaicked LiDAR Digital Elevation Model

The extent of the validation survey done by the Data Validation and Bathymetry Component (DVBC) in Palandok to collect points with which the LiDAR dataset is validated is shown in Figure B-1723. A total of 5,856 survey points were used for calibration and validation of Palandok LiDAR data. Random selection of 80% of the survey points, resulting in 4,685 points, were used for calibration. A good correlation between the uncalibrated mosaicked LiDAR elevation values and the ground survey elevation values is shown in Figure B-1824. Statistical values were computed from extracted LiDAR values using the selected points to assess the quality of data and obtain the value for vertical adjustment. The computed height difference between the LiDAR DTM and calibration elevation values is 4.62 meters with a standard deviation of 0.16 meters. Calibration of Palandok LiDAR data was done by adding the height difference value, 4.62 meters, to Palandok mosaicked LiDAR data. Table B-613 shows the statistical values of the compared elevation values between LiDAR data and calibration data.

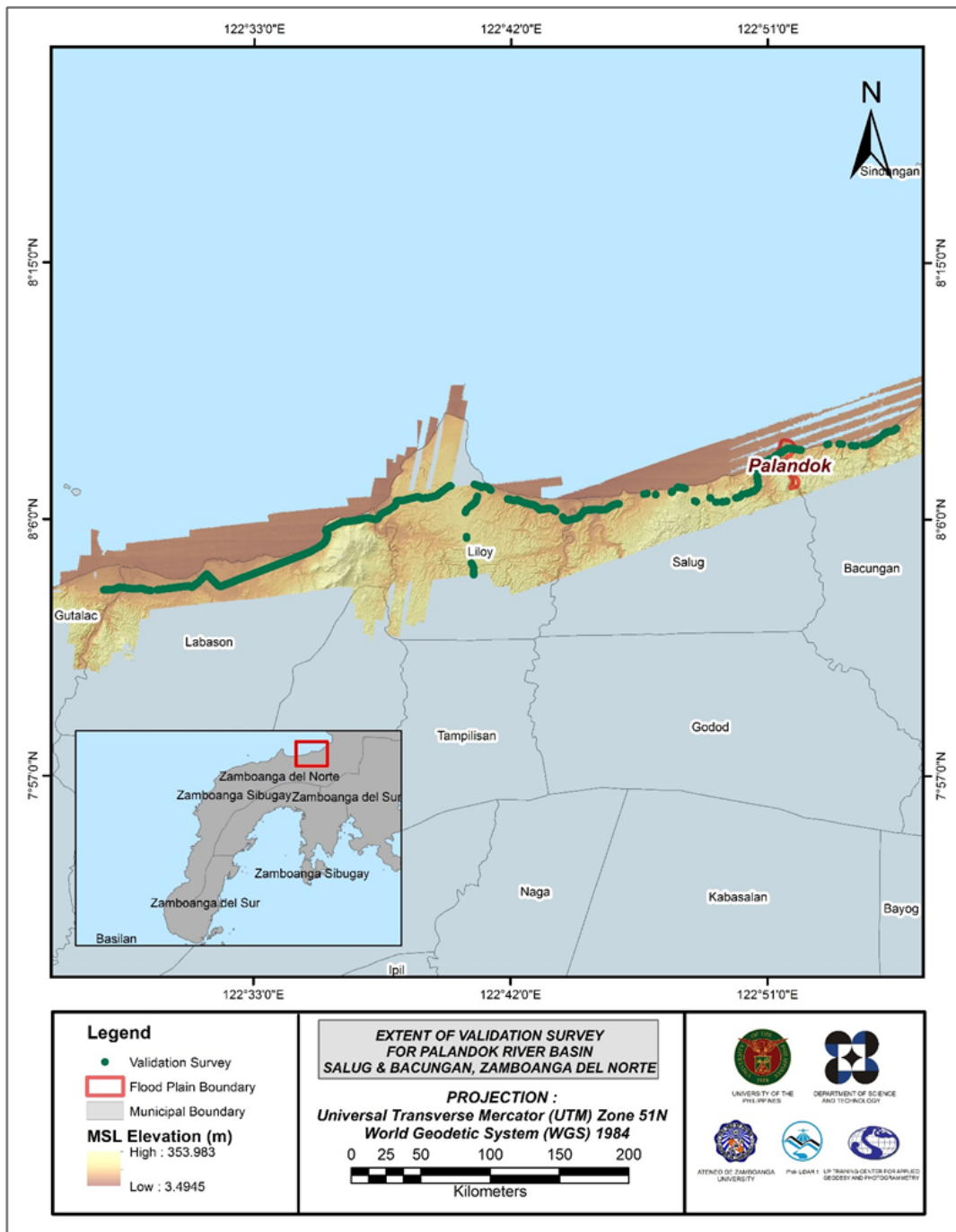


Figure 23. Map of Palandok Floodplain Flood Plain with validation survey points in green.

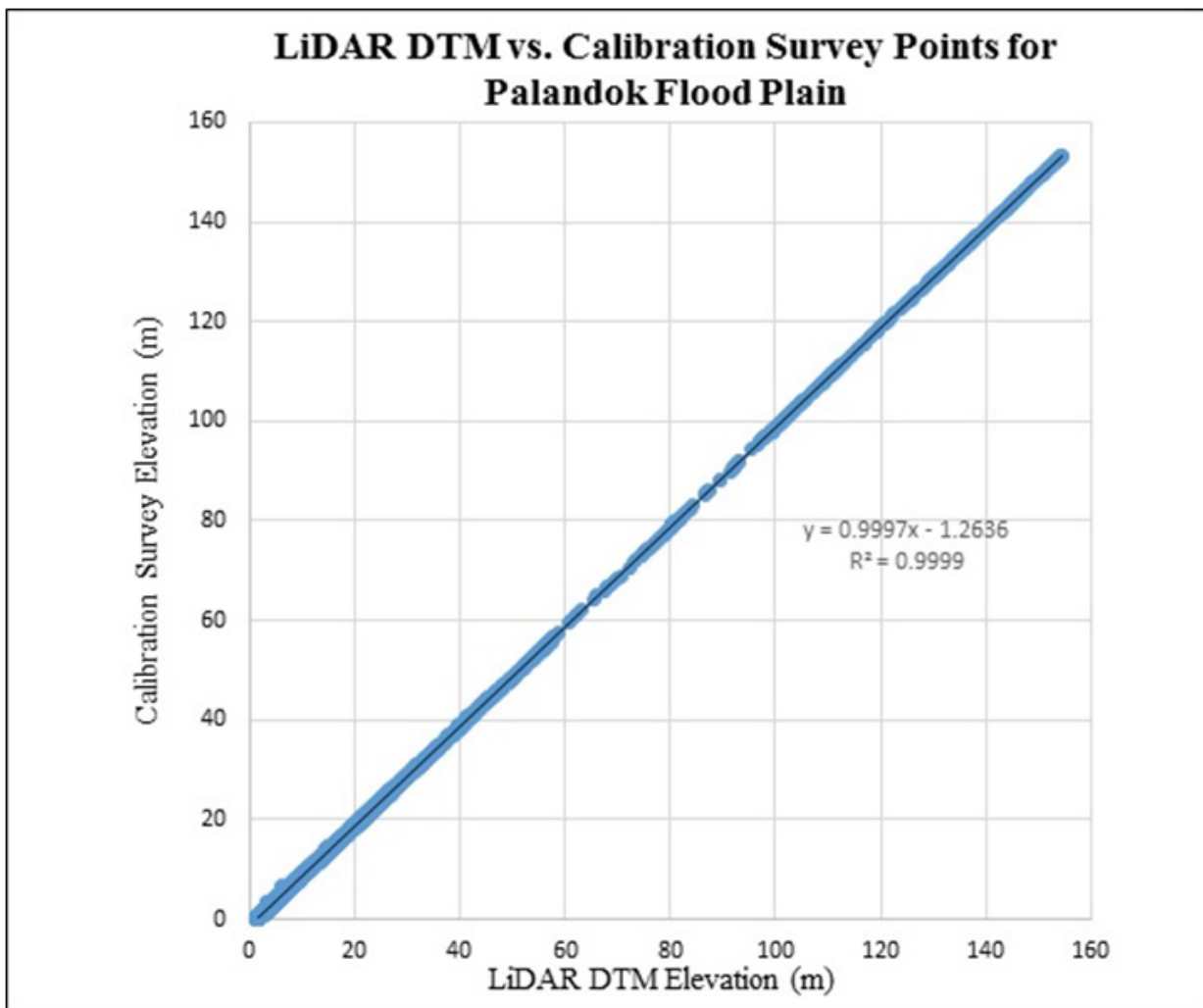


Figure 24. Correlation plot between calibration survey points and LiDAR data.

Table 13. Calibration Statistical Measures.

Calibration Statistical Measures	Value (meters)
Height Difference	4.62
Standard Deviation	0.16
Average	4.62
Minimum	4.30
Maximum	4.93

The remaining 20% of the total survey points is equivalent to 1,171. Fourteen (14) of the said points lie within the Palandok Floodplain flood plain and were used for the validation of calibrated Palandok DTM. A good correlation between the calibrated mosaicked LiDAR elevation values and the ground survey elevation, which reflects the quality of the LiDAR DTM is shown in Figure B-1925. The computed RMSE between the calibrated LiDAR DTM and validation elevation values is 0.06 meters with a standard deviation of 0.02 meters, as shown in Table B-714.

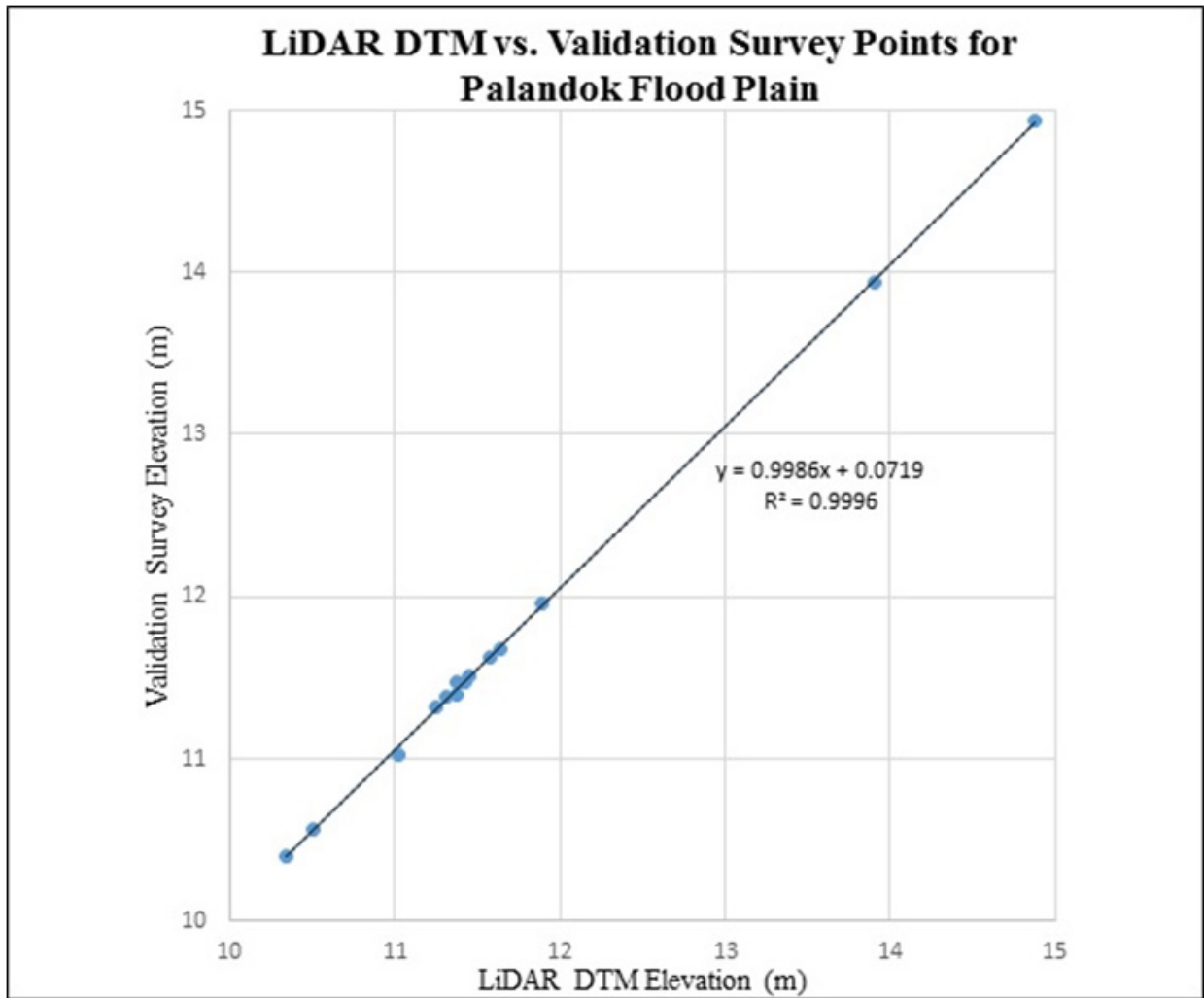


Figure 25. Correlation plot between validation survey points and LiDAR data.

Table 14. Validation Statistical Measures.

Validation Statistical Measures	Value (meters)
RMSE	0.06
Standard Deviation	0.02
Average	-0.05
Minimum	-0.10
Maximum	-0.00

3.11 Integration of Bathymetric Data into the LiDAR Digital Terrain Model

For bathy integration, cross section data was available for Palandok with 1,299 bathymetric survey points. The resulting raster surface produced was done by Inverse Distance Weighted (IDW) interpolation method. After burning the bathymetric data to the calibrated DTM, assessment of the interpolated surface is represented by the computed RMSE value of 0.56 meters. The extent of the bathymetric survey done by the Data Validation and Bathymetry Component (DVBC) in Palandok integrated with the processed LiDAR DEM is shown in Figure B-2026.

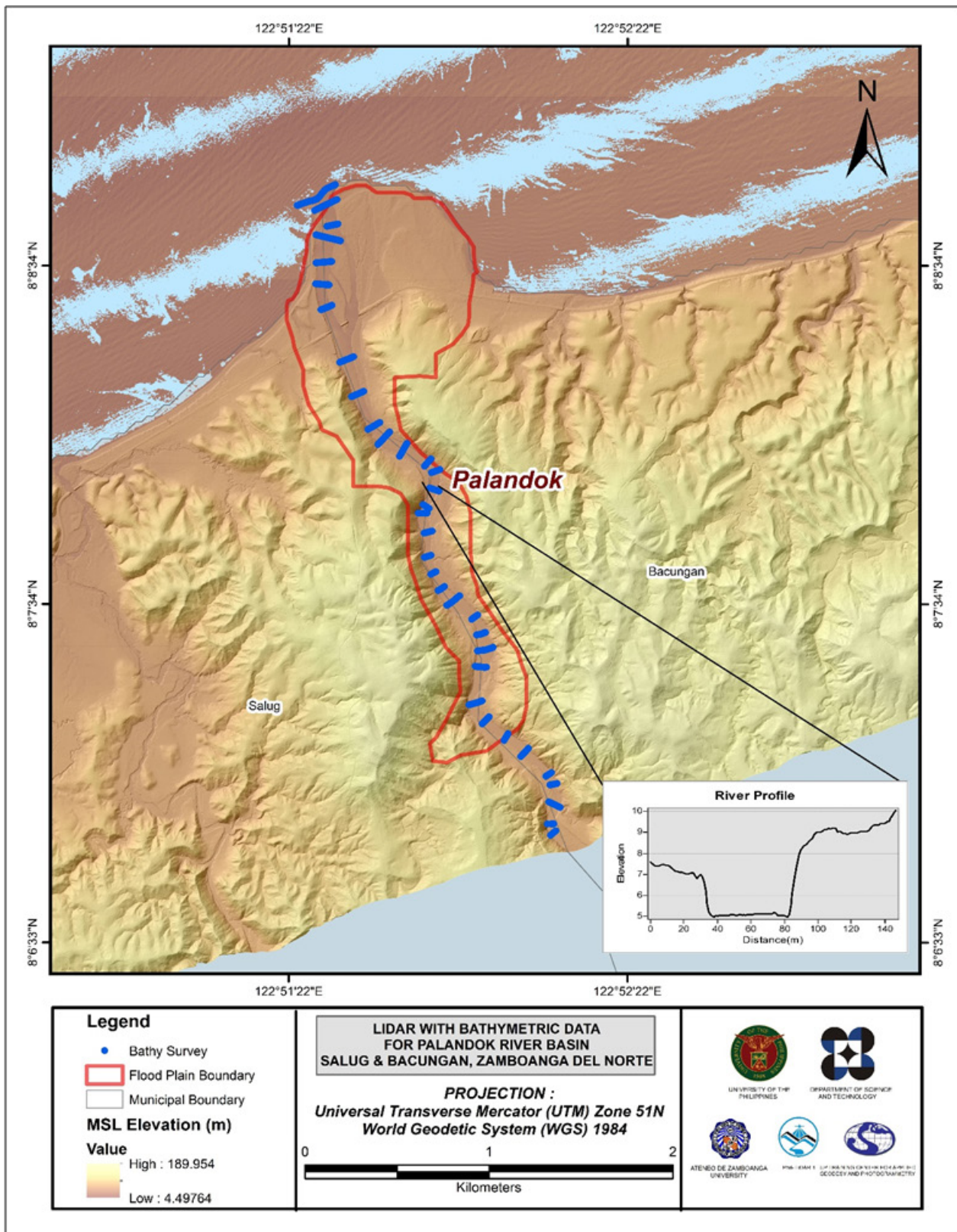


Figure 26. Map of Palandok Floodplain Flood Plain with bathymetric survey points shown in blue.

3.12 Feature Extraction

The features salient in flood hazard exposure analysis include buildings, road networks, bridges and water bodies within the floodplain area with 200 m buffer zone. Mosaicked LiDAR DEM with 1 m resolution was used to delineate footprints of building features, which consist of residential buildings, government offices, medical facilities, religious institutions, and commercial establishments, among others. Road networks comprise of main thoroughfares such as highways and municipal and barangay roads essential for routing of disaster response efforts. These features are represented by a network of road centerlines.

3.12.1 Quality Checking of Digitized Features' Boundary

Palandok floodplain, including its 200 m buffer, has a total area of 3.42 sq km. For this area, a total of 1.0 sq km, corresponding to a total of 264 building features, are considered for QC. Figure B-2127 shows the QC blocks for Palandok Floodplain floodplain.

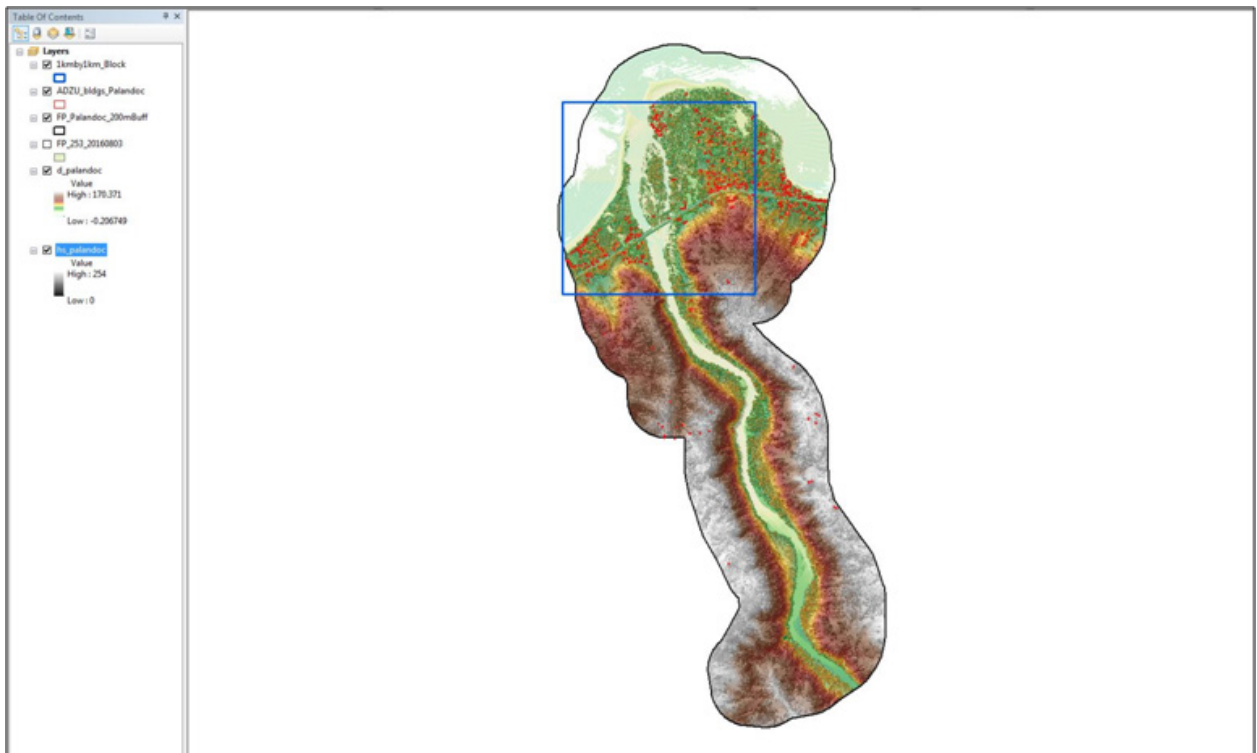


Figure 27. QC blocks for Palandok building features.

Quality checking of Palandok building features resulted in the ratings shown in Figure27.

Table 15. Quality Checking Ratings for Palandok Building Features.

FLOODPLAIN	COMPLETENESS	CORRECTNESS	QUALITY	REMARKS
Palandok	100.00	94.70	89.39	PASSED

3.12.2 Height Extraction

Height extraction was done for 501 building features in Palandok Floodplain. Of these building features, none was filtered out after height extraction, resulting to 501 buildings with height attributes. The lowest building height is at 2.00 m, while the highest building is at 5.67 m.

3.12.3 Feature Attribution

One of the Research Associates of ADZU Phil LiDAR 1 was able to develop GEONYT, an offline web-based application for feature attribution extracted from a LiDAR-based Digital Surface Model and which attribution is conducted by combining automatic data consolidation, geotagging and offline navigation. The app is conveniently integrated in a smart phone/ tablet. The data collected are automatically stored in database and can be viewed as CSV (or excel) and KML (can viewed via google earth). The Geonyt App was the main tool used in all feature attribution activity of the team.

The team, thru through the endorsement of the Local Government Units of the Municipality/ City hired a number of enumerators who conducted the house-to-house survey of the features using the GEONYT application. The team provided the enumerators smart tablets where the GEONYT is integrated. The number of days by which the survey was conducted was dependent on the number of features of the flood plain of the river basin; likewise, the number of enumerators are was also dependent on the availability of the tablet and the number of features of the flood plain.

Table B-916 summarizes the number of building features per type. On the other hand, Table B-1017 shows the total length of each road type, while Table B-1118 shows the number of water features extracted per type.

Table 16. Building Features Extracted for Palandok Floodplain.

Facility Type	No. of Features
Residential	455
School	13
Market	4
Agricultural/Agro-Industrial Facilities	7
Medical Institutions	1
Barangay Hall	1
Military Institution	0
Sports Center/Gymnasium/Covered Court	1
Telecommunication Facilities	0
Transport Terminal	0
Warehouse	8
Power Plant/Substation	0
NGO/CSO Offices	0
Police Station	0
Water Supply/Sewerage	0
Religious Institutions	7
Bank	0
Factory	0
Gas Station	0
Fire Station	0
Other Government Offices	1
Other Commercial Establishments	3
Total	501

Table 17. Total Length of Extracted Roads for Palandok Floodplain.

Floodplain	Road Network Length (km)					Total
	Barangay Road	City/Municipal Road	Provincial Road	National Road	Others	
Palandok	0.66	0.00	0.00	1.45	0.00	2.11

Table 18. Number of Extracted Water Bodies for Palandok Floodplain.

Floodplain	Water Body Type					Total
	Fish Pen	City/Municipal Road	Provincial Road	National Road	Others	
Palandok	3	0	0	0	0	3

One (1) bridge over small channels that are part of the river network was also extracted for the floodplain.

3.12.4 Final Quality Checking of Extracted Features

All extracted ground features were completely given the required attributes. All these output features comprise the flood hazard exposure database for the floodplain. This completes the feature extraction phase of the project.

Figure B-2228 shows the Digital Surface Model (DSM) of Palandok Floodplain floodplain overlaid with its ground features.

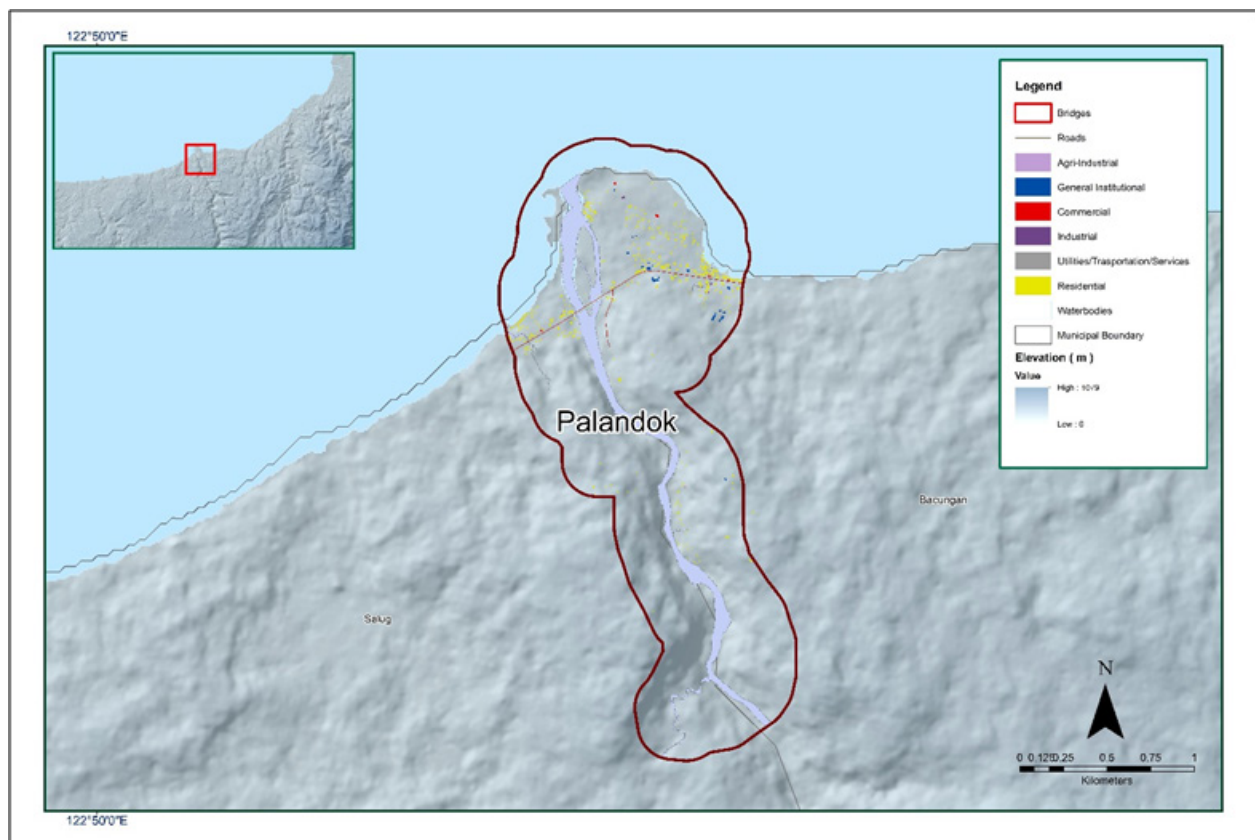


Figure 28. Extracted features for Palandok Floodplain floodplain.

CHAPTER 4: LIDAR VALIDATION SURVEY AND MEASUREMENTS OF THE BATU PALANDOK RIVER BASIN

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The methods applied in this chapter were based on the DREAM methods manual (Balicanta et al., 2014) and further enhanced and updated in Paringit et al. (2017).

4.1 Summary of Activities

In line with this, The ABSD conducted a field survey in Polandok Palandok River April 9, 12, 14, 15, 18, 19, 21, 23, 28, and May 2, 2016 with the following scope of work: cross-section, bridge as-built and water level marking in MSL of Polandok Bridge and manual bathymetric survey from the mouth of the river in Brgy. Rizon to the upstream in Brgy. Morob in the Municipality of Bacungan using GNSS survey technique and Total Station and bathymetry data were gathered by DVC on August 21-31, 2016 using an Ohmex™ Single Beam Echo Sounder and Trimble® SPS 882 GNSS PPK survey technique. In addition to this, validation points acquisition survey was conducted covering the Polandok Palandok River Basin area. The entire survey extent is illustrated in Figure C-129.

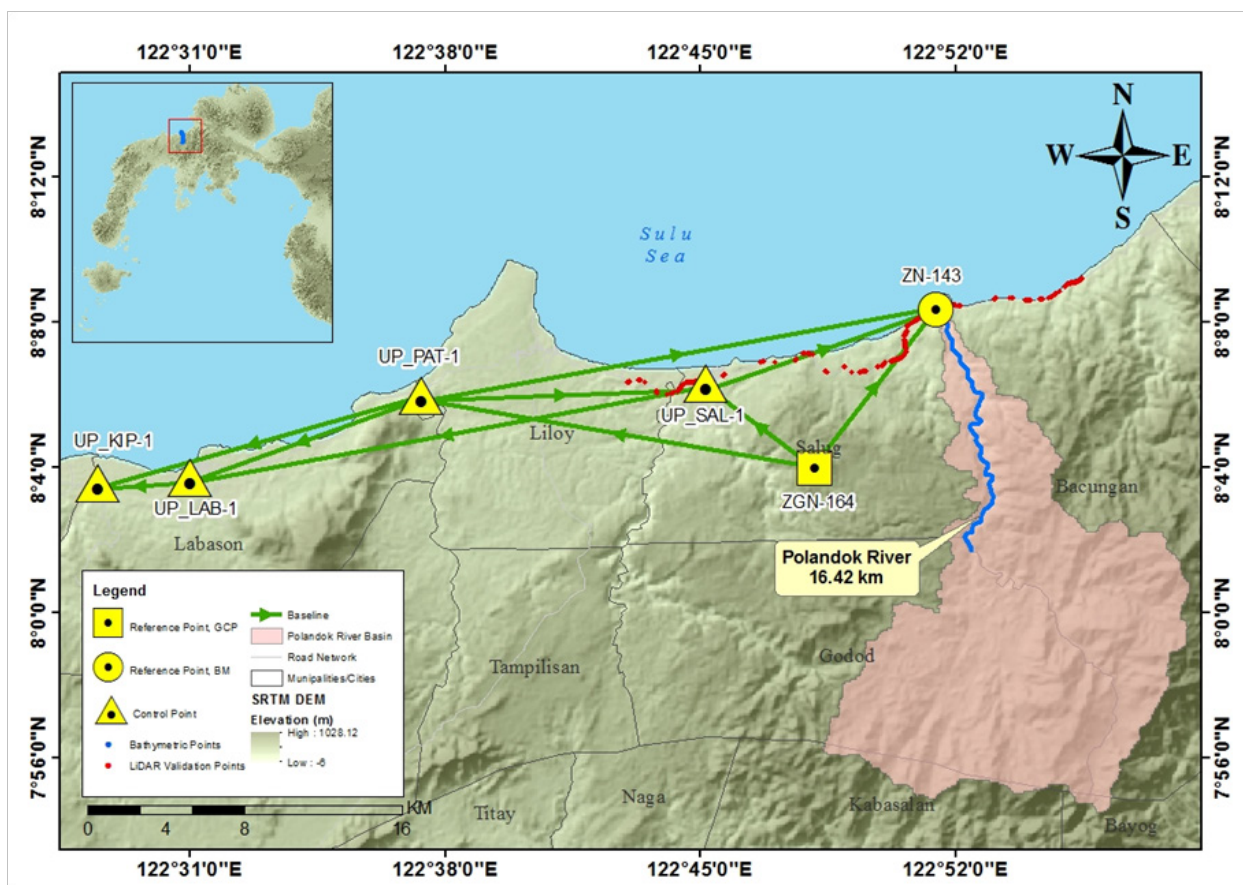


Figure 29. Polandok River Survey Extent

4.2 Control Survey

The GNSS network used for Palandok River is composed of five (5) loops established on August 24, 2016 occupying the following occupying the following reference points: ZGN-164, a second-order GCP, situated inside the barangay hall compound beside basketball court in Brgy. Caracol, Salug, Province of Zamboanga Del Norte; and ZN-143, a first-order BM, located at Brgy. Ramon Magsaysay, Salug, Province of Zamboanga Del Norte.

Four (4) control points established in the area by ABSD: UP_KIP-1 located at the approach of Kipit Bridge in Brgy. Kipit, Municipality of Labason, UP_LAB-1 inat Labason Bridge in Brgy. Antonio, Municipality of Labason, UP_PAT-1 at the side of Labason-Liloy Road near Patawag Bridge in Brgy. Patawag, Municipality of Labason, and UP_SAL-1 located at the side of Ipil-Dipolog Highway near Salug Bridge in Brgy. La Libertad, Municipality of Gutalac. The summary of reference and control points and its location is summarized in Table C-119 while GNSS network established is illustrated in Figure C-230.

Figure 71. Table 19. Table C-1 List of reference and control points used during the survey in Palandok Palandok River. (Source: NAMRIA, UP-TCAGP)

Control Point	Order of Accuracy	Geographic Coordinates (WGS 84)				
		Latitude	Longitude	Ellipsoidal Height (m)	MSL Elevation (m)	Date Established
ZGN-164	2nd Order GCP	8°03'58.80475" N	E122°48'08.60698" E	296.130	229.325	2004
ZN-143	1st Order BM	8°08'21.39646" N		22°51' 28.86114" E	11.526	2009
UP_KIP-1	Established	8°03'35.83524" N	122°28'26.48383"E	78.022	12.435	08-24-16
UP_LAB-1	Established	8°03'44.29109" N	122°30'59.74333"E	75.708	9.889	08-24-16
UP_PAT-1	Established	8°06'00.79142" N	122°37'19.54470" E	76.488	10.835	08-24-16
UP_SAL-1	Established	8°06'20.46964"N	122°45'09.85390"E	76.124	10.080	08-24-16

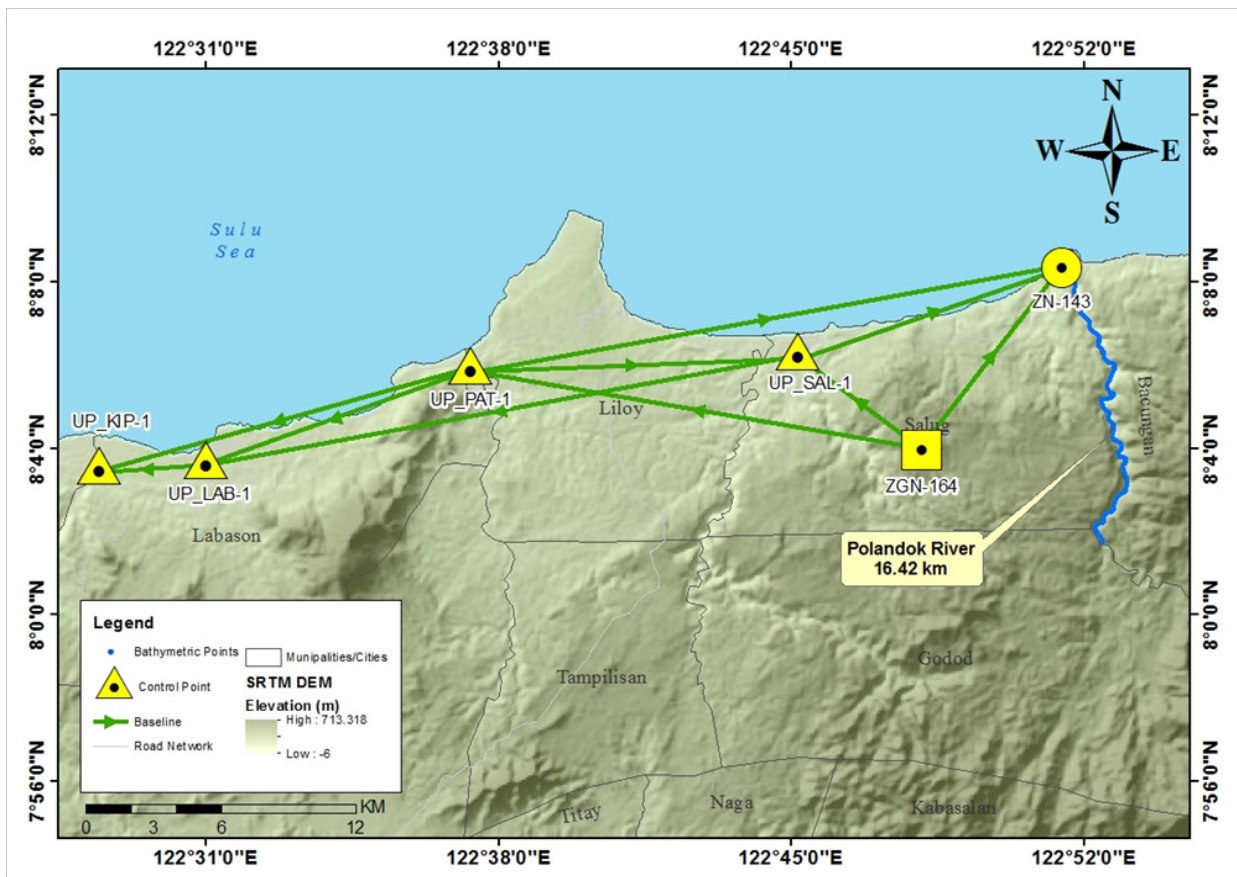


Figure 30. Polandok River Basin Control Survey Extent

The GNSS set-ups on recovered reference points and established control points in Polandok Palandok River are shown from Figure C-331 to Figure C-836.



Figure 31. GNSS rover, Hi-Target™ V30, ZGN-164, situated inside the barangay hall compound beside basketball court in Brgy. Caracol, Salug, Province of Zamboanga del Norte



Figure 32. GNSS rover, Hi-Target™ V30, at ZN-143, an established control point, located at the right side walk going to Brgy. Rizon direction of Palandok Bridge in Brgy. Ramon Magsaysay, Salug, Province of Zamboanga del Norte



Figure 33. Figure C-5UP_KIP-1 located at the approach of Kipit Bridge in Brgy. Kipit, Municipality of Labason, Province of Zamboanga del Norte



Figure 34. Figure C-6 GNSS receiver set up, Hi-Target™ V30, at UP_LAB-1 at Labason Bridge in Brgy. Antonio, Municipality of Labason, Province of Zamboanga del Norte

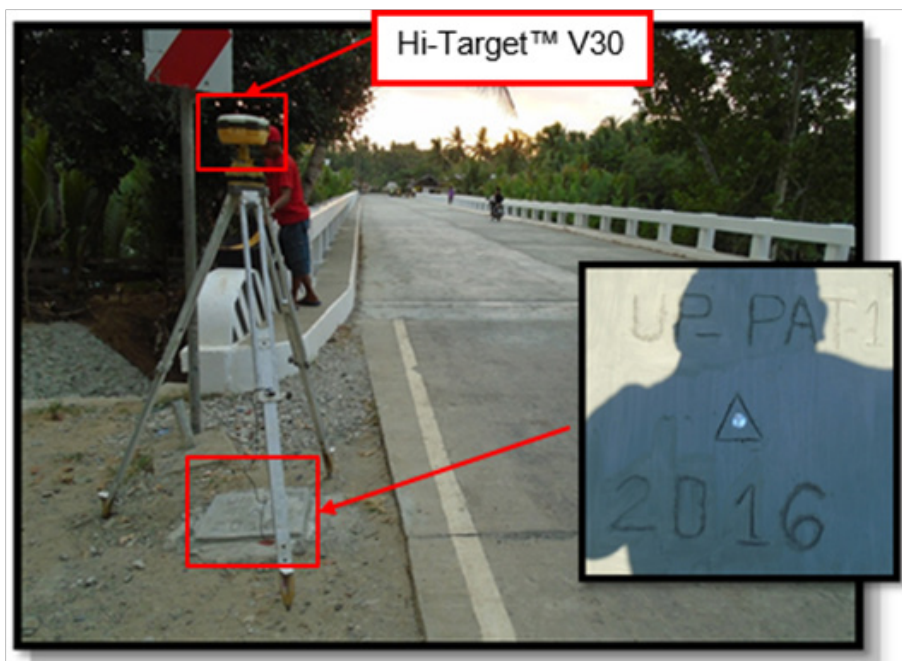


Figure 35. GNSS receiver set up, Hi-Target™ V30, UP_PAT-1 at the side of Labason-Liloy Road near Patawag Bridge in Brgy. Patawag, Municipality of Labason, Province of Zamboanga del Norte



Figure 36. GNSS receiver set up, Hi-Target™ V30, UP_SAL-1 located at the side of Ipil-Dipolog Highway near Salug Bridge in Brgy. La Libertad, Municipality of Gutalac, Province of Zamboanga del Norte

4.3 Baseline Processing

GNSS Baselines were processed simultaneously in TBC by observing that all baselines have fixed solutions with horizontal and vertical precisions within ± 20 cm and ± 10 cm requirement, respectively. In case where one or more baselines did not meet all of these criteria, masking is was performed. Masking is done by removing/masking portions of these baseline data using the same processing software. It is repeatedly processed until all baseline requirements are met. If the reiteration yields out of the required accuracy, resurvey is initiated. Baseline processing result of control points in Palandok River Basin is summarized in Table C-220 generated by TBC software.

Table 20. Baseline Processing Report for Polandok River Static Survey (First Network)

Observation	Date of Observation	Solution Type	H. Prec. (Meter)	V. Prec. (Meter)	Geodetic Az.	Ellipsoid Dist. (Meter)	ΔHeight (Meter)
UP_KIP-1 --- UP_PAT-1	8-27-2016	Fixed	0.006	0.035	254°44'54"	16917.407	1.523
UP_LAB-1 --- UP_SAL-1	8-27-2016	Fixed	0.005	0.054	259°34'18"	26466.198	-0.410
UP_LAB-1 --- UP_KIP-1	8-27-2016	Fixed	0.006	0.033	266°50'03"	4699.763	2.326
UP_SAL-1 --- UPPAT1	8-27-2016	Fixed	0.006	0.040	87°35'11"	14411.370	-0.290
UP_LAB-1 --- UP_PAT-1	8-27-2016	Fixed	0.013	0.081	250°10'37"	12361.399	-0.729

As shown Table C-220 a total of five (5) baselines were processed with coordinate and ellipsoidal height values of UP_PAT-1 and UP_SAL-1 held fixed. All of them passed the required accuracy.

Table 21. Baseline Processing Report for Polandok River Static Survey (Second Network)

Observation	Date of Observation	Solution Type	H. Prec. (Meter)	V. Prec. (Meter)	Geodetic Az.	Ellipsoid Dist. (Meter)	ΔHeight (Meter)
UP_PAT-1 --- ZGN-164	8-27-2016	Fixed	0.006	0.029	280°41'32"	20222.545	-219.677
ZGN-164 --- UP_ SAL-1	8-27-2016	Fixed	0.004	0.030	308°29'46"	6992.329	-219.983
ZGN-164 --- ZN- 143	8-27-2016	Fixed	0.005	0.021	37°13'49"	10132.429	-218.798
UP_PAT-1 --- UP_SAL-1	8-27-2016	Fixed	0.009	0.047	87°35'12"	14411.385	-0.393
UP_PAT-1 --- ZN- 143	8-27-2016	Fixed	0.008	0.042	80°33'03"	26357.443	0.785
UP_SAL-1 --- ZN- 143	8-27-2016	Fixed	0.006	0.035	72°14'19"	12183.132	1.211

As shown in Table C-321, a total of six (6) baselines were processed with coordinate and ellipsoidal height values of ZGN-164 and ZN-143 held fixed. All of them passed the required accuracy.

4.4 Network Adjustment

After the baseline processing procedure, network adjustment is was performed using TBC. Looking at the Adjusted Grid Coordinates table of the TBC generated Network Adjustment Report, it is observed that the square root of the squares of x and y must be less than 20 cm and z less than 10 cm in equation form:

$$\sqrt{(x_e)^2 + (y_e)^2} < 20 \text{ cm and } z_e < 10 \text{ cm}$$

Where:

- x_e is the Easting Error,
- y_e is the Northing Error, and
- z_e is the Elevation Error

for each control point. See the Network Adjustment Report shown from Table C-322 to Table C-5Table 24 for the complete details. Refer to Annex A for the computation for the accuracy of ABSD.

The four (4) control points, UP_KIP-1, UP_LAB-1, UP_PAT-1, UP_SAL-1 were occupied and observed simultaneously to form a GNSS loop. For the second network, the four (4) control points ZGN-164, ZN-143, UP_PAT-1, and UP_SAL-1. The coordinates and ellipsoidal height of UP_PAT-1 and UP_SAL-1, and ZGN-164 and ZN-143 were held fixed during the processing of the control points as presented in Tables C-422 and C-523. Through this reference point, the coordinates and ellipsoidal height of the unknown control points will be computed.

Table 22. Control Point Constraints (First Network)

Point ID	Type	East σ (Meter)	North σ (Meter)	Height σ (Meter)	Elevation σ (Meter)
UP_PAT-1	Local	Fixed	Fixed	Fixed	
UP_SAL-1	Local	Fixed	Fixed	Fixed	
Fixed = 0.000001(Meter)					

Table 23. Control Point Constraints (Second Network)

Point ID	Type	East σ (Meter)	North σ (Meter)	Height σ (Meter)	Elevation σ (Meter)
ZGN-164	Local	Fixed	Fixed		
ZN-143	Grid				Fixed
Fixed = 0.000001(Meter)					

The lists of adjusted grid coordinates, i.e. Northing, Easting, Elevation and computed standard errors of the control points in the network are indicated in Tables C-624 and C-725. All fixed control points have no values for grid errors and elevation error.

Table 24. Adjusted Grid Coordinated (First Network)

Point ID	Easting (Meter)	Easting Error (Meter)	Northing (Meter)	Northing Error (Meter)	Elevation (Meter)	Elevation Error (Meter)	Cons traint
UP_KIP-1	442045.506	0.011	890963.192	0.011	12.435	0.053	
UP_LAB- 1	446736.710	0.011	891217.077	0.011	9.889	0.058	
UP_PAT-1	458365.200	?	895396.684	?	10.835	?	LLh
UP_SAL- 1	472758.821	?	895989.921	?	10.080	?	LLh

With the mentioned equation, $\sqrt{(x_e)^2 + (y_e)^2} < 20\text{cm}$ for horizontal and $z_e < 10\text{ cm}$ for the vertical; the computation for the accuracy are as follows:

UP_KIP-1
 horizontal accuracy = $\sqrt{(0.1)^2 + (1.1)^2}$
 = $\sqrt{0.01 + 1.21}$
 = $1.10 < 20\text{ cm}$
 vertical accuracy = $5.3 < 10\text{ cm}$

UP_LAB-1
 horizontal accuracy = $\sqrt{(0.1)^2 + (1.1)^2}$
 = $\sqrt{0.01 + 1.21}$
 = $1.10 < 20\text{ cm}$
 vertical accuracy = $5.8 < 10\text{ cm}$

UP_PAT-1
 horizontal accuracy = Fixed
 vertical accuracy = Fixed

UP_SAL-1
 horizontal accuracy = Fixed
 vertical accuracy = Fixed

Table 25. Adjusted Grid Coordinated

Point ID	Easting (Meter)	Easting Error (Meter)	Northing (Meter)	Northing Error (Meter)	Elevation (Meter)	Elevation Error (Meter)	Constraint
ZGN-164	478227.123	?	891636.530	?	229.325	0.040	LL
ZN-143	484358.928	0.006	899697.912	0.006	11.526	?	e
UP_PAT-1	458365.200	0.007	895396.684	0.006	10.835	0.054	
UP_SAL-1	472758.821	0.006	895989.921	0.005	10.080	0.051	

a. ZGN-164
 horizontal accuracy = Fixed
 vertical accuracy = Fixed

b. ZN-143
 horizontal accuracy = Fixed
 vertical accuracy = Fixed

c. UP_PAT-1
 horizontal accuracy = $\sqrt{(0.7)^2 + (0.6)^2}$
 = $\sqrt{0.49 + 0.36}$
 = $0.85 < 20\text{ cm}$
 vertical accuracy = $5.4 < 10\text{ cm}$

d. UP_SAL-1
 horizontal accuracy = $\sqrt{(0.6)^2 + (0.5)^2}$
 = $\sqrt{0.36 + 0.25}$
 = $0.61 < 20\text{ cm}$
 vertical accuracy = $5.1 < 10\text{ cm}$

Following the given formula, the horizontal and vertical accuracy result of the six (6) occupied control points are within the required precision.

Table 26. Adjusted Geodetic Coordinates (First Network)

Point ID	Latitude	Longitude	Height (Meter)	Height Error (Meter)	Constraint
UP_KIP-1	8°03'35.83524" N	122°28'26.48383"E	78.022	0.053	
UP_LAB-1	8°03'44.29109" N	122°30'59.74333"E	75.708	0.058	
UP_PAT-1	8°06'00.79142" N	122°37'19.54470" E	76.488	?	LLh
UP_SAL-1	8°06'20.46964"N	122°45'09.85390"E	76.124	?	LLh

Table 27. Adjusted Geodetic Coordinates (Second Network)

Point ID	Latitude	Longitude	Height (Meter)	Height Error (Meter)	Constraint
ZGN-164	8°03'58.80475" N	122°48'08.60698" E	296.130	0.040	LL
ZN-143	8°08'21.39646" N	122°51'28.86114" E	77.323	?	e
UP_PAT-1	8°06'00.79142" N	122°37'19.54470" E	76.488	0.054	
UP_SAL-1	8°06'20.46964" N	122°45'09.85390" E	76.124	0.051	

The corresponding geodetic coordinates of the observed points are within the required accuracy as shown in Tables C-826 and Table C-927. Based on the result of the computation, the equation is satisfied; hence, the required accuracy for the program was met.

The summaries of reference control points used are indicated in Tables C-1028 and C-1129.

Table 28. Reference and control points used and its location (Source: NAMRIA, UP-TCAGP)

Control Point	Order of Accuracy	Geographic Coordinates (WGS 84)			UTM ZONE 51 N		
		Latitude	Longitude	Ellipsoidal Height (Meter)	Northing (m)	Easting (m)	BM Ortho (m)
UP_KIP-1	Established	8°03'35.83524" N	122°28'26.48383"E	78.022	890963.192	442045.506	12.435
UP_LAB-1	Established	8°03'44.29109" N	122°30'59.74333"E	75.708	891217.077	446736.710	9.889
UP_PAT-1	Established	8°06'00.79142" N	122°37'19.54470" E	76.488	895396.684	458365.200	10.835
UP_SAL-1	Established	8°06'20.46964"N	122°45'09.85390"E	76.124	895989.921	472758.821	10.080

Table 29. Reference and control points used and its location (Source: NAMRIA, UP-TCAGP)

Control Point	Order of Accuracy	Geographic Coordinates (WGS 84)			UTM ZONE 51 N		
		Latitude	Longitude	Ellipsoidal Height (Meter)	Northing (m)	Easting (m)	BM Ortho (m)
ZGN-164	2nd Order GCP	8°03'58.80475" N	122°48'08.60698" E	296.130	891636.530	478227.123	229.325
ZN-143	1st Order BM	8°08'21.39646" N	122°51'28.86114" E	77.323	899697.912	484358.928	11.526
UP_PAT-1	Established	8°06'00.79142" N	122°37'19.54470" E	76.488	895396.684	458365.200	10.835
UP_SAL-1	Established	8°06'20.46964" N	122°45'09.85390" E	76.124	895989.921	472758.821	10.080

4.5 Cross-section and Bridge As-Built Survey and Water Level Marking

Network Adjustment

Cross-section and as-built surveys were conducted on April 9, 2016 at the upstream side of Polandok Palandok Bridge in Brgy., Ramon Magsaysay, Municipality of Salug, Province of Zamboanga del Norte, as shown in Figure C-9 Figure 37. A Nikon® Total Station was utilized for this survey as shown in Figure C-1038.



Figure 37. Figure C-9 Polandok Bridge facing upstream



Figure 38. As-built survey of Polandok Bridge

The cross-sectional line of Polandok Palandok Bridge is about 362.221 m with two hundred fifteen (215) cross-sectional points using the control points UP_PAL-1 and UP_PAL-2 as the GNSS base stations. The cross-section diagram, location map, and the bridge data form are shown in Figure C-1139 to Figure C-13 Figure 41.

No bridge cross-section or bridge points checking data were gathered for Polandok Palandok Bridge because the contractor's data passed the quality assessment.

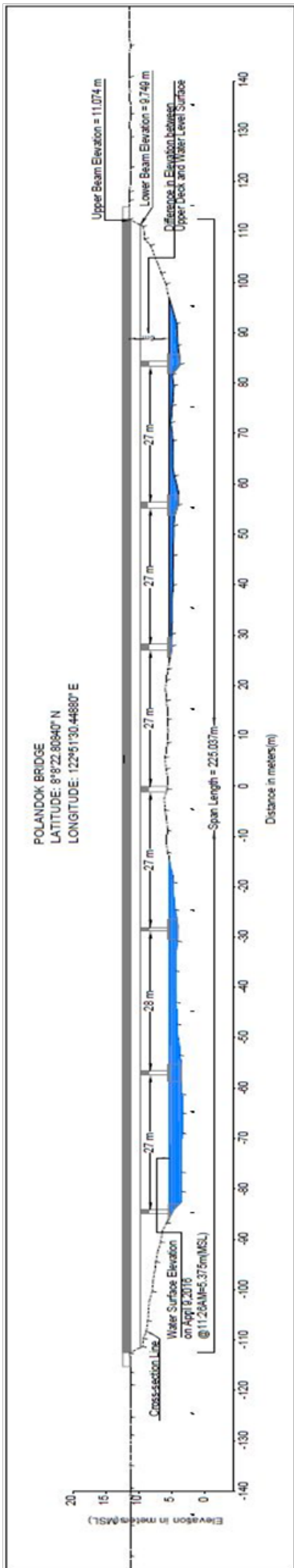


Figure 39. PolandokPalandok Bridge Cross-section Diagram

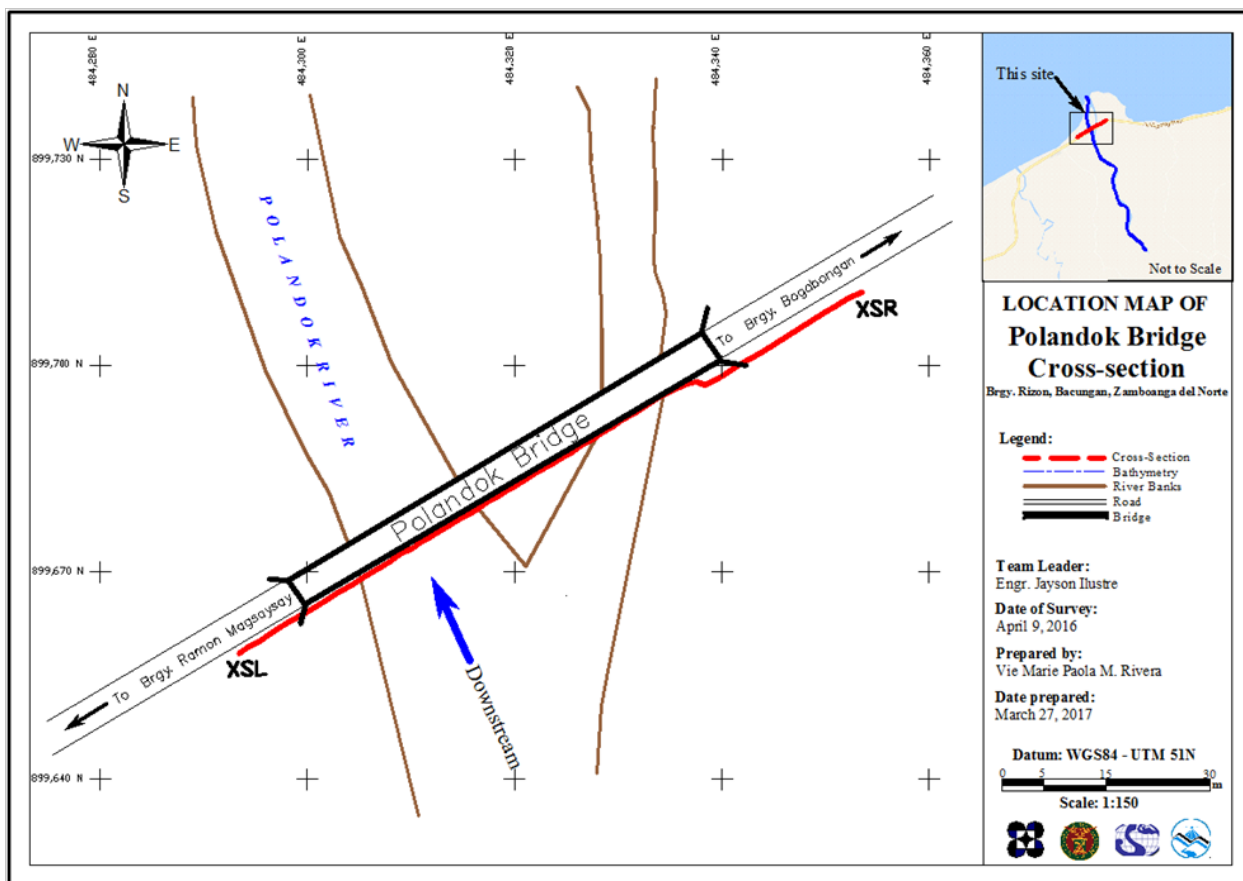


Figure 40. Polandok Bridge Location Map

Bridge Data Form

Bridge Name: POLANDOK BRIDGE

River Name: POLANDOK RIVER

Location (Brgy, City, Region): Bacungan, Zamboanga Del Norte

Survey Team: Jayson Ilustre Ryan Antonio

Date and Time: April 9, 2016, 11:26 A.M.

Flow Condition: low normal high Deck/Scam

Weather Condition: fair rainy

2.768 225.037 m 1.850 2.793 11.445 m

- Legend:
- BA = Bridge Approach
 - P = Pier
 - Ab = Abutment
 - D = Deck
 - WL = Water Level/Surface
 - MSL = Mean Sea Level
 - = Measurement Value

Line Segment	Measurement (m)	Remarks
1. BA1-BA2	2.773	
2. BA2-BA3	225.037 m	
3. BA3-BA4	2.793 m	
4. BA1-Ab1	5.86 m	
5. Ab2-BA4	7.271 m	
6. Deck/beam thickness	1.850 m	
7. Deck elevation	11.445 m	

Note: Observer should be facing downstream

Figure 41. PolandokPalandok Bridge Data Sheet

Water surface elevation of Polandok Palandok River was determined by a Nikon® Total Station on April 9, 2016 at 11:26 AM at Polandok Palandok Bridge area with a value of 5.375 m in MSL as shown in Figure C-11Figure 39. This was translated into marking on the bridge's pier as shown in Figure C-14Figure 42. The marking will serveserved as reference for flow data gathering and depth gauge deployment of the partner HEI responsible for Polandok Palandok River, the Ateneo de Zamboanga University.



Figure 42. Water-level markings on Polandok Palandok Bridge

4.6 Validation Points Acquisition Survey

Validation points acquisition survey was conducted by DVBC from August 27, 2016 using a survey grade GNSS Rover receiver, Trimble® SPS 882, mounted on a range pole which was attached on the side of the vehicle as shown in Figure 43.Figure C-15. It was secured with cable ties and ropes to ensure that it was horizontally and vertically balanced. The antenna height was 1.278 m and measured from the ground up to the bottom of the quick release of the GNSS Rover receiver. The PPK technique utilized for the conduct of the survey was set to continuous topo mode with ZN-143 occupied as the GNSS base station in the conduct of the survey.



Figure 43. Validation points acquisition survey set-up for Polandok River

The survey started from Brgy. Bacungan, Municipality of Bacungan, Zamboanga del Norte going west along the national highway covering thirteen (13) barangays in three (3) municipalities, namely, the municipalities of Bacungan, Salug, and Liloy, and ending in Brgy. Kayok, Municipality of Liloy, Zamboanga del Norte. The survey gathered a total of 993 points with approximate length of 24 km using ZN-143 as GNSS base station for the entire extent of validation points acquisition survey as illustrated in the map in Figure 44.

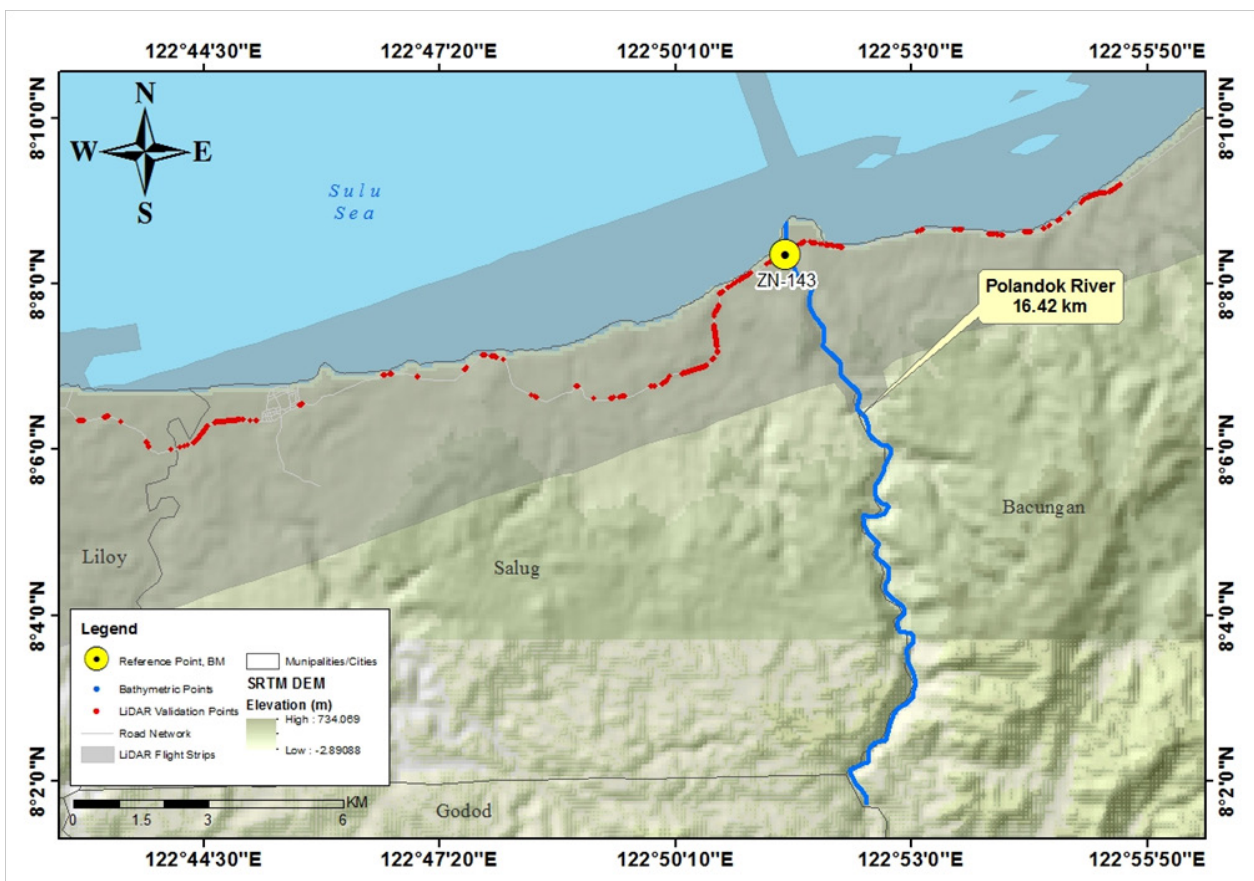


Figure 44. Validation points acquisition covering the Polandok River Basin Area

4.7 River Bathymetric Survey

Bathymetric survey was executed on April 15, 18, 19, 23, 28, and May 2, 2016 at Polandok Palandok River using a Nikon® Total Station as illustrated in Figures C-17Figure 45. The survey started from the mouth of the river at Brgy. Rizon, Bacungan, Zamboanga del Norte with coordinates 8°9'11.80343"N, 122°55'30.86879"E and ended in Brgy. Morob, Bacungan, Zamboanga Del Norte as well, with coordinates 8°6'20.92905"N, 122°43'00.12187"E. The control points UP_POL-1 and UP-POL-2 were used as GNSS base stations all throughout the entire survey.



Figure 45. Manual bathymetric survey at Polandok River using Nikon® Total Station

The bathymetric survey for Polandok Palandok River gathered a total of 4,806 points covering 16.484 km of the river traversing Brgy. Rizon to Brgy. Morob, along the boundary between the municipalities of Bacungan and Salug. A CAD drawing was also produced to illustrate the riverbed profile of Palandok Palandok River. As shown in Figure C-20Figure 48, elevation drop of 148.22 m of the river bed was recorded. An elevation drop of 178.36 m was observed within the distance of approximately 16.484 km.

Gathering of random points for the checking of ABSD's bathymetric data was performed by DVBC on August 21-31, 2016 using an Ohmex™ Single Beam Echo Sounder and Trimble® SPS 882 GNSS PPK survey technique. A map showing the DVC bathymetric checking points is shown in Figure C-1947.

Linear square correlation (R^2) and RMSE analysis were performed on the two (2) datasets. The linear square coefficient range is determined to ensure that the submitted data of the contractor is within the accuracy standard of the project which is ± 20 cm and ± 10 cm for horizontal and vertical, respectively. The R^2 value must be within 0.85 to 1. An R^2 approaching 1 signifies a strong correlation between the vertical (elevation values) of the two datasets. A computed R^2 value of 0.87 was obtained was obtained by comparing the data of the contractor and DVBC; signifying a strong correlation between the two (2) datasets.

In addition to the Linear Square correlation, Root Mean Square (RMSE) analysis is also performed in order to assess the difference in elevation between the DVBC checking points and the contractor's. The RMSE value should only have a maximum radial distance of 5 m and the difference in elevation within the radius of 5 meters should not be beyond 0.50 m. For the bridge cross-section data, a computed value of 0.154 which is within the accuracy required by the program.

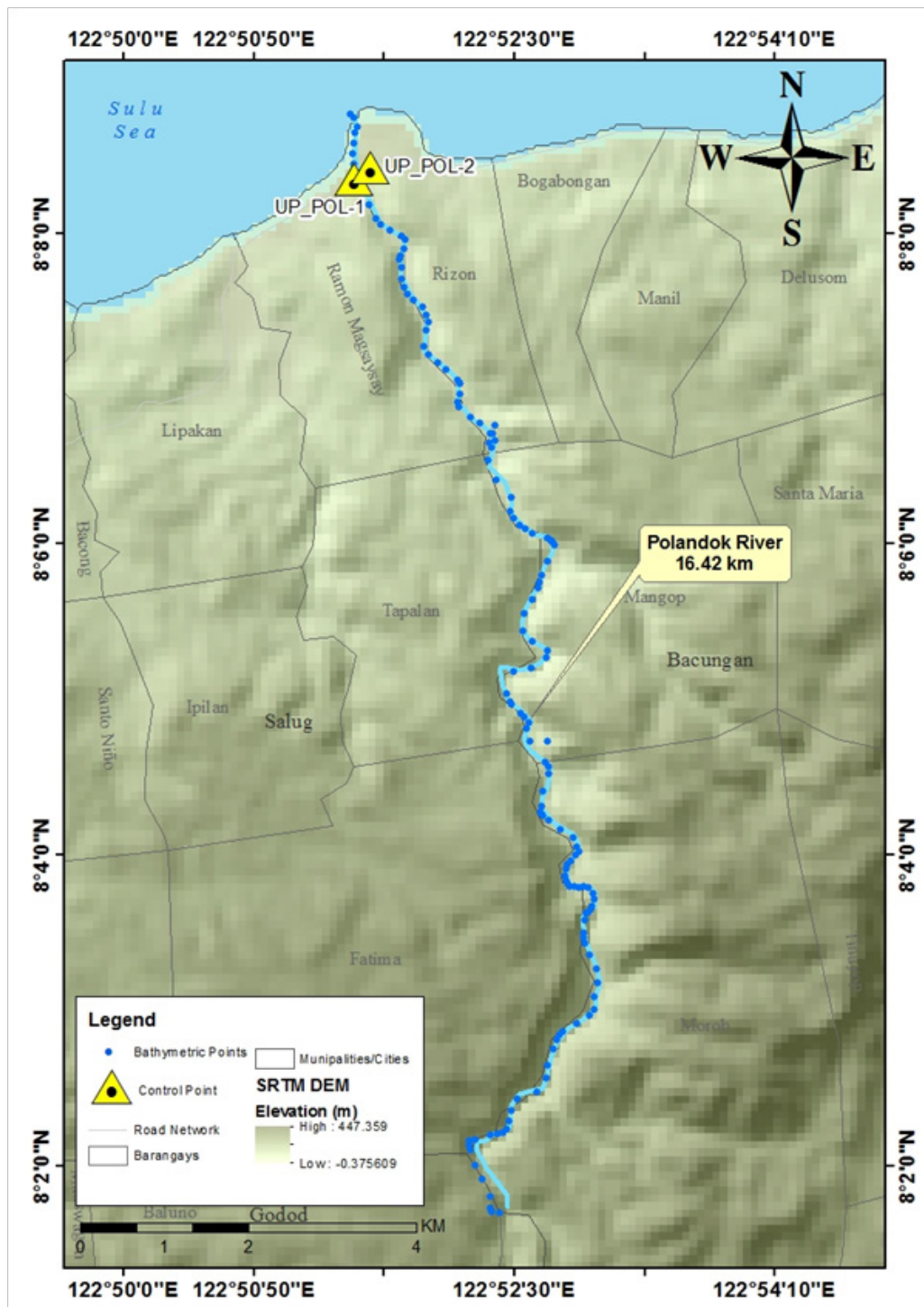


Figure 46. Figure C-18 Bathymetric survey of Polandok River

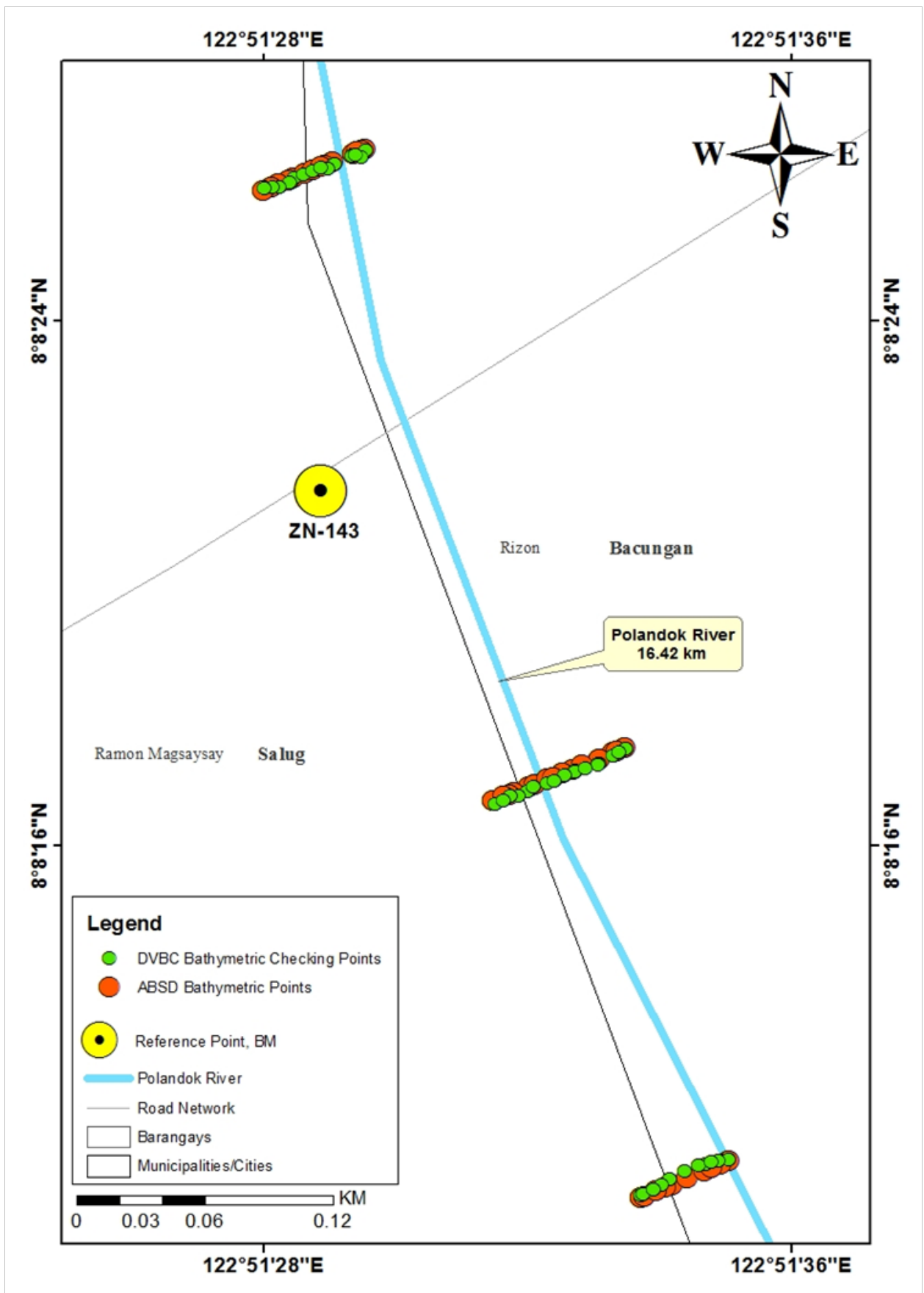


Figure 47. Quality checking points gathered along Polandok River by DVBC

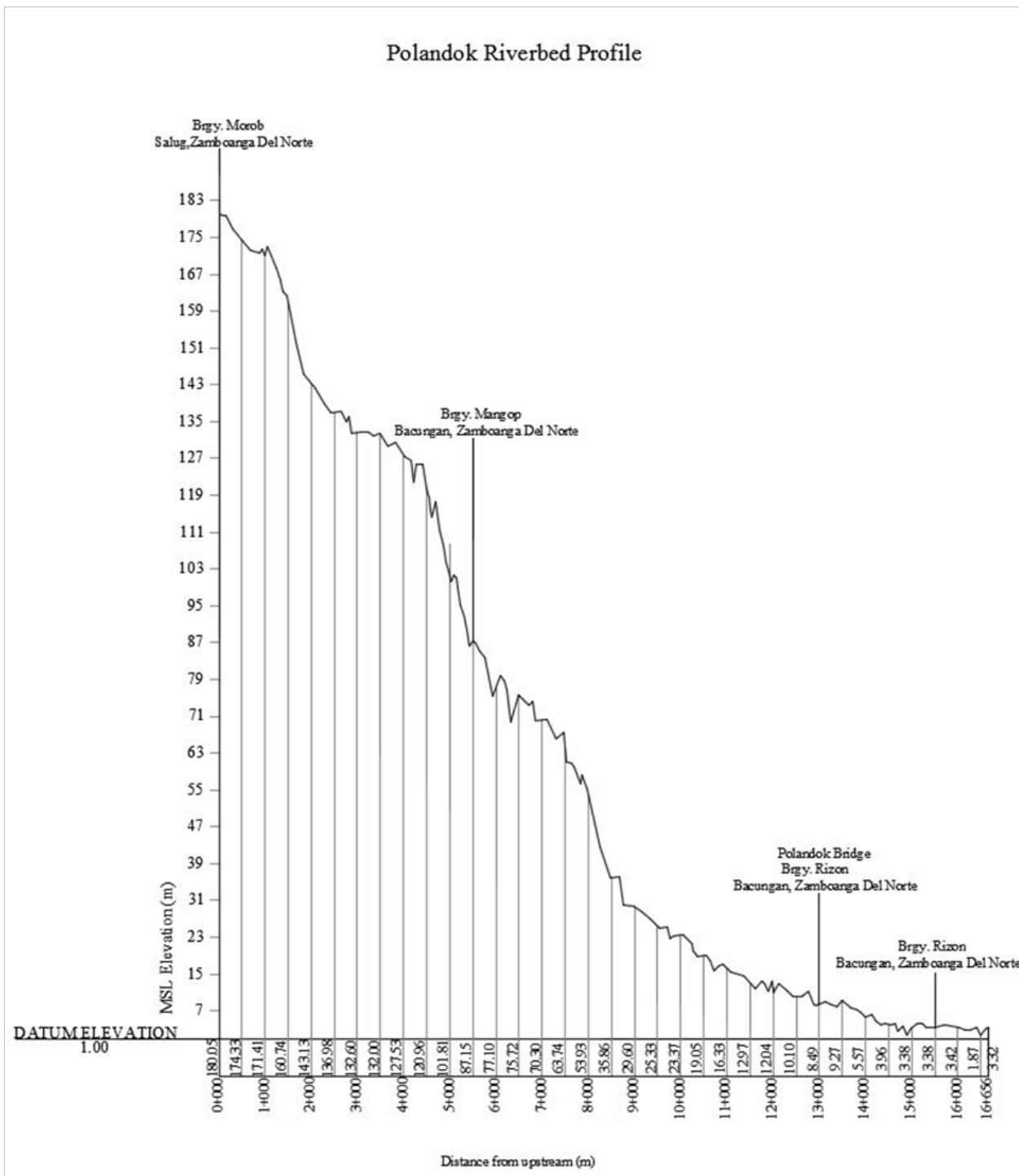


Figure 48. Figure C-20 Polandok Riverbed Profile

CHAPTER 5: FLOOD MODELING AND MAPPING

Dr. Alfredo Mahar Lagmay, Christopher Uichanco, Sylvia Sueno, Marc Moises, Hale Ines, Miguel del Rosario, Kenneth Punay, Neil Tingin

The methods applied in this chapter were based on the DREAM methods manual (Lagmay et al., 2014) and further enhanced and updated in Paringit et al. (2017).

5.1 Data Used for Hydrologic Modeling

5.1.1 Hydrometry and Rating Curves

Components and data that affect the hydrologic cycle of the river basin were monitored, collected, and analyzed. Rainfall, water level, and flow in a certain period of time, which may affect the hydrologic cycle of the Batu Palandok River Basin were monitored, collected, and analyzed. These include the rainfall, water level, and flow in a certain period of time.

5.1.2 Precipitation

Precipitation data was taken from a manually read Rain Gauge at Brgy. Morob, Leon B. Postigo, Zamboanga del Norte ($8^{\circ} 4' 33.10''$ N, $122^{\circ} 22' 44.00''$ E). (Figure 491). The precipitation data collection started from July 27, 2016 at 09:00 AM to July 28, 2016 at 7:00AM with 10 minutes recording interval.

The total precipitation for this event in Brgy. Morob was 100.8 mm. It has a peak rainfall of 18.4 mm. on July 27, 2016 at 4:30 PM. The lag time between the peak rainfall and discharge is 2 hours.

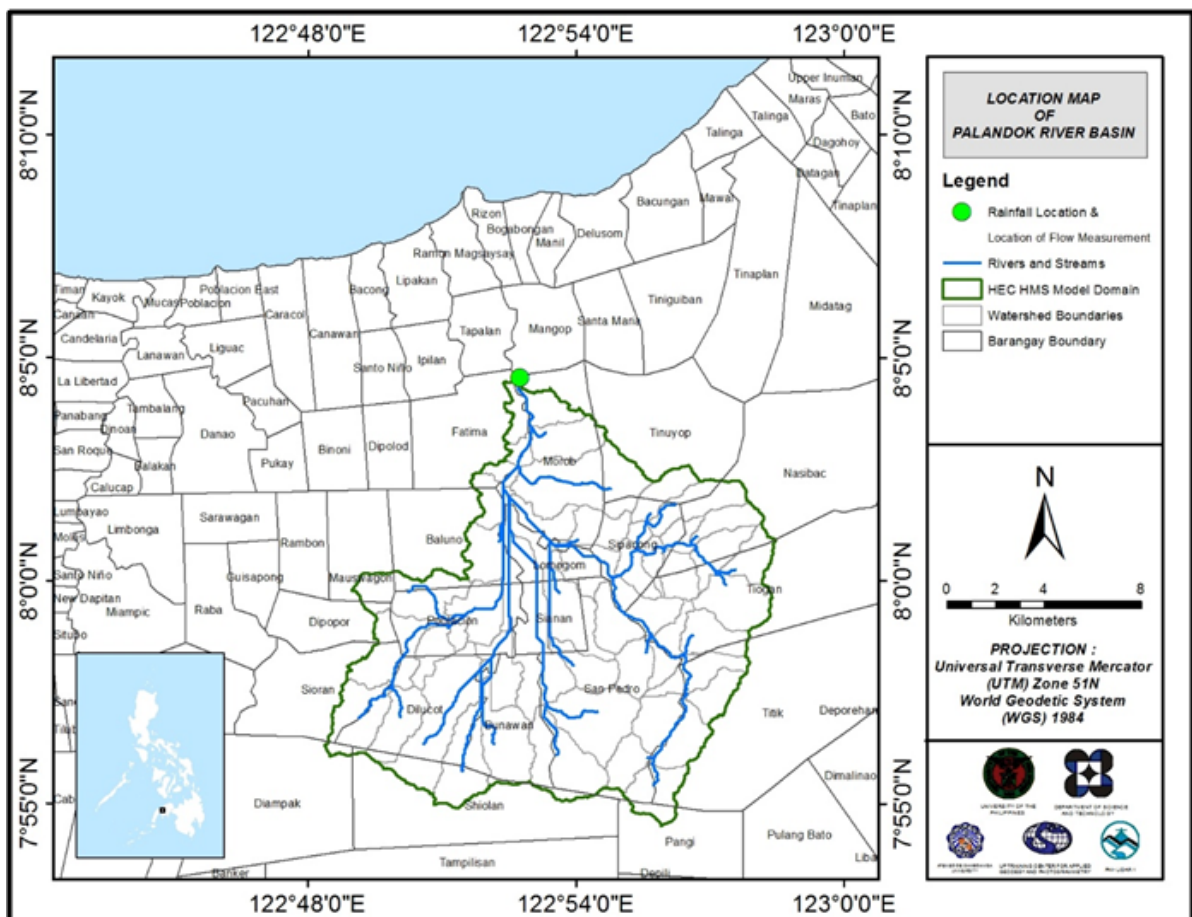


Figure 49. The location map of Palandok HEC-HMS model used for calibration

5.1.3 Rating Curves and River Outflow

A rating curve was developed at Brgy. Morob, Leon B. Postigo, Zamboanga del Norte (7° 59' 30.43" N, 122° 52' 49.33" E). It gives the relationship between the observed water levels at Brgy. Morob and outflow of the watershed at this location.

For Brgy. Morob, the rating curve is expressed as $Q = 2E-136e^{2.2412h}$ as shown in Figure 513.

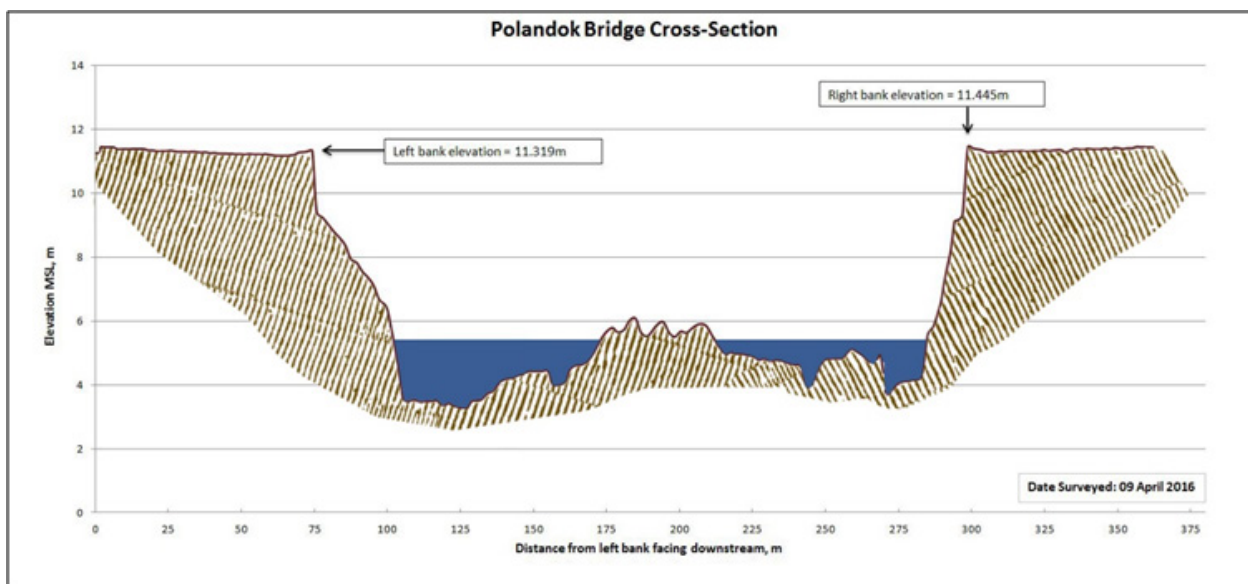


Figure 50. Cross-Section Plot of Brgy. Morob Bridge

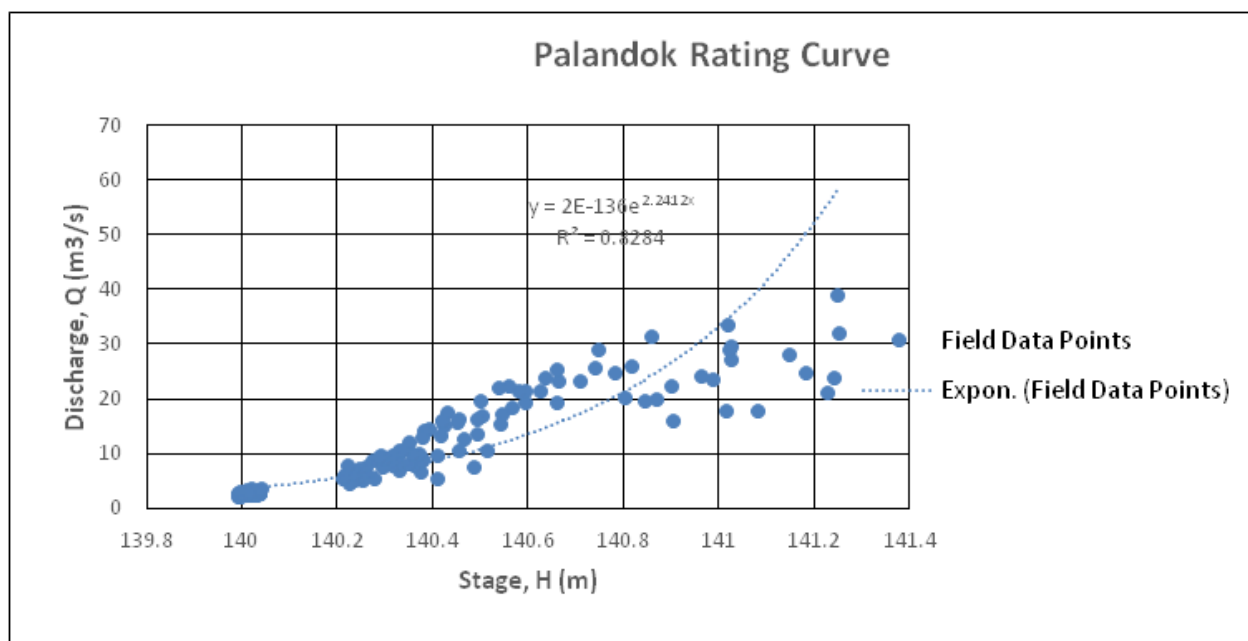


Figure 51. Rating Curve at Brgy. Morob, Leon B. Postigo, Zamboanga del Norte

This rating curve equation was used to compute the river outflow at Brgy. Morob for the calibration of the HEC-HMS model shown in Figure 351. Peak discharge is 39 cubic meters per second at 6:30 PM, July 27, 2016.

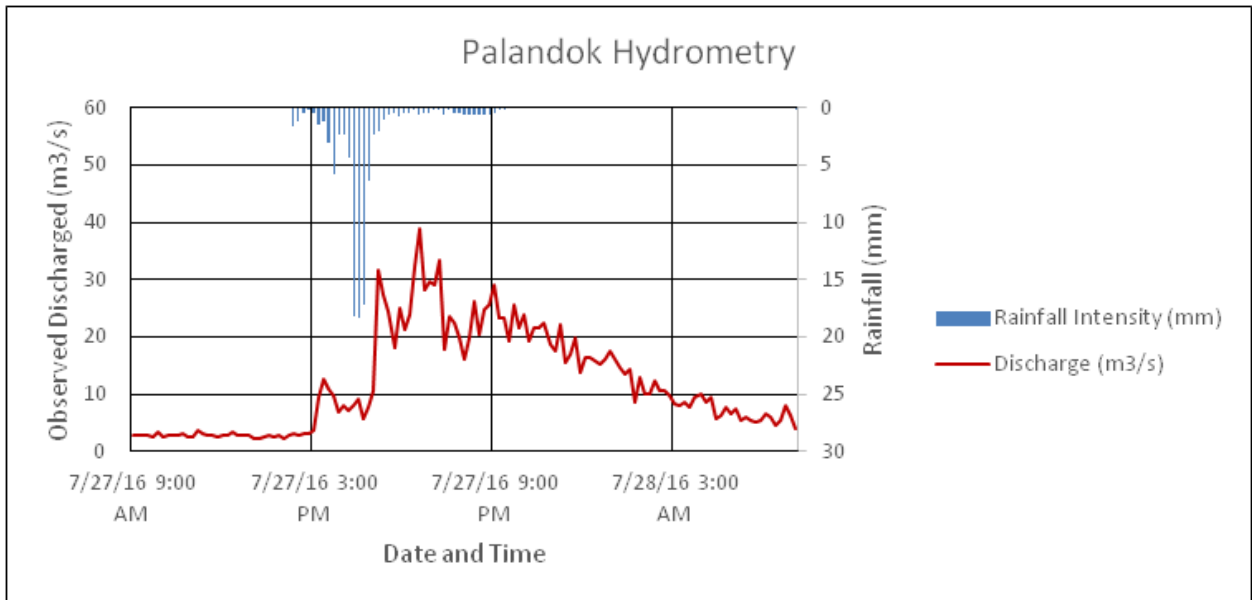


Figure 52. Rainfall and outflow data at Brgy. Morob used for modeling

5.2 RIDF Station

The Philippines Atmospheric Geophysical and Astronomical Services Administration (PAGASA) computed Rainfall Intensity Duration Frequency (RIDF) values for the Zamboanga City Rain Gauge. The RIDF rainfall amount for 24 hours was converted to a synthetic storm by interpolating and re-arranging the value in such a way certain peak value will be attained at a certain time. This station was chosen based on its proximity to the Palandok watershed. The extreme values for this watershed were computed based on a 59-year record.

Table 30. RIDF values for Zamboanga City Rain Gauge computed by PAGASA

COMPUTED EXTREME VALUES (in mm) OF PRECIPITATION									
T (yrs)	10 mins	20 mins	30 mins	1 hr	2 hrs	3 hrs	6 hrs	12 hrs	24 hrs
2	15.5	23.3	28.4	36.9	45.6	50.7	60	66.1	77.3
5	21.4	31.6	38.3	50.4	61.2	38.2	82.5	91.5	107.8
10	25.3	37.1	44.8	59.4	71.6	79.8	97.5	108.3	127.9
15	27.5	40.2	48.5	64.4	77.4	86.4	105.9	117.8	139.3
20	29	42.3	51.1	68	81.5	91	111.8	124.4	147.3
25	30.2	44	53.1	70.7	84.7	94.5	116.3	129.5	153.4
50	33.9	49.1	59.2	79.1	94.4	105.4	130.4	145.3	172.3
100	37.5	54.2	65.3	87.4	104	116.2	144.3	161	191.1

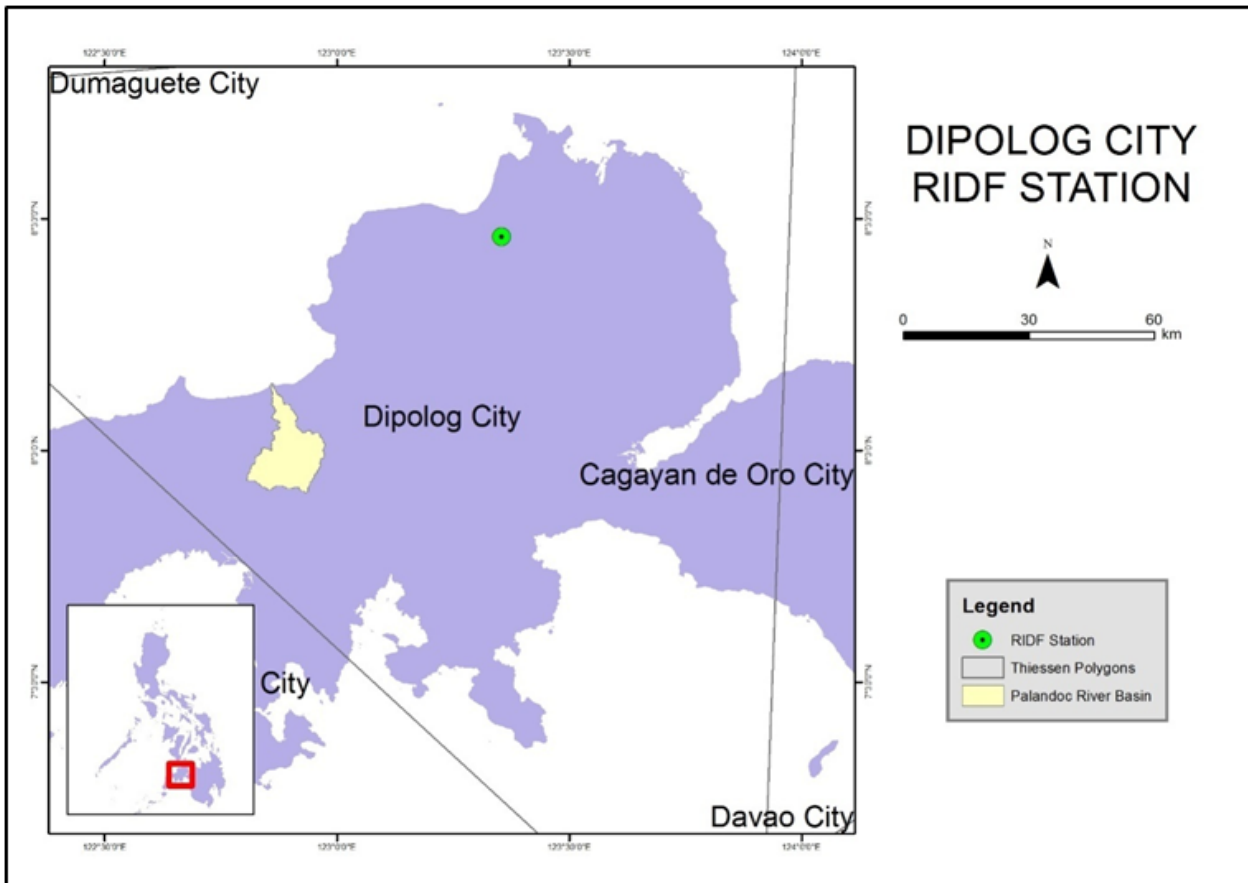


Figure 53. Dipolog City RIDF location relative to Palandok River Basin

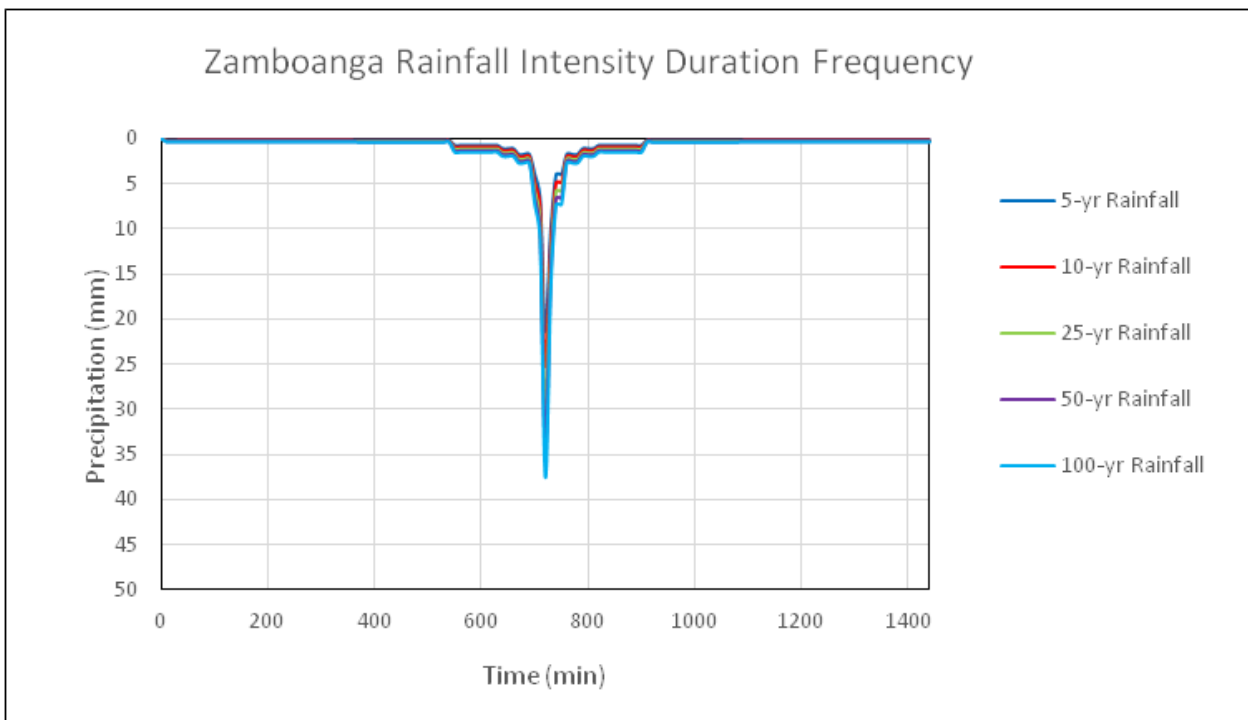


Figure 54. Synthetic storm generated for a 24-hr period rainfall for various return periods

5.3 HMS Model

The soil dataset was taken from and generated by the Bureau of Soils and Water Management (BSWM) under the Department of Agriculture. The soil shapefile was taken on 2004 from the Bureau of Soils; this is under the Department of Environment and Natural Resources Management (DENR). The land cover shape file is from the National Mapping and Resource information Authority (NAMRIA). The soil and land cover of the Palandok River Basin are shown in Figures 59 55 and 6056, respectively.

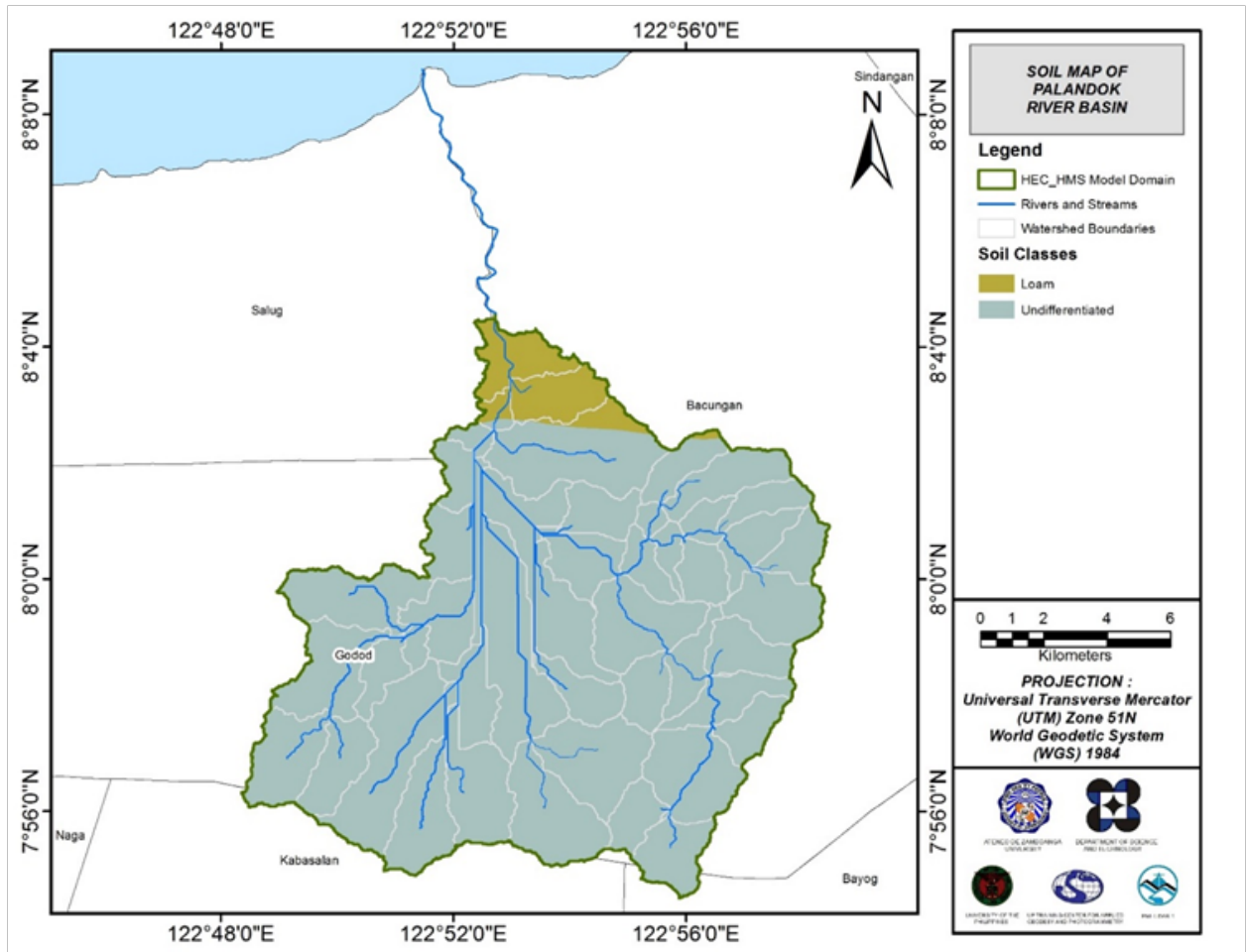


Figure 55. Soil Map of Palandok River Basin

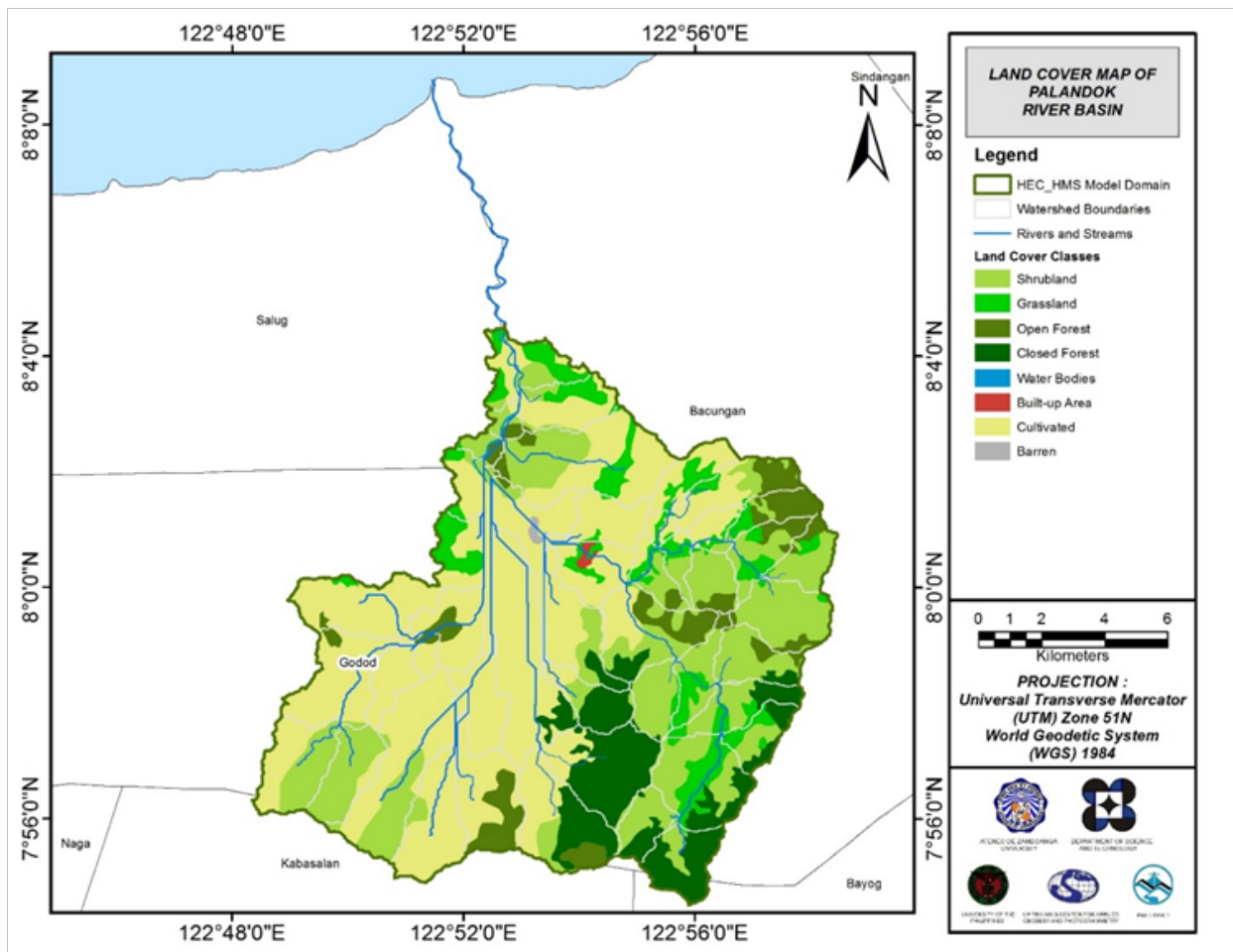


Figure 56. Land Cover Map of Palandok River Basin

For Palandok, the soil classes identified were loam and undifferentiated mountain soil. The land cover types identified were shrubland, grassland, open and closed canopy forests, built-up areas and cultivated areas.

Figure 57. [insert Slope Map]

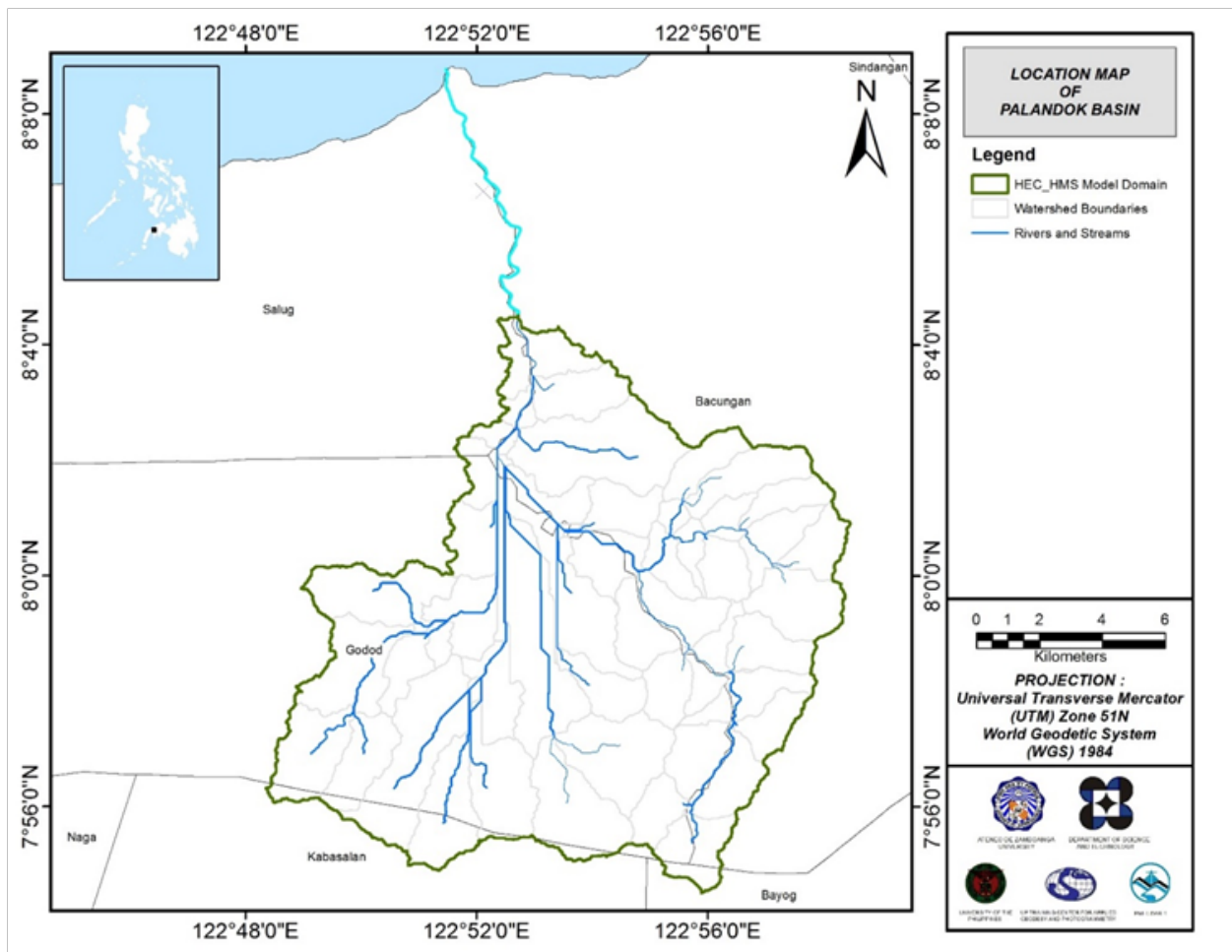


Figure 58. Stream delineation map of Palandok river basin

Using the SAR-based DEM, the Palandok basin was delineated and further subdivided into subbasins. The model consists of 51 sub basins, 25 reaches, and 25 junctions as shown in Figure 597. The main outlet is at Brgy. Morob, Leon B. Postigo, Zamboanga del Norte.

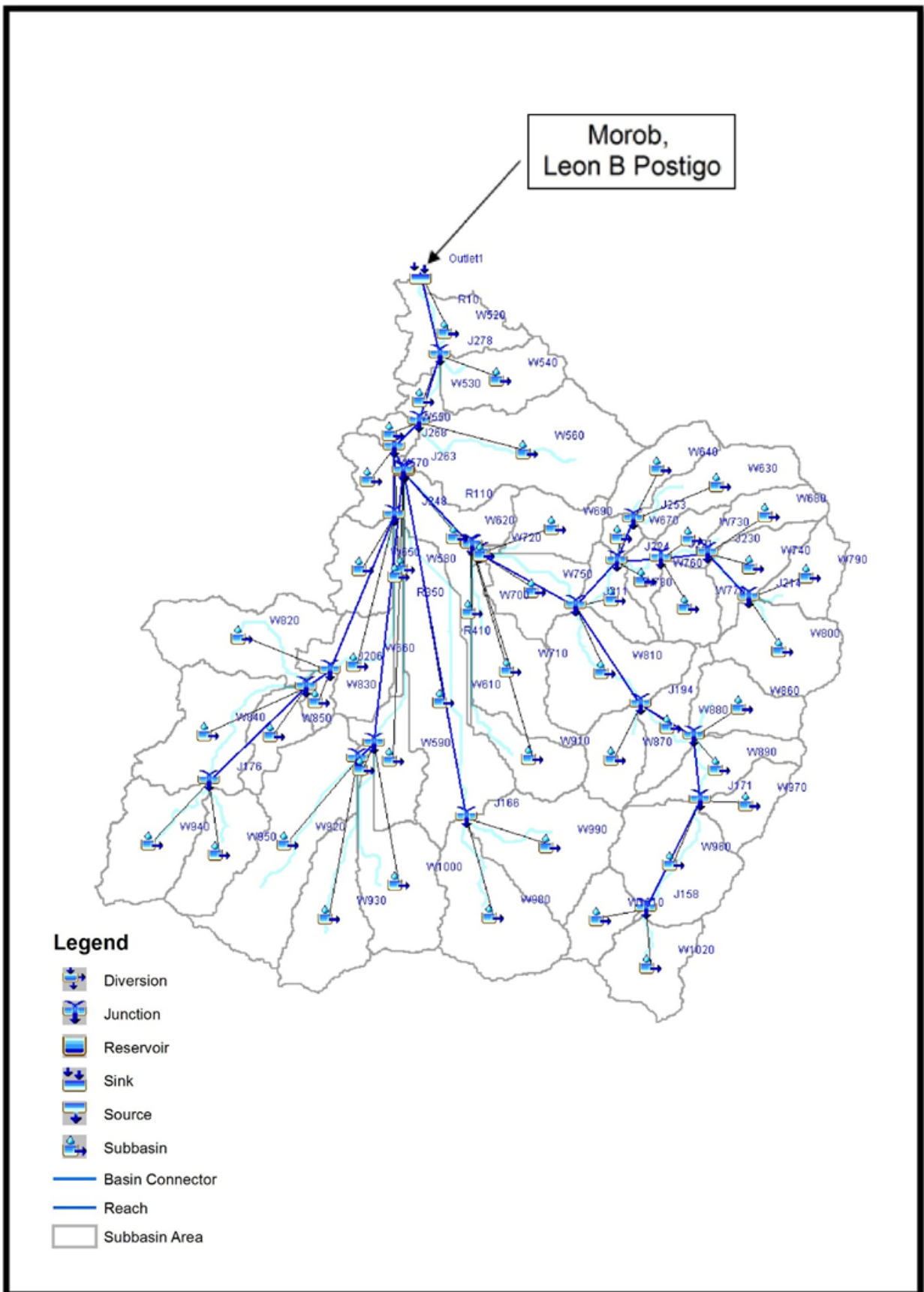


Figure 59. The Palandok river basin model generated using HEC-HMS

5.4 Cross-section Data

Riverbed cross-sections of the watershed are crucial in the HEC-RAS model setup. The cross-section data for the HEC-RAS model was derived using the LiDAR DEM data. It was defined using the Arc GeoRAS tool and was post-processed in ArcGIS.

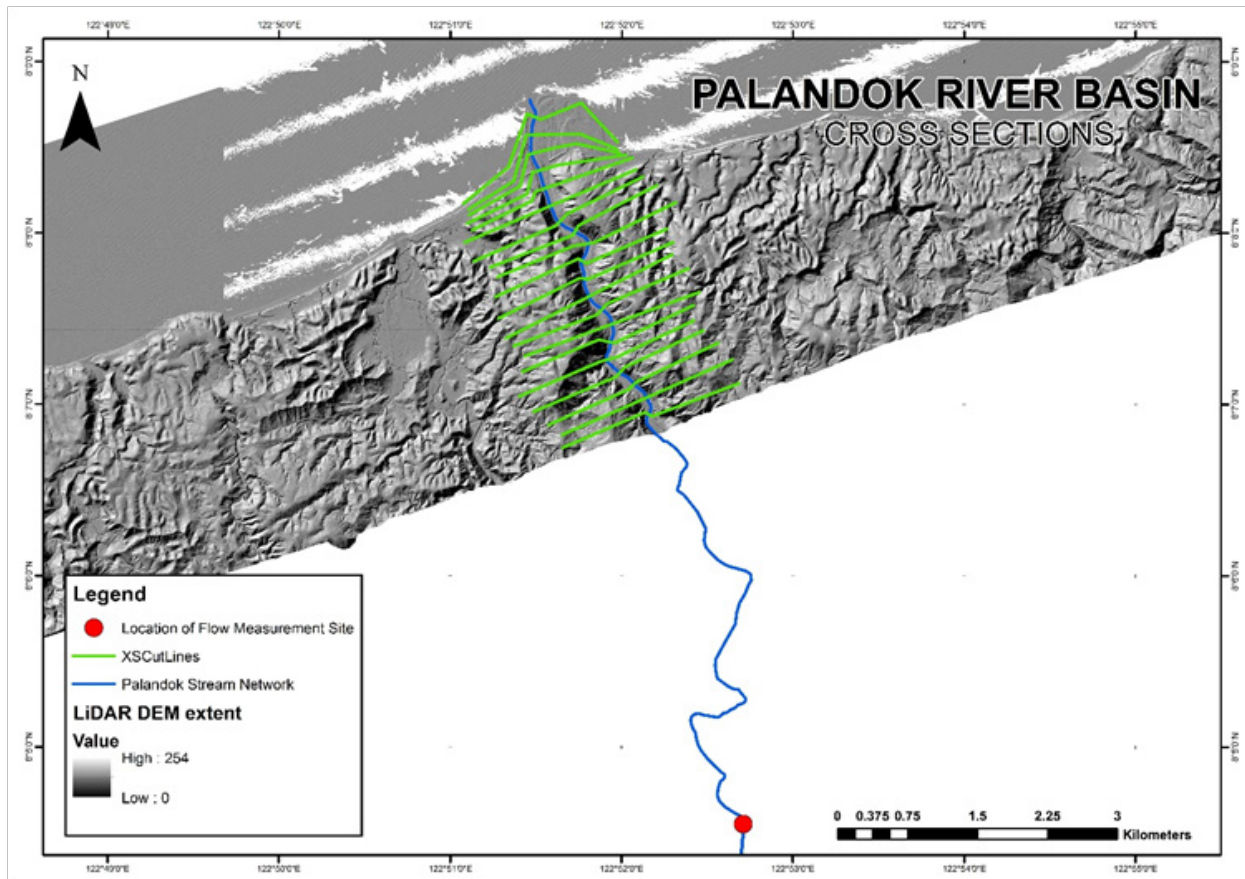


Figure 60. River cross-section of Palandok River generated through Arcmap HEC GeorAS tool

5.5 Flo 2D Model

The automated modelling process allows for the creation of a model with boundaries that are almost exactly coincidental with that of the catchment area. As such, they have approximately the same land area and location. The entire area is divided into square grid elements, 10 meter by 10 meter in size. Each element is assigned a unique grid element number which serves as its identifier, then attributed with the parameters required for modelling such as x-and y-coordinate of centroid, names of adjacent grid elements, Manning coefficient of roughness, infiltration, and elevation value. The elements are arranged spatially to form the model, allowing the software to simulate the flow of water across the grid elements and in eight directions (north, south, east, west, northeast, northwest, southeast, southwest). Based on the elevation and flow direction, it is seen that the water will generally flow from the southeast of the model to the northwest, following the main channel. As such, boundary elements in those particular regions of the model are assigned as inflow and outflow elements respectively.

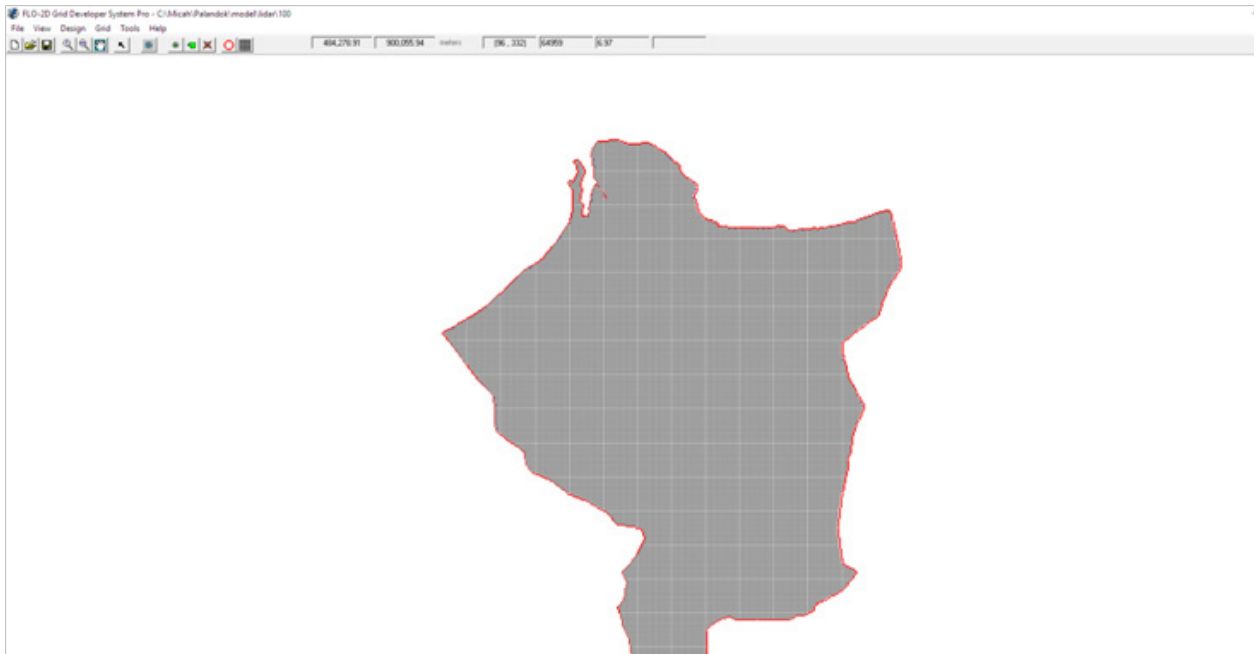


Figure 61. Screenshot of subcatchment with the computational area to be modeled in FLO-2D GDS Pro

The simulation is then run through FLO-2D GDS Pro. This particular model had a computer run time of 4.57031 hours. After the simulation, FLO-2D Mapper Pro is used to transform the simulation results into spatial data that shows flood hazard levels, as well as the extent and inundation of the flood. Assigning the appropriate flood depth and velocity values for Low, Medium, and High creates the following food hazard map. Most of the default values given by FLO-2D Mapper Pro are used, except for those in the Low hazard level. For this particular level, the minimum h (Maximum depth) is set at 0.2 m while the minimum vh (Product of maximum velocity (v) times maximum depth (h)) is set at 0 m²/s.

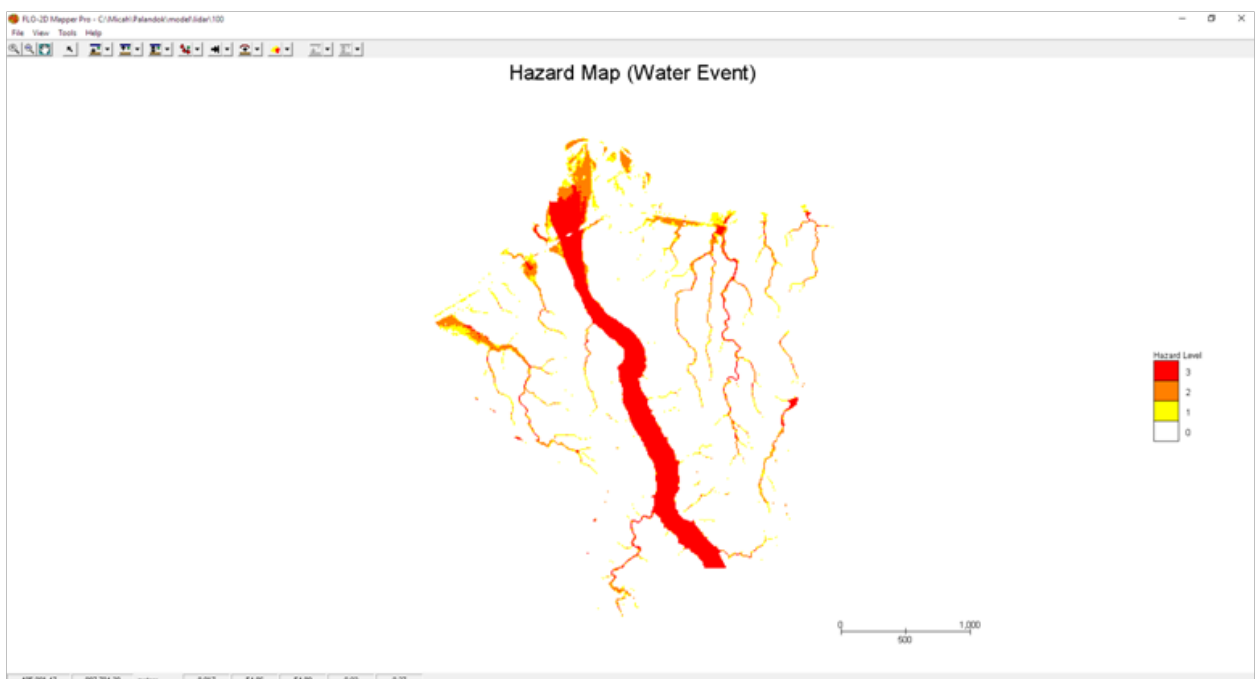


Figure 62. Generated 100-year rain return hazard map from FLO-2D Mapper

The creation of a flood hazard map from the model also automatically creates a flow depth map depicting the maximum amount of inundation for every grid element. The legend used by default in Flo-2D Mapper is not a good representation of the range of flood inundation values, so a different legend is used for the layout. In this particular model, the inundated parts cover a maximum land area of 6818900.00 m².

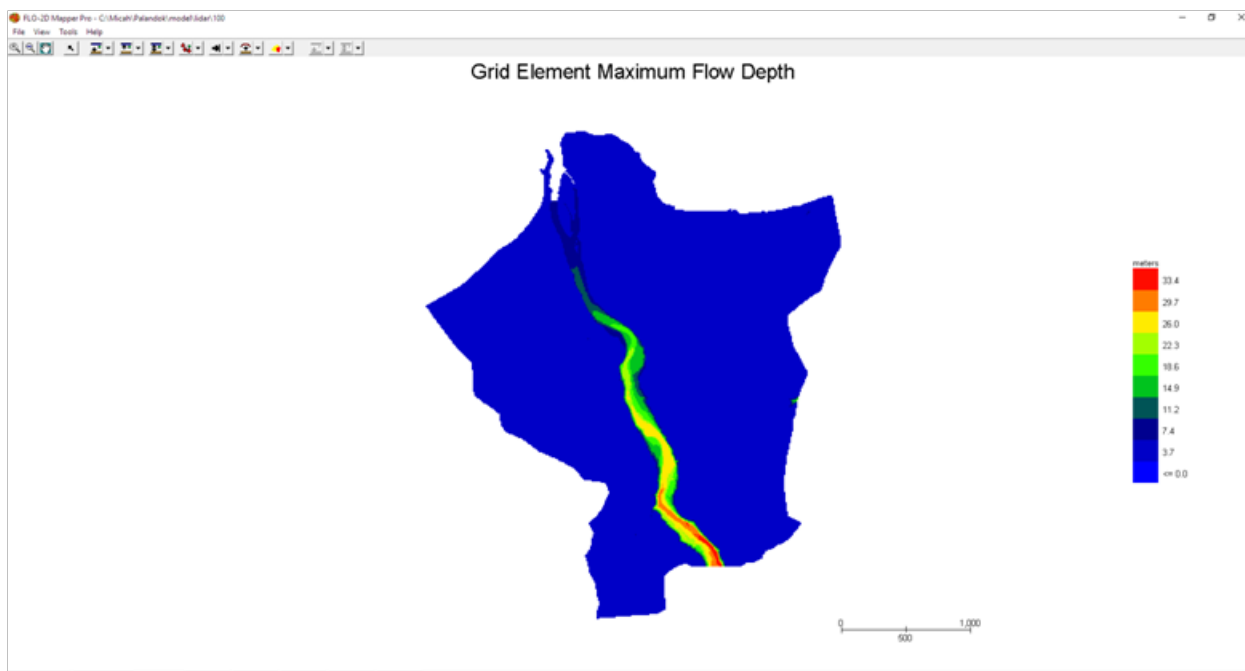


Figure 63. Generated 100-year rain return flow depth map from FLO-2D Mapper

There is a total of 32116000.86 m³ of water entering the model. Of this amount, 1407591.65 m³ is due to rainfall while 30708409.22 m³ is inflow from other areas outside the model. 512356.75 m³ of this water is lost to infiltration and interception, while 399268.01 m³ is stored by the flood plain. The rest, amounting up to 31204376.06 m³, is outflow.

Discharge data using Dr. Horritts's recommended hydrologic method

The river discharge values for the river entering the floodplain are shown in and the peak values are summarized in Table 314.

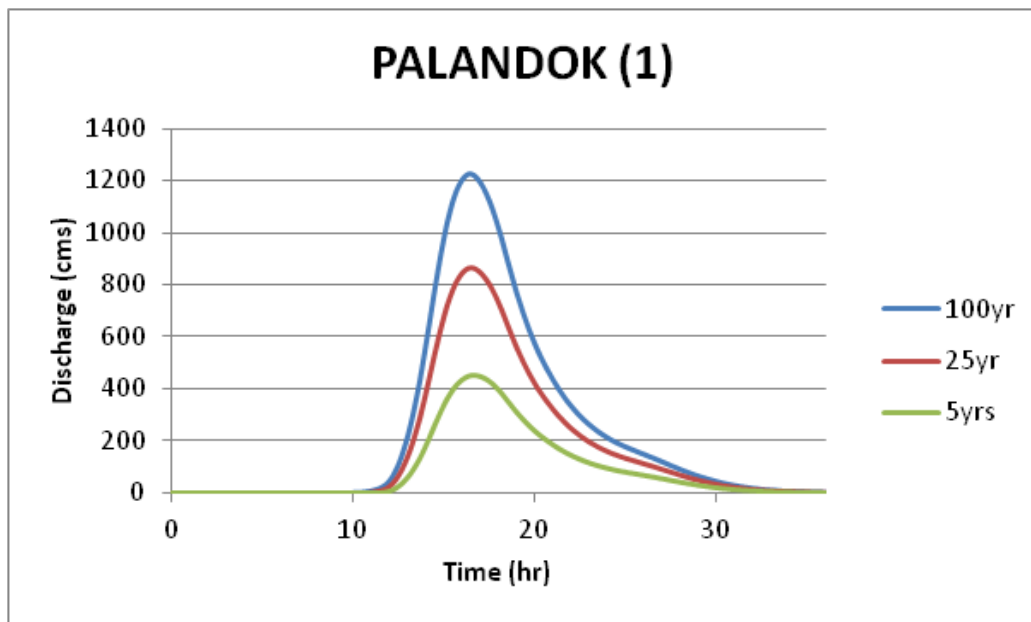


Figure 64. Palandok Rriver (1) generated discharge using 5-, 25-, and 100-year Dipolog rainfall intensity-duration-frequency (RIDF) in HEC-HMS

Table 31. Summary of Palandok river (1) discharge generated in HEC-HMS

RIDF Period	Peak discharge (cms)	Time-to-peak
100-Year	1223.3	16 hours, 30 minutes
25-Year	868.6	16 hours, 30 minutes
5-Year	455	16 hours, 40 minutes

Table 32. Validation of river discharge estimates

Discharge Point	QMED(SCS), cms	QBANKFUL, cms	QMED(SPEC), cms	VALIDATION	
				Bankful Discharge	Specific Discharge
Palandok (1)	400.400	379.442	521.954	Pass	Pass

The HEC-HMS river discharge estimates were able to satisfy the conditions for validation using the bankful method and specific discharge methods. The passing values are based on theory but are supported using other discharge computation methods so they were good to use for flood modeling. These values will need further investigation for the purpose of validation. It is therefore recommended to obtain actual values of the river discharges for higher-accuracy modeling.

5.6 Results of HMS Calibration

Table 7. After calibrating the Palandok HEC-HMS river basin model, its accuracy was measured against the observed values. Figure 12 65 shows the comparison between the two discharge data.

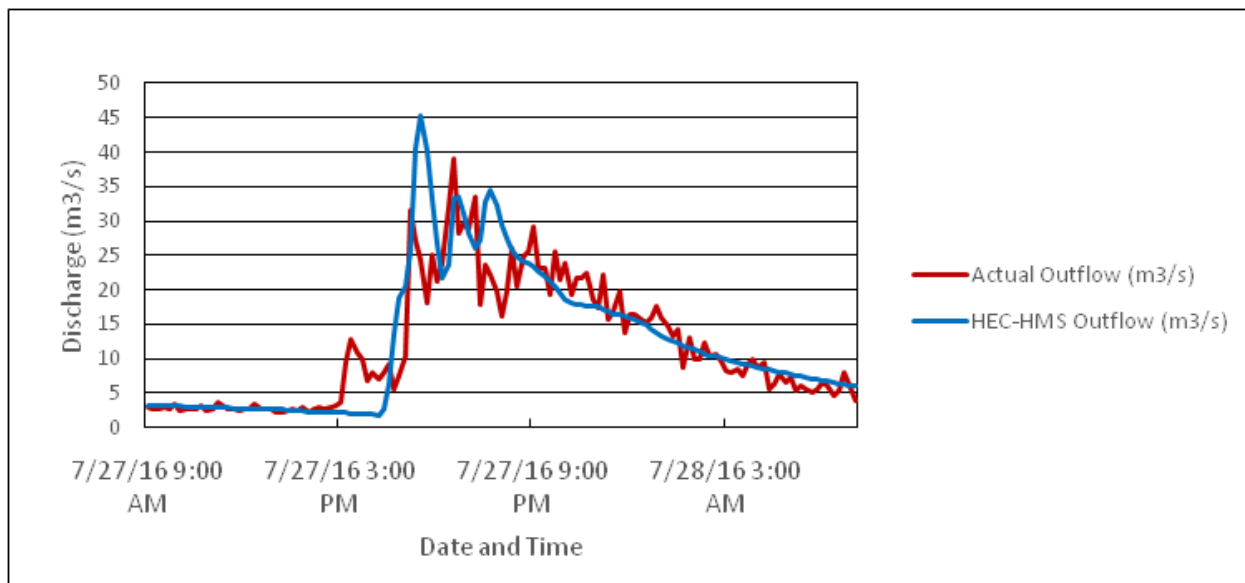


Figure 65. Outflow Hydrograph of Palandok produced by the HEC-HMS model compared with observed outflow

Table 9. Enumerated in Table 33 are the adjusted ranges of values of the parameters used in calibrating the model.

Table 33. Range of Calibrated Values for Palandok

Hydrologic Element	Calculation Type	Method	Parameter	Range of Calibrated Values
Basin	Loss	SCS Curve Number	Initial Abstraction (mm)	22.66 – 101.27
			Curve Number	15.13 – 27.92
	Transform	Clark Unit Hydrograph	Time of Concentration (hr)	0.44 – 1.70
			Storage Coefficient (hr)	0.017 – 0.66
	Baseflow	Recession	Recession Constant	0.05
Ratio to Peak			0.1	
Reach	Routing	Muskingum-Cunge	Manning’s Coefficient	0.044

Initial abstraction defines the amount of precipitation that must fall before surface runoff. The magnitude of the outflow hydrograph increases as initial abstraction decreases. The range of values from 22.66mm to 101.27mm means that there is a minimal amount of infiltration or rainfall interception by vegetation.

Curve number is the estimate of the precipitation excess of soil cover, land use, and antecedent moisture. The range of curve numbers in this area is 15.13 – 27.92. The magnitude of the outflow hydrograph increases as curve number increases. For Palandok, the soil classes identified were loam and undifferentiated mountain soil. The land cover types identified were shrubland, grassland, open and closed canopy forests, built-up areas and cultivated areas.

Time of concentration and storage coefficient are the travel time and index of temporary storage of runoff in a watershed. The range of calibrated values from 0.017 hours to 1.70 hours determines the reaction time of the model with respect to the rainfall. The peak magnitude of the hydrograph also decreases when these parameters are increased.

Recession constant is the rate at which baseflow recedes between storm events and ratio to peak is the ratio of the baseflow discharge to the peak discharge. Recession constant of 0.05 indicates that the basin is not likely to quickly go back to its original discharge. Ratio to peak of 0.1 indicates a shallower receding limb of the outflow hydrograph.

Table 34. Summary of the Efficiency Test of Palandok HMS Model

RMSE	0.552075
r2	0.8284
NSE	0.695213
PBIAS	-3.27858
RSR	0.551759

The Root Mean Square Error (RMSE) method aggregates the individual differences of these two measurements. It was computed as 0.552075 (m³/s).

The Pearson correlation coefficient (r²) assesses the strength of the linear relationship between the observations and the model. This value being close to 1 corresponds to an almost perfect match of the observed discharge and the resulting discharge from the HEC HMS model. Here, it measured 0.8284.

The Nash-Sutcliffe (E) method was also used to assess the predictive power of the model. Here the optimal value is 1. The model attained an efficiency coefficient of 0.695213.

A positive Percent Bias (PBIAS) indicates a model’s propensity towards under-prediction. Negative values indicate bias towards over-prediction. Again, the optimal value is 0. In the model, the PBIAS is -3.27858.

The Observation Standard Deviation Ratio, RSR, is an error index. A perfect model attains a value of 0 when the error in the units of the valuable a quantified. The model has an RSR value of 0.551759.

5.7 Calculated Outflow hydrographs and Discharge Values for different Rainfall Return Periods

5.7.1 Hydrograph using the Rainfall Runoff Model

The summary graph (Figure 1266) shows the Palandok outflow using the Zamboanga City Rainfall Intensity-Duration-Frequency (RIDF) curves (RIDF) in 5 different return periods (5-year, 10-year, 25-year, 50-year, and 100-year rainfall time series) based on the Philippine Atmospheric Geophysical and Astronomical Services Administration (PAG-ASA) data. The simulation results reveal significant increase in outflow magnitude as the rainfall intensity increases for a range of durations and return periods.

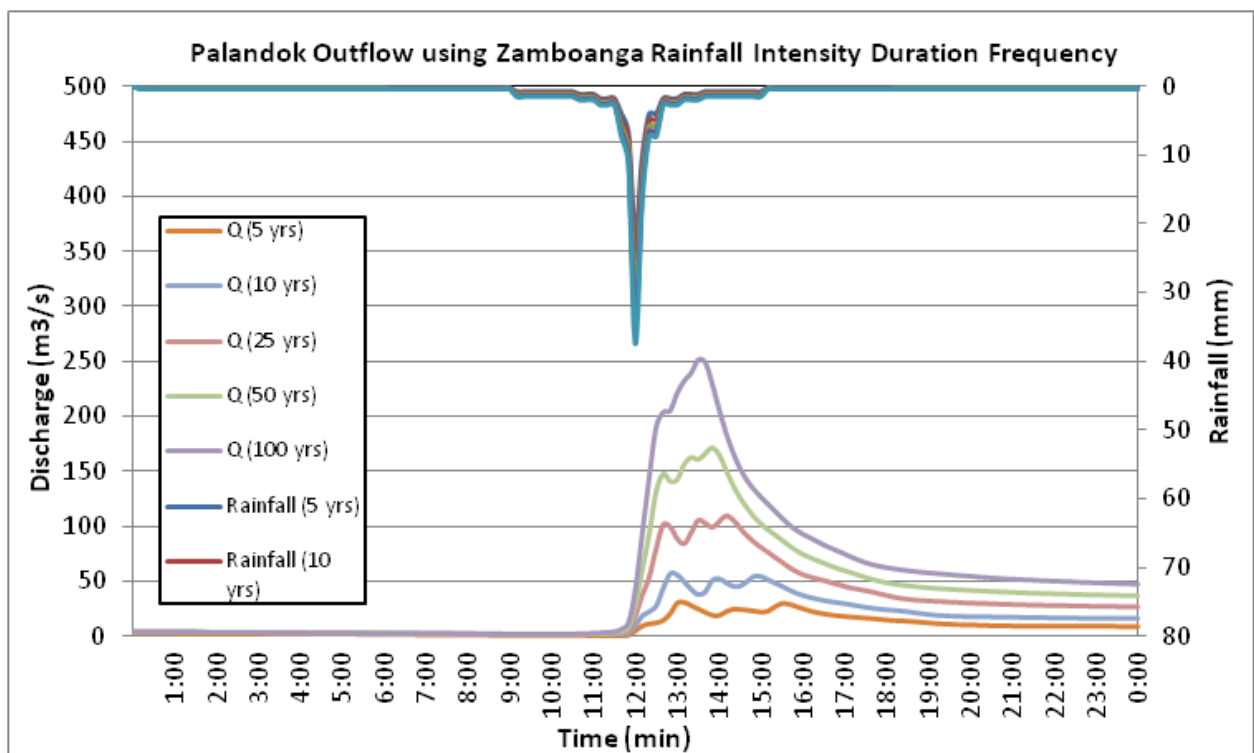


Figure 66. Outflow hydrograph at Brgy. Morob Station generated using Zamboanga City RIDF simulated in HEC-HMS

A summary of the total precipitation, peak rainfall, peak outflow and time to peak of the Palandok discharge using the Zamboanga City Rainfall Intensity-Duration-Frequency (RIDF) curves (RIDF) in five different return periods is shown in Table 35.

Table 35. Peak values of the Palandok HECHMS Model outflow using the Zamboanga City RIDF

RIDF Period	Total Precipitation (mm)	Peak Rainfall (mm)	Peak Outflow (m3/s)	Time to Peak
5-year RIDF	107.80	21.40	30.50	13 hours and 10 minutes
10-year RIDF	127.90	25.30	56.40	12 hours and 50 minutes
25-year RIDF	153.40	30.20	110.20	14 hours and 10 minutes
50-year RIDF	172.30	33.90	171.50	13 hours and 50 minutes
100-year RIDF	191.10	37.50	251.70	13 hours and 30 minutes

5.8 River Analysis (RAS) Model Simulation

The HEC-RAS Flood Model produced a simulated water level at every cross-section for every time step for every flood simulation created. The resulting model will be used in determining the flooded areas within the model. The simulated model will be an integral part in determining real-time flood inundation extent of the river after it has been automated and uploaded on the DREAM website. The sample generated map of Palandok River using the calibrated HMS base flow is shown in Figure 67.



Figure 67. Sample output of Palandok RAS Model

5.9 Flood Hazard and Flow Depth

The resulting hazard and flow depth maps have a 10m resolution. Figure 14 to Figure 19 shows the 1005-, 25-, and 5100-year rain return scenarios of the Palandok floodplain.

The generated flood hazard maps for the Palandok Floodplain were used to assess the vulnerability of the educational and medical institutions in the floodplain. Using the flood depth units of PAG-ASA for hazard maps - “Low”, “Medium”, and “High” - the affected institutions were given their individual assessment for each Flood Hazard Scenario (5 yr, 25 yr, and 100 yr).

Table 36. Municipalities affected in Palandok floodplain

Municipality	Total Area	Area Flooded	% Flooded
Bacungan	250.41	4.11	1.64%
Salug	144.44	2.62	1.81%

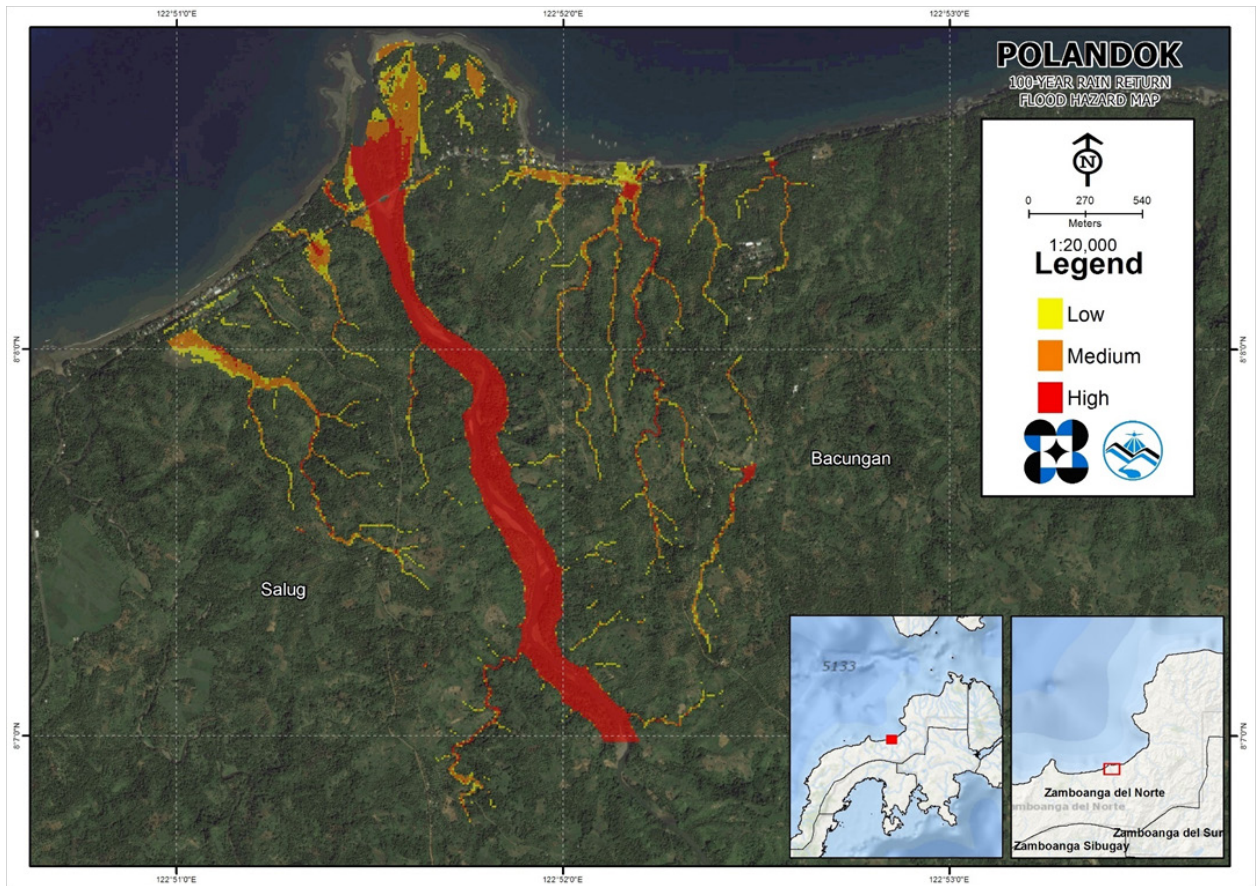


Figure 68. 100-year Flood Hazard Map for Palandok Floodplain

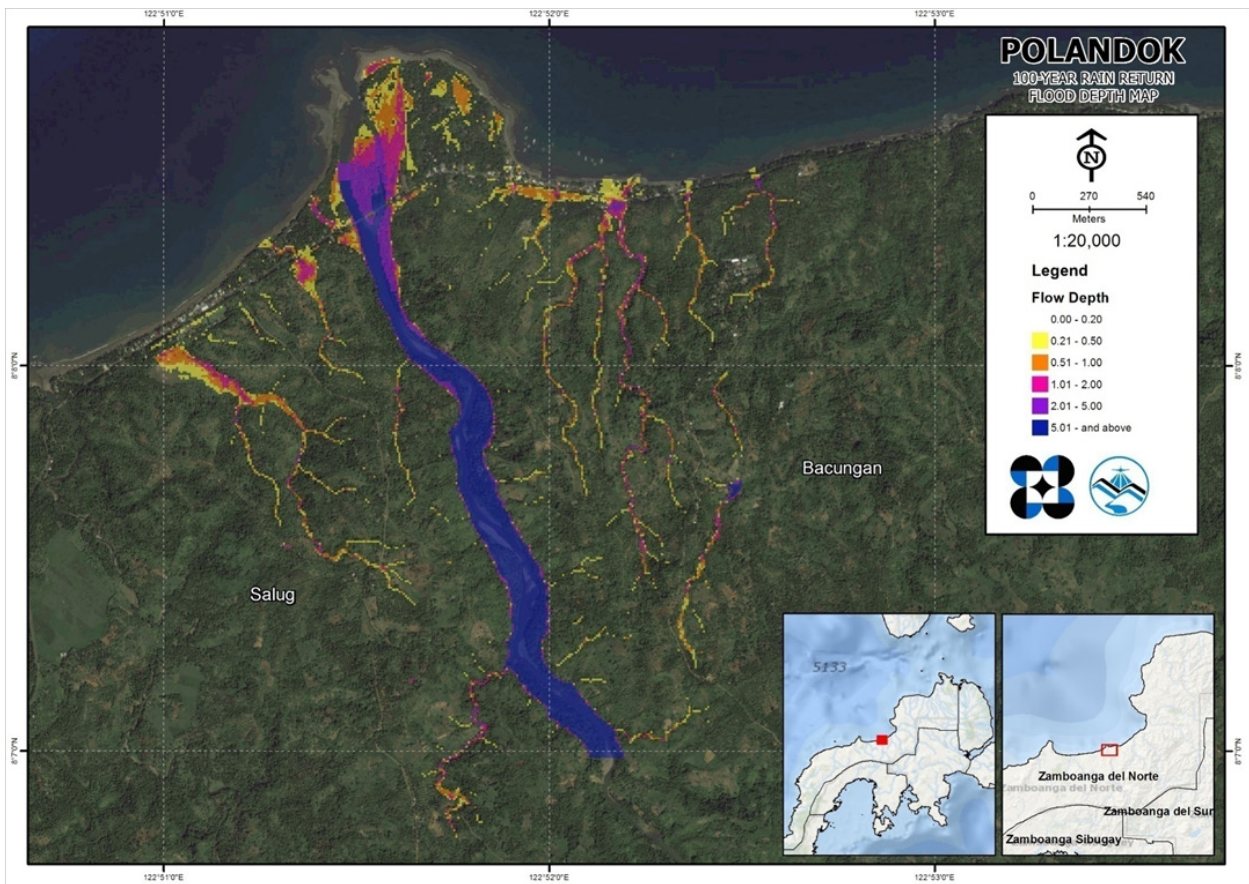


Figure 69. 100-year Flow Depth Map for Palandok Floodplain

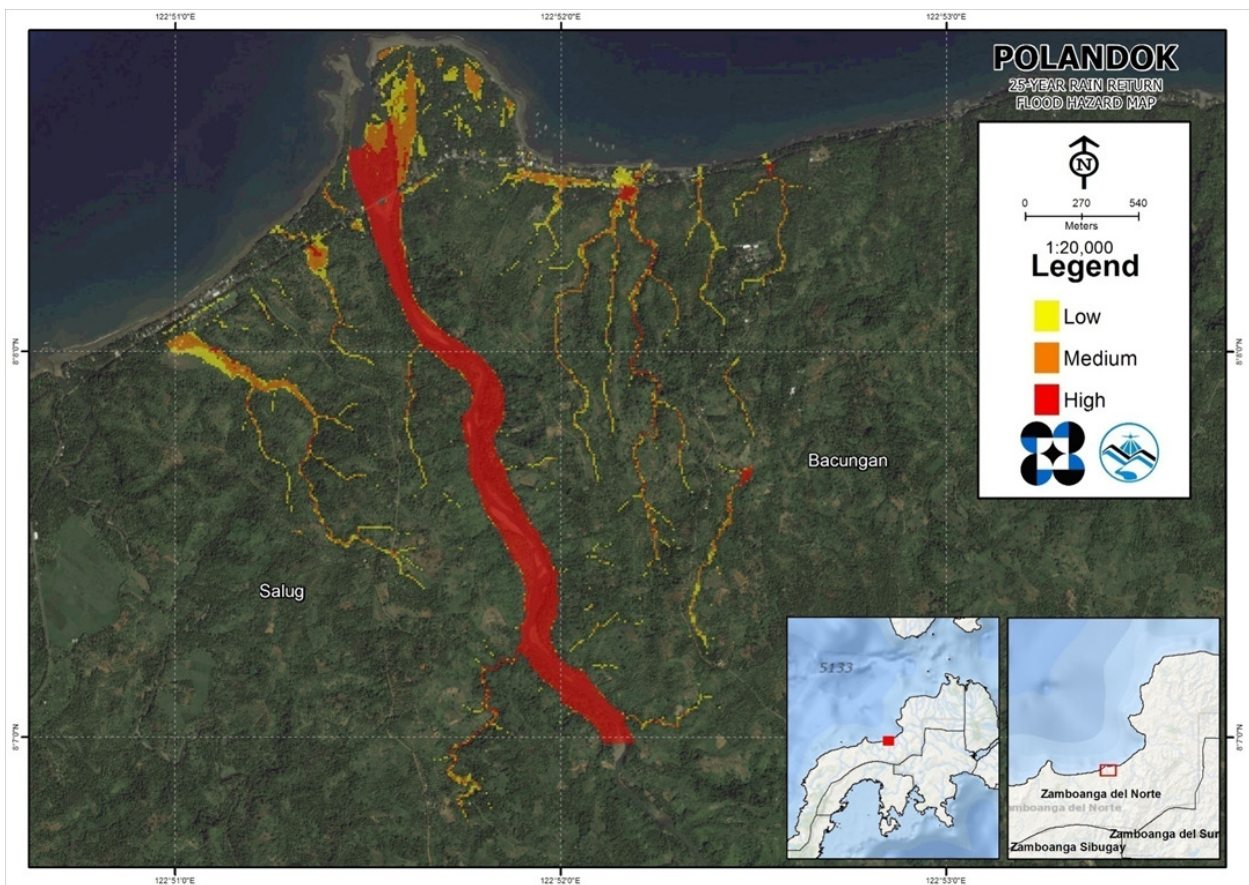


Figure 70. 25-year Flood Hazard Map for Palandok Floodplain

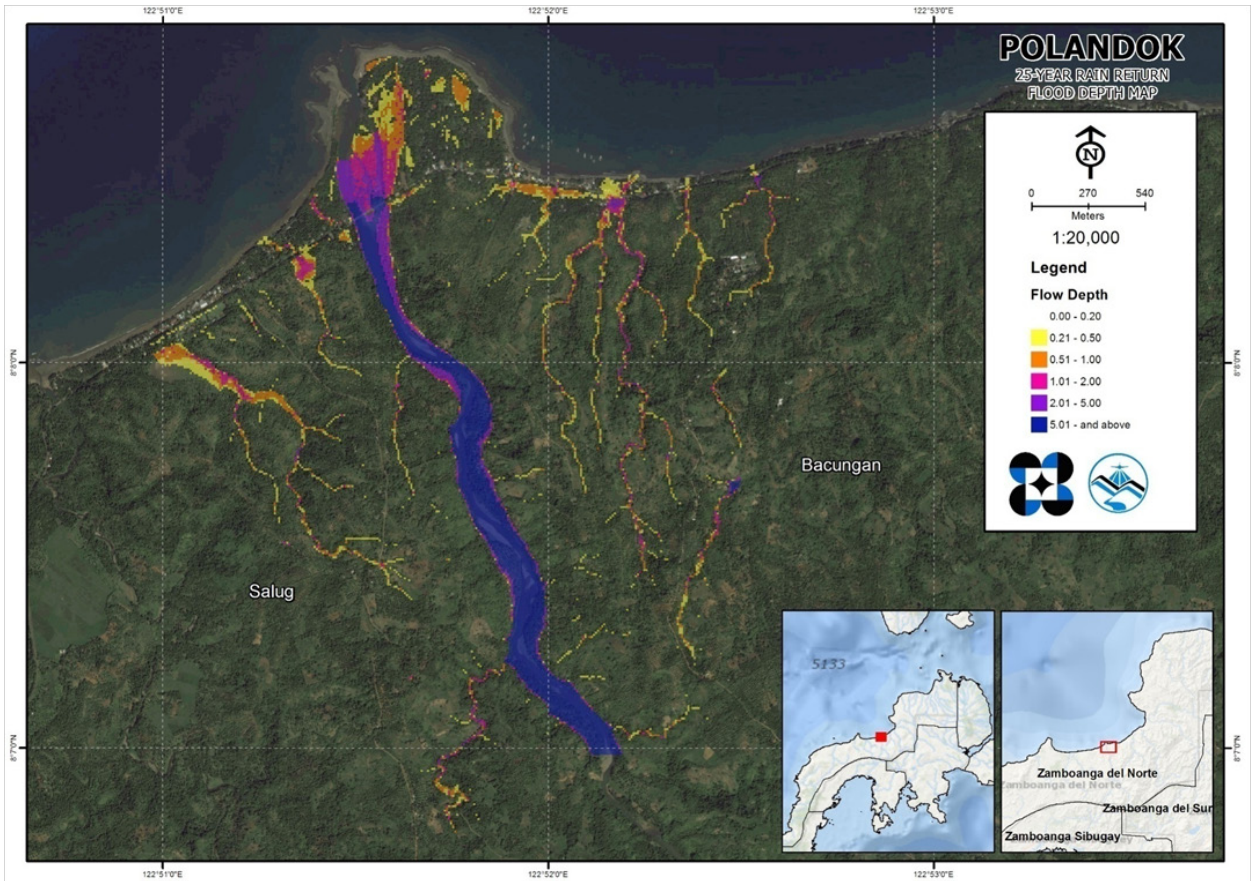


Figure 71. 25-year Flow Depth Map for Palandok Floodplain

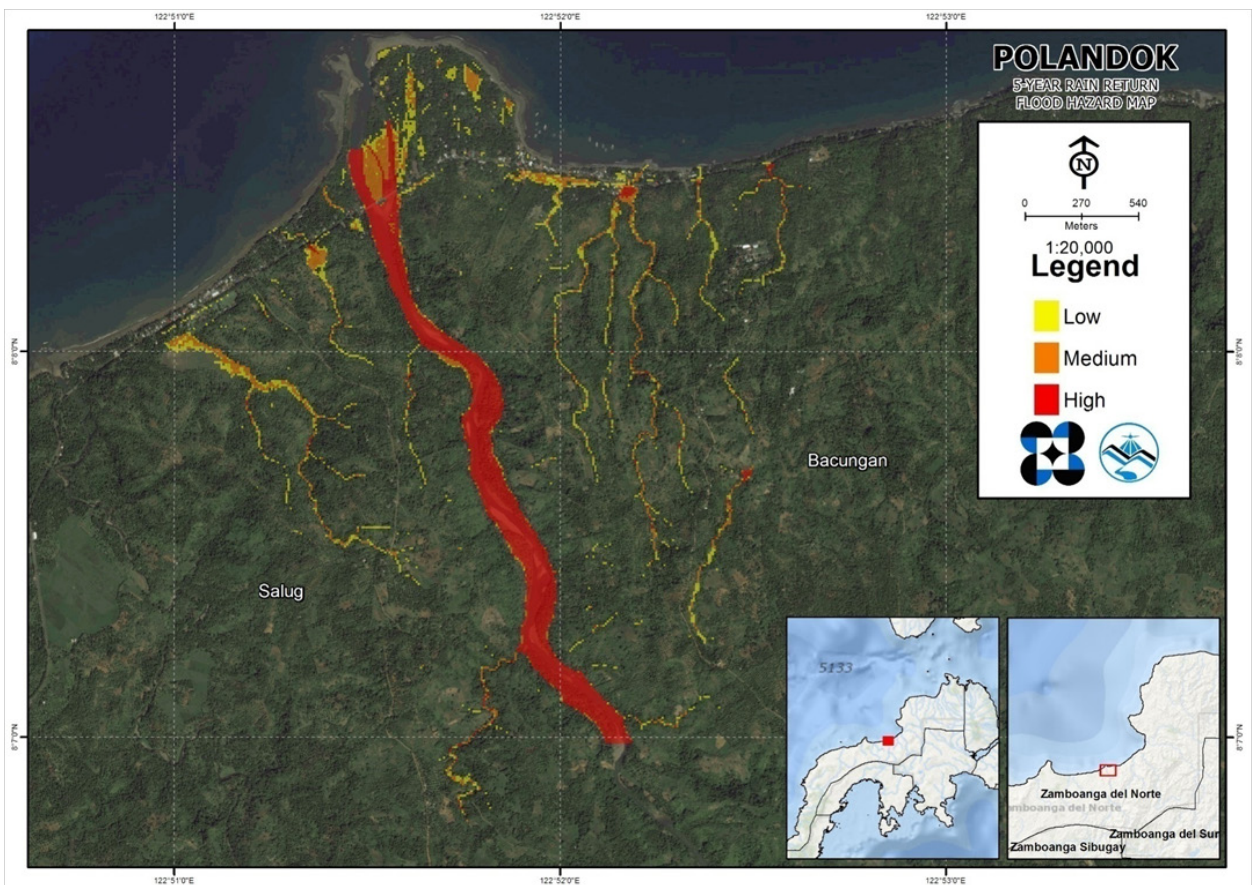


Figure 72. 5-year Flood Hazard Map for Palandok Floodplain

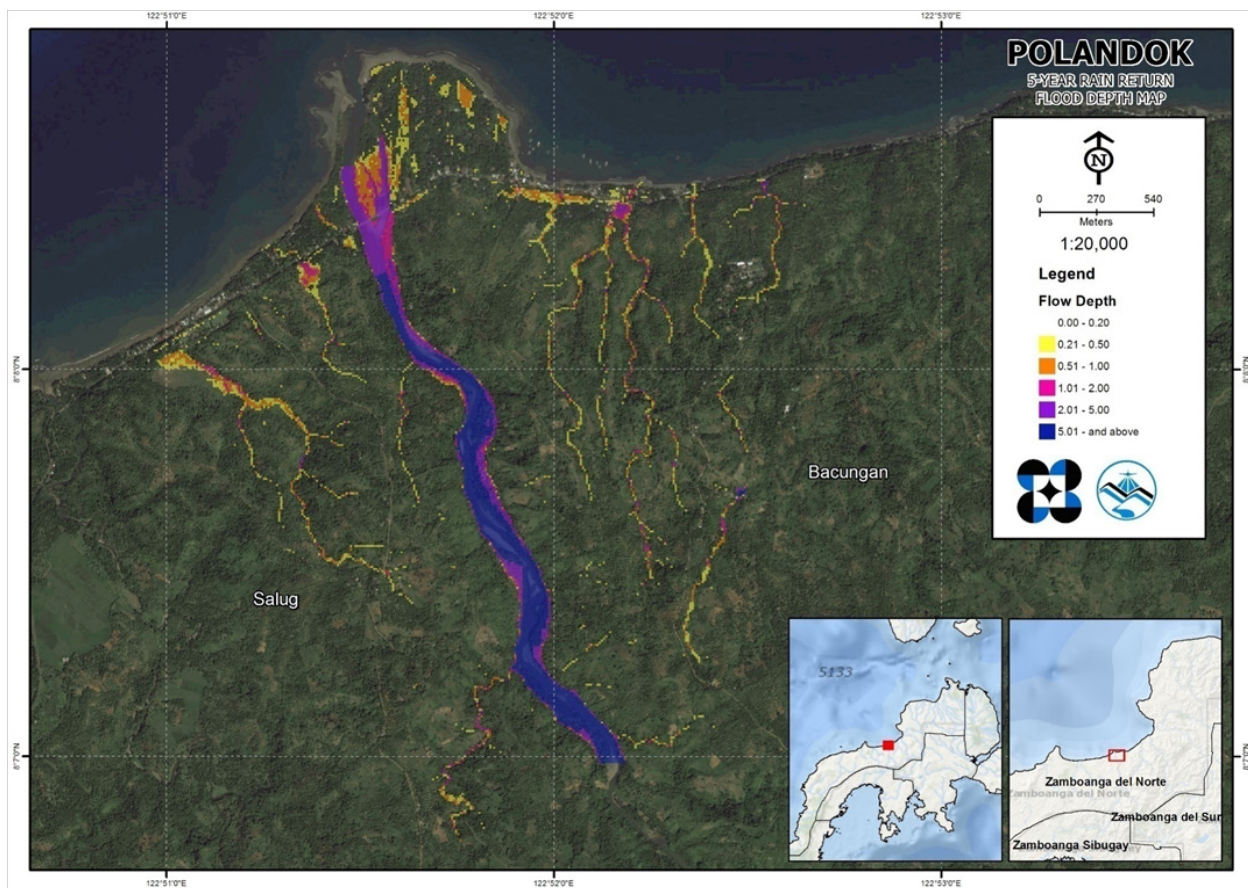


Figure 73. 5-year Flood Depth Map for Palandok Floodplain

5.10 Inventory of Areas Exposed to Flooding

Listed below are the barangays affected by the Palandok River Basin, grouped accordingly by municipality. For the said basin, two (2) municipalities consisting of 3 barangays are expected to experience flooding when subjected to a 5-year rainfall return period.

For the 5-year return period, 1.43% of the municipality of Bacungan with an area of 250.41 sq. km. will experience flood levels of less than 0.20 meters. 0.05% of the area will experience flood levels of 0.21 to 0.50 meters while 0.03%, 0.02%, 0.04%, and 0.08% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in the table 37 are the affected areas in square kilometers by flood depth per barangay.

Table 37. Affected Areas in Bacungan, Zamboanga del Norte during 5-Year Rainfall Return Period

Affected Area (sq. km.)	Affected area (sq. km.) by flood depth (in m.)	Area of affected barangays in Roseller Lim (in sq. km.)	
		Bogabongan	Rizon
Affected Area (sq. km.)	1	0.38	3.19
	2	0.011	0.11
	3	0.0049	0.082
	4	0.0011	0.043
	5	0.0012	0.092
	6	0.00025	0.19

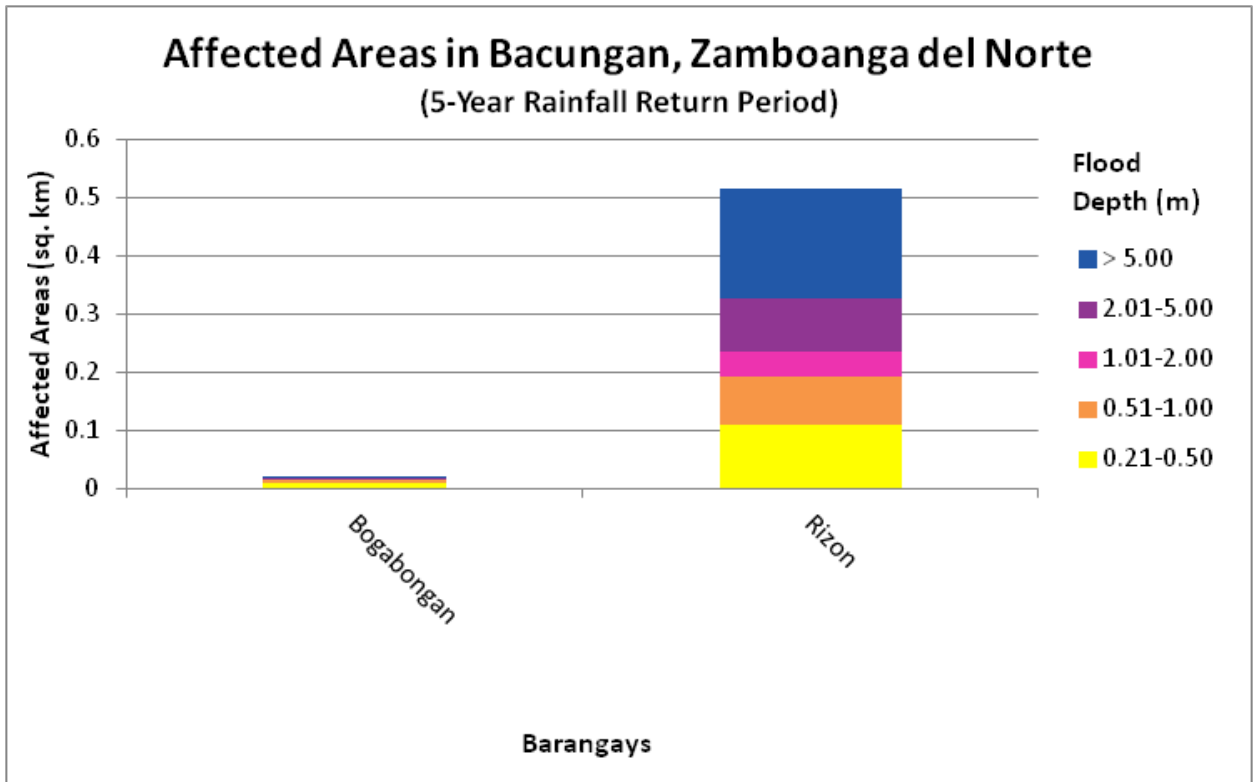


Figure 74. Affected Areas in Bacungan, Zamboanga del Norte during 5-Year Rainfall Return Period

For the 5-year return period, 1.65% of the municipality of Salug with an area of 144.44 sq. km. will experience flood levels of less than 0.20 meters. 0.04% of the area will experience flood levels of 0.21 to 0.50 meters while 0.03%, 0.02%, 0.02%, and 0.05% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in the table 38 are the affected areas in square kilometers by flood depth per barangay.

Table 38. Affected Areas in Salug, Zamboanga del Norte during 5-Year Rainfall Return Period

Affected Area (sq. km.)	Affected area (sq. km.) by flood depth (in m.)	
	Area of affected barangays in Roseller Lim (in sq. km.)	
	Ramon Magsaysay	
1	2.39	
2	0.064	
3	0.043	
4	0.022	
5	0.03	
6	0.067	

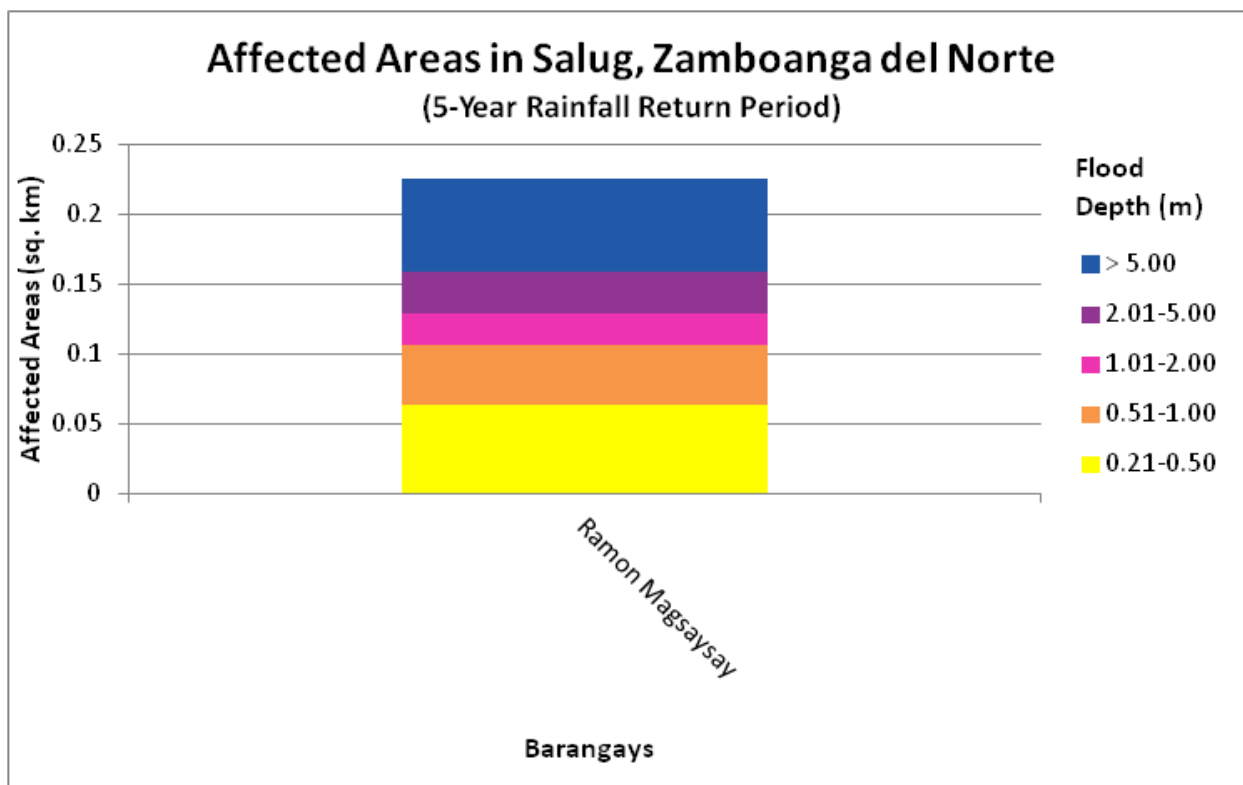


Figure 75. Affected Areas in Salug, Zamboanga del Norte during 5-Year Rainfall Return Period

For the 25-year return period, 1.39% of the municipality of Bacungan with an area of 250.41 sq. km. will experience flood levels of less than 0.20 meters. 0.05% of the area will experience flood levels of 0.21 to 0.50 meters while 0.04%, 0.03%, 0.03%, and 0.10% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in the table 39 are the affected areas in square kilometers by flood depth per barangay.

Table 39. Affected Areas in Bacungan, Zamboanga del Norte during 25-Year Rainfall Return Period

Affected Area (sq. km.)	Affected area (sq. km.) by flood depth (in m.)	Area of affected barangays in Roseller Lim (in sq. km.)	
		Bogabongan	Rizon
1	1	0.38	3.09
2	2	0.011	0.12
3	3	0.0076	0.1
4	4	0.0018	0.065
5	5	0.0014	0.073
6	6	0.00036	0.26

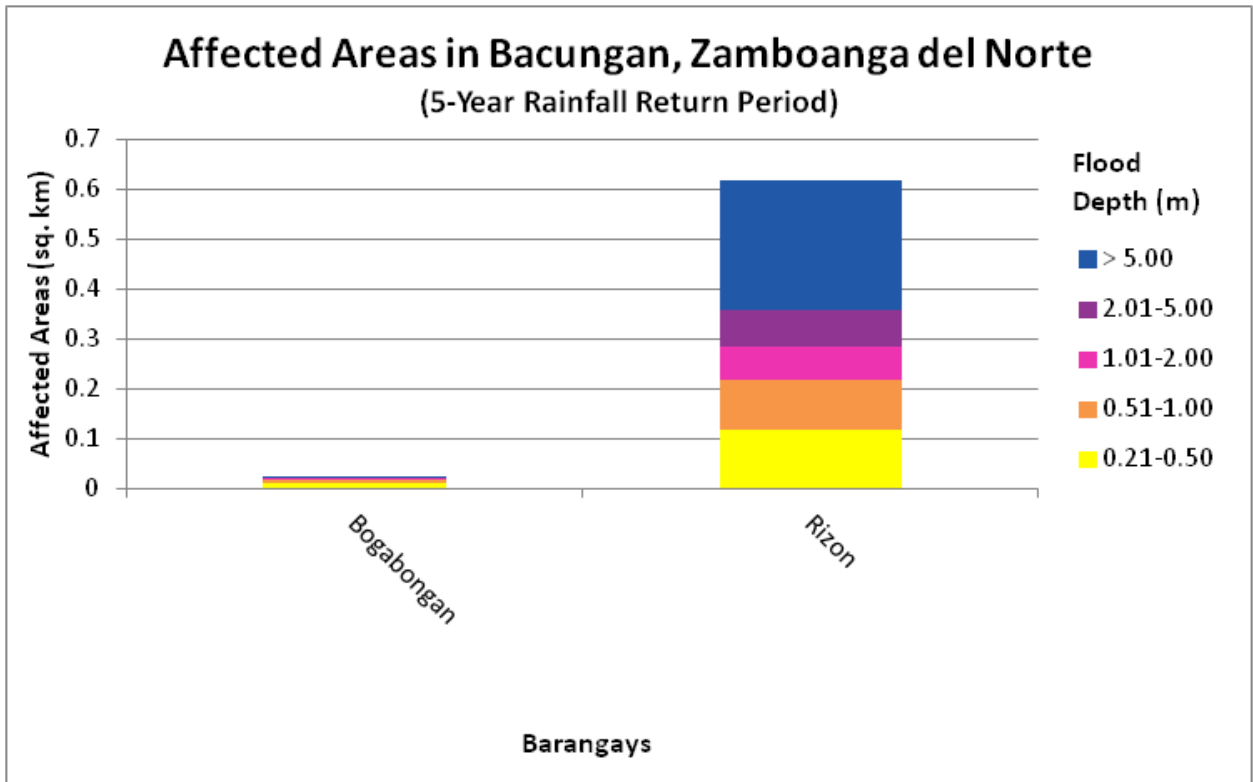


Figure 76. Affected Areas in Bacungan, Zamboanga del Norte during 25-Year Rainfall Return Period

For the 25-year return period, 1.61% of the municipality of Salug with an area of 144.44 sq. km. will experience flood levels of less than 0.20 meters. 0.05% of the area will experience flood levels of 0.21 to 0.50 meters while 0.04%, 0.02%, 0.02%, and 0.07% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in the table 40 are the affected areas in square kilometers by flood depth per barangay.

Table 40. Affected Areas in Salug, Zamboanga del Norte during 25-Year Rainfall Return Period

Affected area (sq. km.) by flood depth (in m.)		Area of affected barangays in Roseller Lim (in sq. km.)
		Ramon Magsaysay
Affected Area (sq. km.)	1	2.33
	2	0.075
	3	0.054
	4	0.031
	5	0.029
	6	0.095

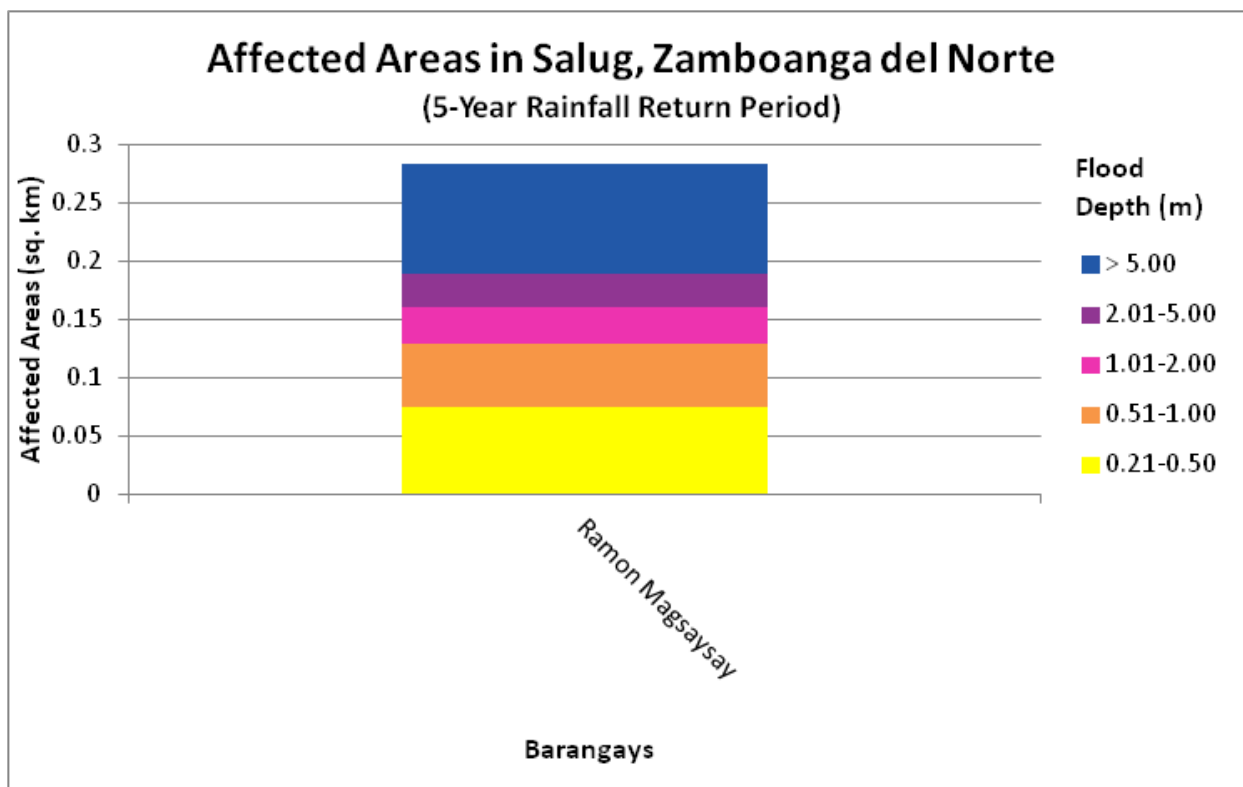


Figure 77. Affected Areas in Salug, Zamboanga del Norte during 25-Year Rainfall Return Period

For the 100-year return period, 1.36% of the municipality of Bacungan with an area of 250.41 sq. km. will experience flood levels of less than 0.20 meters. 0.06% of the area will experience flood levels of 0.21 to 0.50 meters while 0.05%, 0.03%, 0.03%, and 0.12% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in the table 41 are the affected areas in square kilometers by flood depth per barangay.

Table 41. Affected Areas in Bacungan, Zamboanga del Norte during 100-Year Rainfall Return Period

Affected area (sq. km.) by flood depth (in m.)		Area of affected barangays in Roseller Lim (in sq. km.)	
		Bogabongan	Rizon
Affected Area (sq. km.)	1	0.38	3.02
	2	0.011	0.13
	3	0.0087	0.11
	4	0.003	0.079
	5	0.0013	0.079
	6	0.00052	0.29

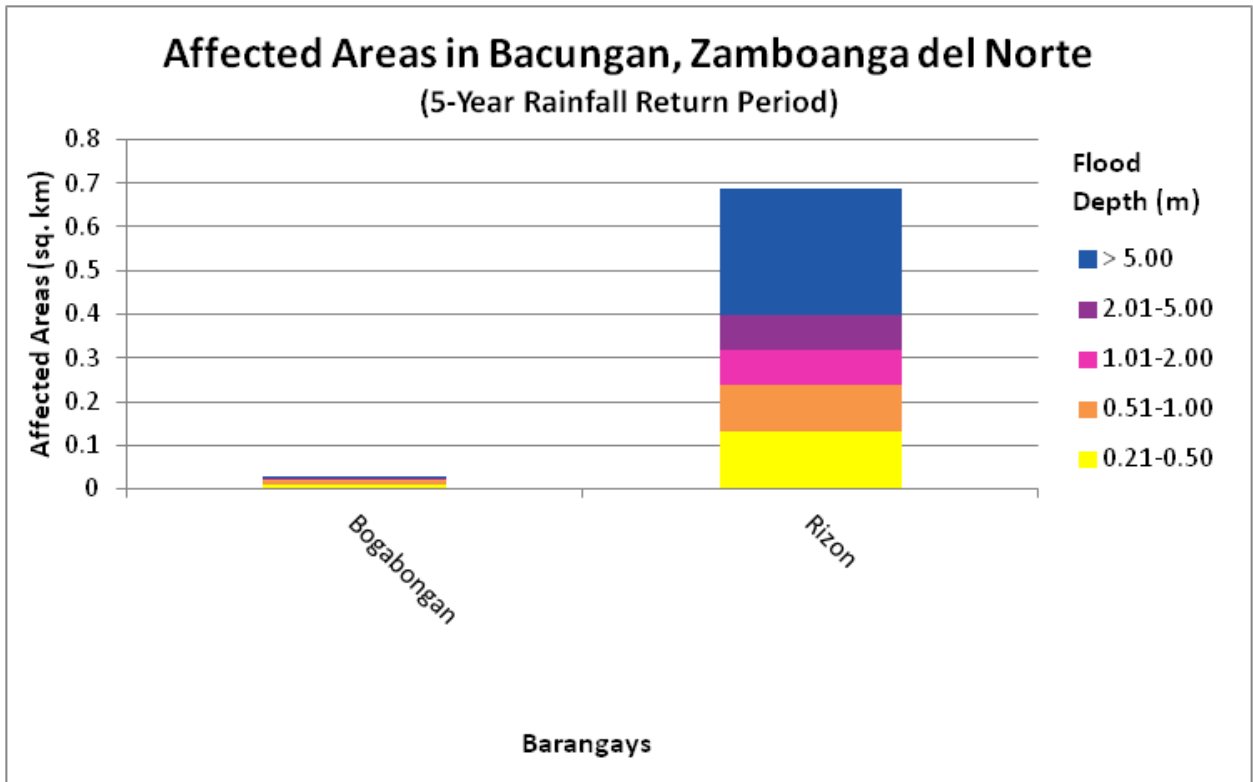


Figure 78. Affected Areas in Bacungan, Zamboanga del Norte during 100-Year Rainfall Return Period

For the 100-year return period, 1.58% of the municipality of Salug with an area of 144.44 sq. km. will experience flood levels of less than 0.20 meters. 0.06% of the area will experience flood levels of 0.21 to 0.50 meters while 0.05%, 0.03%, 0.02%, and 0.08% of the area will experience flood depths of 0.51 to 1 meter, 1.01 to 2 meters, 2.01 to 5 meters, and more than 5 meters, respectively. Listed in the table 42 are the affected areas in square kilometers by flood depth per barangay.

Table 42. Affected Areas in Salug, Zamboanga del Norte during 100-Year Rainfall Return Period

Affected Area (sq. km.)	Affected area (sq. km.) by flood depth (in m.)	
	Area of affected barangays in Roseller Lim (in sq. km.)	
		Ramon Magsaysay
1	2.28	
2	0.085	
3	0.068	
4	0.037	
5	0.025	
6	0.12	

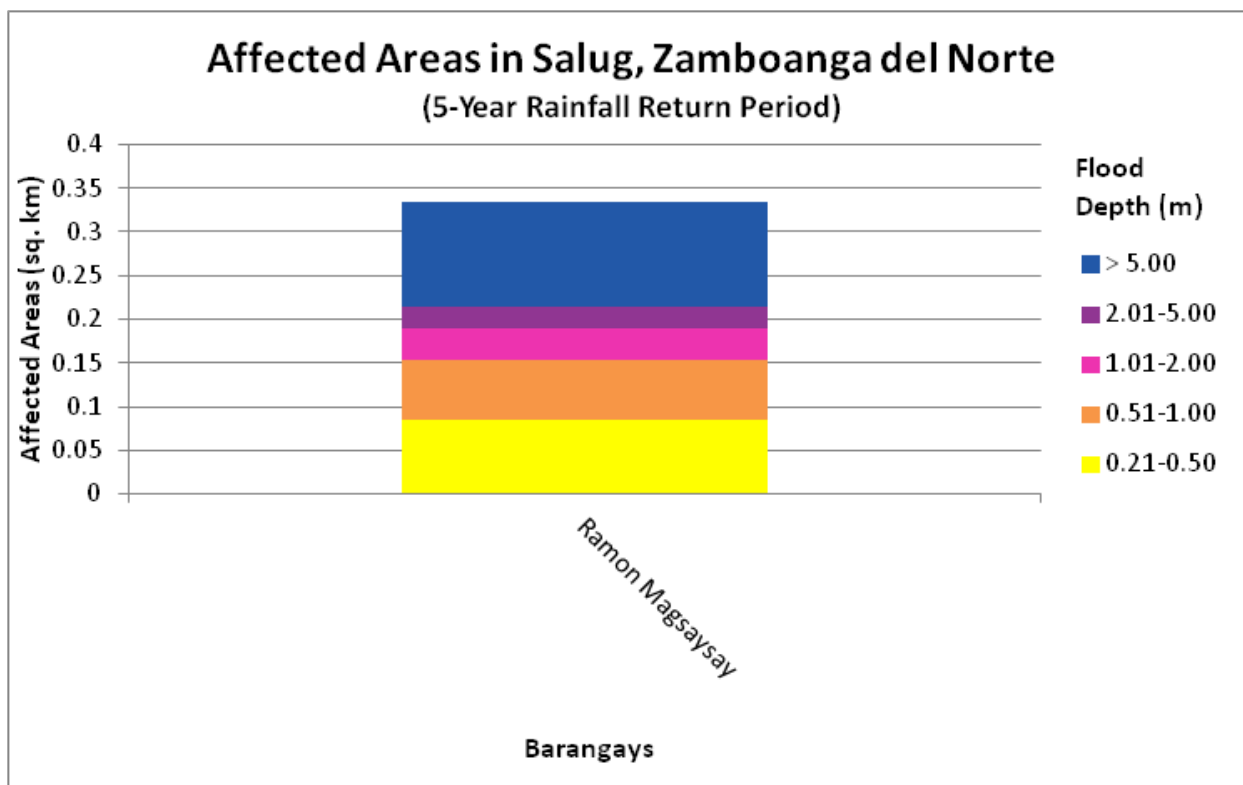


Figure 79. Affected Areas in Salug, Zamboanga del Norte during 100-Year Rainfall Return Period

Moreover, the generated flood hazard maps for the Palandok floodplain were used to assess the vulnerability of the educational and medical institutions in the floodplain. Using the flood depth units of PAG-ASA for hazard maps - “Low”, “Medium”, and “High” - the affected institutions were given their individual assessment for each Flood Hazard Scenario (5 yr, 25 yr, and 100 yr).

Table 43. Area covered by each warning level with respect to the rainfall scenario

Warning Level	Area Covered in sq. km.		
	5 year	25 year	100 year
Low	0.18	0.20	0.22
Medium	0.17	0.22	0.26
High	0.40	0.50	0.57
TOTAL	0.75	0.92	1.05

Of the 6 identified educational and medical institutions and buildings, none were assessed to be exposed to any flood hazard level for any rain return scenario.

See Appendix Annex D 12 and E Annex 13 for a detailed enumeration of schools, hospitals and clinics in the Palandok floodplain.

5.11 Flood Validation

The flood validation consists of 189 points randomly selected all over the Palandok flood plain. It has an RMSE value of 1.66.

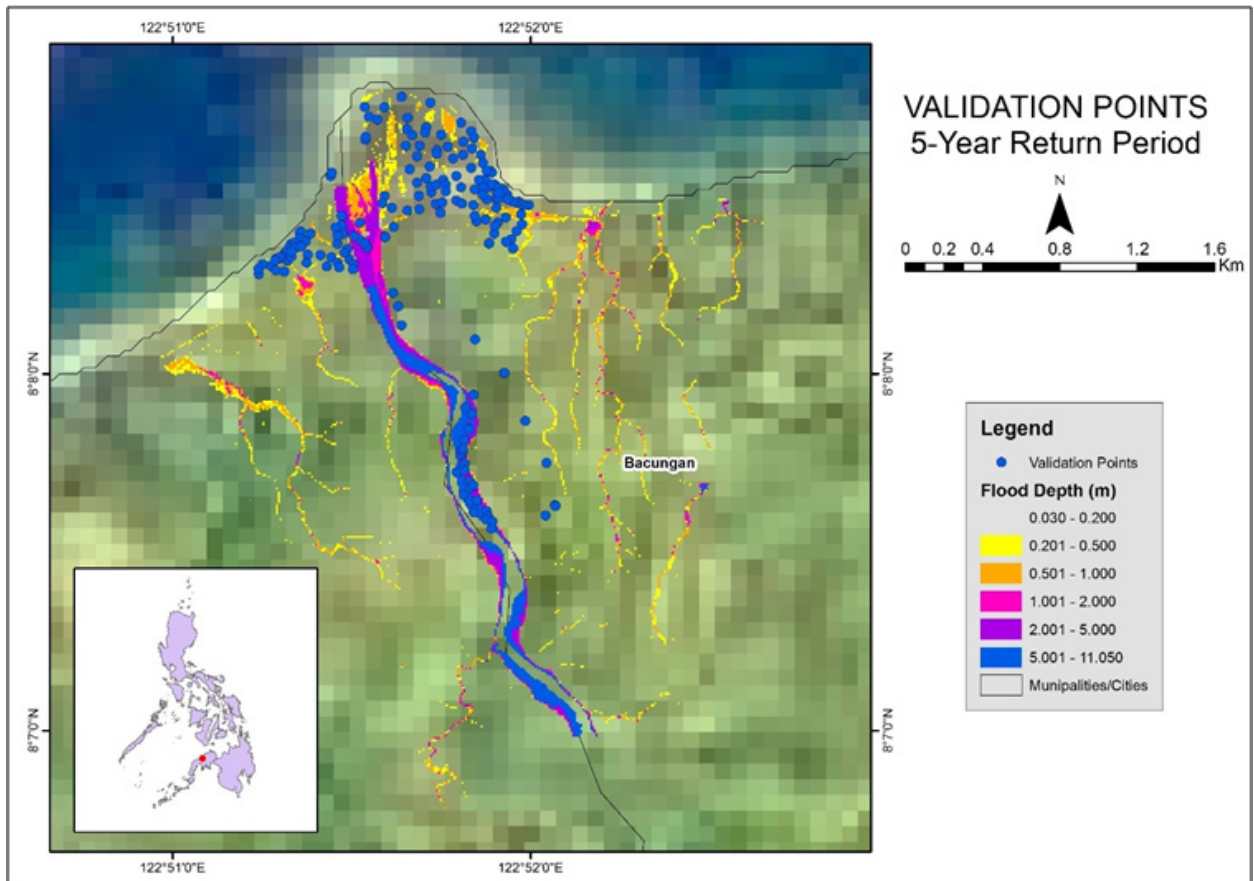


Figure 80. Validation points for 5-year Flood Depth Map of Palandok Floodplain

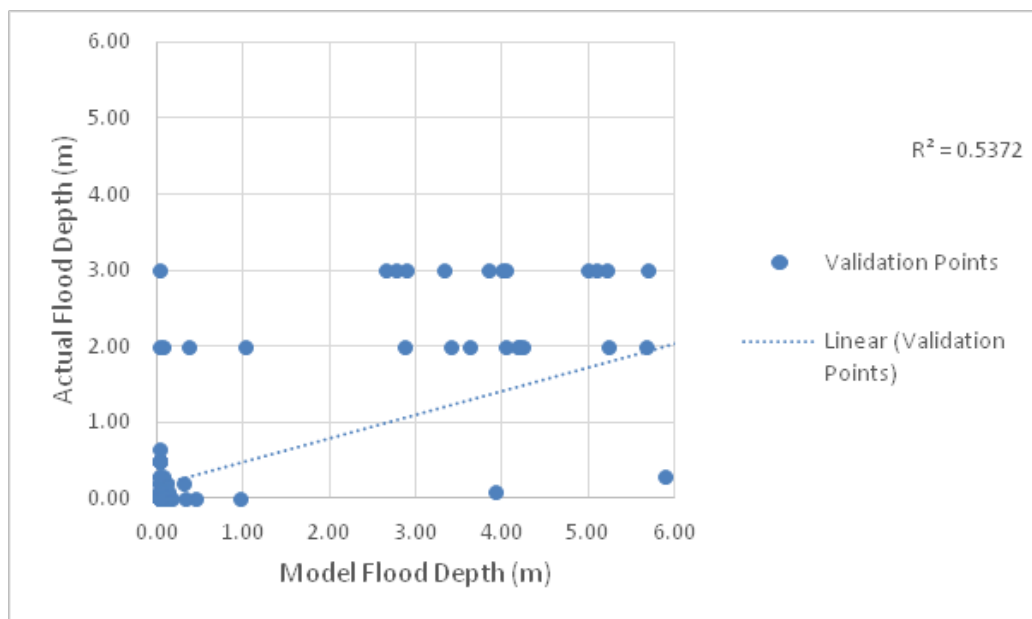


Figure 81. Flood map depth vs actual flood depth

Table 45. Actual Flood Depth vs Simulated Flood Depth in Palandok

PALANDOK BASIN		Modeled Flood Depth (m)						Total
		0-0.20	0.21-0.50	0.51-1.00	1.01-2.00	2.01-5.00	> 5.00	
Actual Flood Depth (m)	0-0.20	143	3	1	0	1	0	148
	0.21-0.50	8	0	0	0	0	1	9
	0.51-1.00	1	0	0	0	0	0	1
	1.01-2.00	2	1	0	1	7	7	18
	2.01-5.00	1	0	0	0	7	5	13
	> 5.00	0	0	0	0	0	0	0
	Total	155	4	1	1	15	13	189

The overall accuracy generated by the flood model is estimated at 79.89% with 151 points correctly matching the actual flood depths. In addition, there were 23 points estimated one level above and below the correct flood depths while there were 10 points and 5 points estimated two levels above and below, and three or more levels above and below the correct flood. A total of 4 points were overestimated while a total of 13 points were underestimated in the modelled flood depths of PalandocPalandok.

Table 46. Summary of Accuracy Assessment in Palandok

	No. of Points	%
Correct	151	79.89
Overestimated	25	13.23
Underestimated	13	6.88
Total	189	100.00

REFERENCES

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- Brunner, G. H. 2010a. HEC-RAS River Analysis System Hydraulic Reference Manual. Davis, CA: U.S. Army Corps of Engineers, Institute for Water Resources, Hydrologic Engineering Center.
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- Sarmiento C., Paringit E.C., et al. 2014. DREAM Data Acquisition Component Manual. Quezon City, Philippines: UP Training Center for Applied Geodesy and Photogrammetry.
- UP TCAGP 2016, Acceptance and Evaluation of Synthetic Aperture Radar Digital Surface Model (SAR DSM) and Ground Control Points (GCP). Quezon City, Philippines: UP Training Center for Applied Geodesy and Photogrammetry.
- <http://www.islandsproperties.com/places/zambonor.htm>).

Sources:

Municipal Profile of Leon B. Postigo

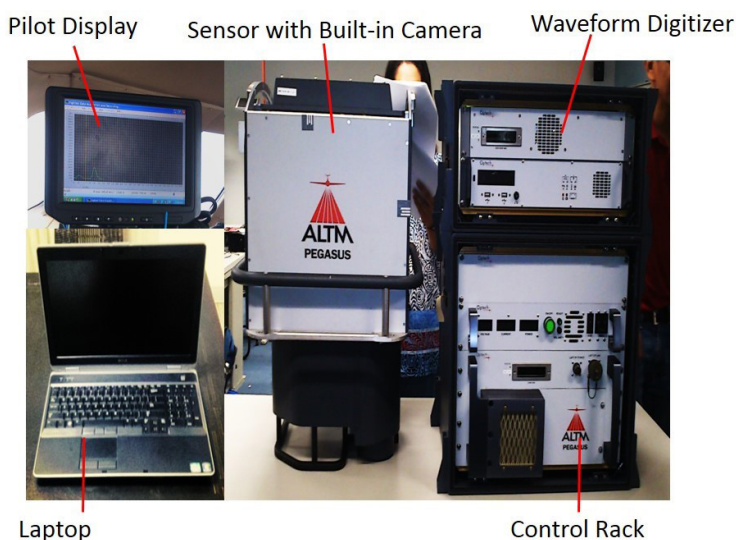
Mines and Geosciences Bureau

Philippine Institute of Vulcanology and Seismology

ANNEXES

Annex 1. Technical Specifications of the LIDAR Sensors used in the Palandok Floodplain Survey

Table A-1.1 Parameters and Specifications



Parameter	Specification
Operational envelope (1,2,3,4)	150-4000 m AGL, nominal
Laser wavelength	1064 nm
Horizontal accuracy (2)	1/5,500 x altitude, (m AGL)
Elevation accuracy (2)	<5-35 cm, 1 σ
Effective laser repetition rate	Programmable, 33-167 kHz
Position and orientation system	POS AV™ AP50 (OEM); 220-channel dual frequency GPS/GNSS/Galileo/L-Band receiver
Scan width (WOV)	Programmable, 0-50°
Scan frequency (5)	Programmable, 0-70 Hz (effective)
Sensor scan product	1000 maximum
Beam divergence	Dual divergence: 0.25 mrad (1/e) and 0.8 mrad (1/e), nominal
Roll compensation	Programmable, $\pm 5^\circ$ (FOV dependent)
Range capture	Up to 4 range measurements, including 1st, 2nd, 3rd, and last returns
Intensity capture	Up to 4 intensity returns for each pulse, including last (12 bit)
Video Camera	Internal video camera (NTSC or PAL)
Image capture	Compatible with full Optech camera line (optional)
Full waveform capture	12-bit Optech IWD-2 Intelligent Waveform Digitizer (optional)
Data storage	Removable solid state disk SSD (SATA II)
Power requirements	28 V; 900 W; 35 A(peak)

Dimensions and weight	Sensor: 260 mm (w) x 190 mm (l) x 570 mm (h); 23 kg Control rack: 650 mm (w) x 590 mm (l) x 530 mm (h); 53 kg
Operating temperature	-10°C to +35°C (with insulating jacket)
Relative humidity	0-95% no-condensing

Annex 2. NAMRIA Certification of Reference Points Used in the LIDAR Survey

ZGN-132

Figure A-2.1 ZGN-132



Republic of the Philippines
Department of Environment and Natural Resources
NATIONAL MAPPING AND RESOURCE INFORMATION AUTHORITY

October 30, 2014

CERTIFICATION

To whom it may concern:

This is to certify that according to the records on file in this office, the requested survey information is as follows -

Province: ZAMBOANGA DEL NORTE		
Station Name: ZGN-132		
Order: 2nd		
Island: MINDANAO	Barangay: MANDIH	
Municipality: SINDANGAN	MSL Elevation:	
PRS92 Coordinates		
Latitude: 8° 12' 44.29460"	Longitude: 123° 0' 19.12667"	Ellipsoidal Hgt: 11.50200 m.
WGS84 Coordinates		
Latitude: 8° 12' 40.63408"	Longitude: 123° 0' 24.56923"	Ellipsoidal Hgt: 75.58000 m.
PTM / PRS92 Coordinates		
Northing: 908029.029 m.	Easting: 500585.389 m.	Zone: 4
UTM / PRS92 Coordinates		
Northing: 907,711.20	Easting: 500,585.18	Zone: 51

Location Description

The station is marked by an 4" copper nail with its head flushed at the center of an cement putty on a concrete open canal with inscription " ZGN-132, 2009 NAMRIA". Located at brgy. Mandih sindangan zamboanga del norte. The monument is situated inside mandih central school 30 meters from the gate going east.

Requesting Party: **PHIL-LIDAR I**
Purpose: **Reference**
OR Number: **8075910 I**
T.N.: **2014-2585**

RUEL DM. BELEN, MNSA
Director, Mapping And Geodesy Branch



NAMRIA OFFICES:
Main : Lawton Avenue, Fort Bonifacio, 1634 Taguig City, Philippines Tel. No.: (632) 810-4831 to 41
Branch : 421 Barraca St. San Nicolas, 1010 Manila, Philippines, Tel. No. (632) 241-3494 to 98
www.namria.gov.ph

ISO 9001: 2008 CERTIFIED FOR MAPPING AND GEOSPATIAL INFORMATION MANAGEMENT

Annex 3. Baseline Processing Reports of Control Points used in the LIDAR Survey

ZN-123

Vector Components (Mark to Mark)

From: ZGN 132					
Grid		Local		Global	
Easting	500585.184 m	Latitude	N8°12'44.29460"	Latitude	N8°12'40.63408"
Northing	907711.203 m	Longitude	E123°00'19.12667"	Longitude	E123°00'24.56923"
Elevation	10.036 m	Height	11.502 m	Height	75.580 m

To: ZN 123 AM					
Grid		Local		Global	
Easting	500592.329 m	Latitude	N8°13'08.18558"	Latitude	N8°13'04.52332"
Northing	908444.828 m	Longitude	E123°00'19.36053"	Longitude	E123°00'24.80249"
Elevation	8.704 m	Height	10.101 m	Height	74.166 m

Vector					
ΔEasting	7.145 m	NS Fwd Azimuth	0°33'32"	ΔX	51.898 m
ΔNorthing	733.625 m	Ellipsoid Dist.	733.953 m	ΔY	-93.002 m
ΔElevation	-1.333 m	ΔHeight	-1.401 m	ΔZ	726.187 m

Standard Errors

Vector errors:					
σ ΔEasting	0.001 m	σ NS fwd Azimuth	0°00'00"	σ ΔX	0.001 m
σ ΔNorthing	0.000 m	σ Ellipsoid Dist.	0.000 m	σ ΔY	0.001 m
σ ΔElevation	0.001 m	σ ΔHeight	0.001 m	σ ΔZ	0.000 m

Aposteriori Covariance Matrix (Meter²)

	X	Y	Z
X	0.0000005039		
Y	-0.0000002536	0.0000007702	
Z	-0.0000000601	0.0000000520	0.0000001616

Figure A-3.1 ZN-123

Annex 4. The LIDAR Survey Team Composition

Data Acquisition Component Sub-Team	Designation	Name	Agency/ Affiliation
PHIL-LIDAR 1	Program Leader	ENRICO C. PARINGIT, D.ENG	UP-TCAGP
Data Acquisition Component Leader	Data Component Project Leader – I	ENGR. CZAR JAKIRI SARMIENTO	UP-TCAGP
Survey Supervisor	Chief Science Research Specialist (CSRS)	ENGR. CHRISTOPHER CRUZ	UP-TCAGP
LiDAR Operation	Supervising Science Research Specialist (Supervising SRS)	LOVELY GRACIA ACUÑA	UP-TCAGP
		LOVELYN ASUNCION	UP-TCAGP
FIELD TEAM			
LiDAR Operation	Senior Science Research Specialist (SSRS)	JASMINE ALVIAR	UP-TCAGP
	Research Associate (RA)	ENGR. IRO NIEL ROXAS	UP-TCAGP
		ENGR. GRACE SINADJAN	UP-TCAGP
		KRISTINE JOY ANDAYA	UP-TCAGP
Ground Survey, Data Download and Transfer	RA	ENGR. RENAN PUNTO	UP-TCAGP
LiDAR Operation	Airborne Security	SSG. RONALD MONTENEGRO	PHILIPPINE AIR FORCE (PAF)
	Pilot	CAPT. JOHN BRYAN DONGUINES	ASIAN AEROSPACE CORPORATION (AAC)
		CAPT. FERDINAND DE OCAMPO	AAC

Table A-4.1. The LiDAR Survey Team Composition

Annex 5. Data Transfer Sheet Palandok Floodplain

DATA TRANSFER SHEET
15/11/2014 (gponlog)

DATE	FLIGHT NO.	MISSION NAME	SENSOR	RAIN LAS		LOS(M)	POS	RAW (MAGESAS)	MISSION LOG FILE(SAS) LOGS	RANGE	DIST/FEET	BASE STATIONS		OPERATOR LOGS (OP LOG)	FLIGHT PLAN		SERVER LOCATION
				Output LAS	KML (weekly)							BASE STATIONS	Base Info (url)		Actual	KML	
19-Oct	2099P	1BLK69CAL0292A	PEGASUS	106	na	3.84	113	9.11	83	3.84	na	4.08	19B	19B	47/23	na	Z:\D\C\RAW DATA
22-Oct	2111P	1BLK69B295A	PEGASUS	2.01	453	12.4	236	29.5	292	22.2	na	7.59	19B	19B	29/04/58	na	Z:\D\C\RAW DATA
23-Oct	2113P	1BLK69B296A	PEGASUS	1.62	364	10	216	36.4	200	19.4	na	15.1	19B	19B	48/5/4481	na	Z:\D\C\RAW DATA
23-Oct	2115P	1BLK6970A296B	PEGASUS	7.64	171	6.26	105	13.6	115	8.18	na	15.1	19B	19B	49/5/25	na	Z:\D\C\RAW DATA
24-Oct	2117P	1BLK69B297A	PEGASUS	1.24	237	5.73	113	18.6	164	12.4	na	7.64	19B	19B	48/5/150	na	Z:\D\C\RAW DATA
26-Oct	2125P	1BLK69C299A	PEGASUS	1.5	287	8.41	211	32	239	15.4	na	37.4	19B	19B	48/5/1041/564322	na	Z:\D\C\RAW DATA
26-Oct	2127P	1BLK6970A299B	PEGASUS	2.64	na	5.91	114	16	1	12.9	na	37.4	19B	19B	27/1	na	Z:\D\C\RAW DATA
28-Oct	2133P	1BLK69FE301A	PEGASUS	2.06	244/854	10.2	225	57.4	343	na	na	37.3	19B	19B	73/5/18129	na	Z:\D\C\RAW DATA
28-Oct	2135P	1BLK69F301B	PEGASUS	2.68	170/87	3.42	94.6	na	na	5.4	na	37.3	19B	19B	92	na	Z:\D\C\RAW DATA
29-Oct	2137P	1BLK700302A	PEGASUS	1.09	45/920	7.37	190	28.3	181	11.2	na	19.4	19B	19B	106/93/8075	na	Z:\D\C\RAW DATA
31-Oct	2145P	1BLK69C304A	PEGASUS	2.33	1118/04	8.11	182	48.6	342	22.8	na	6.52	19B	19B	74	na	Z:\D\C\RAW DATA
1-Nov	2149P	1BLK70B305A	PEGASUS	1.79	338	7.39	182	32.1	227	20.3	na	26.2	19B	19B	54/79	na	Z:\D\C\RAW DATA
3-Nov	2157P	1BLK70C307A	PEGASUS	2.26	198	11.3	240	40.6	295	25.1	na	35.5	19B	19B	33	na	Z:\D\C\RAW DATA
6-Nov	2169P	1BLK73A310A	PEGASUS	3.01	623	12.1	240	62.9	na	26.9	na	21.3	19B	19B	80	na	Z:\D\C\RAW DATA
8-Nov	2177P	1BLK70C312A	PEGASUS	1.75	327	8.89	196	23.1	na	17.5	na	17.8	19B	19B	60/5/97	na	Z:\D\C\RAW DATA
9-Nov	2181P	1BLK69F313A	PEGASUS	1.46	291	8.11	182	22.4	182	15.2	na	17	19B	19B	66/6/281	na	Z:\D\C\RAW DATA

Received from
 Name: C. Jomawin
 Position: PA
 Signature: [Signature]

Received by
 Name: Angelo Carlo Bongat
 Position: SSRS
 Signature: [Signature] 11/11/2014

Figure A-5.1. Transfer Sheet for Palandok Floodplain

Annex 6. Flight Logs for the Flight Missions

Flight Log for 1BLK70C307A Mission

Flight Log No.: 2157P

PHIL-LIDAR 1 Data Acquisition Flight Log

1 LIDAR Operator: I. ROSAS	2 ALTM Model: PEGASUS	3 Mission Name: 1BLK 70C307A	4 VFR Type: VFR	5 Aircraft Type: Casmat206H	6 Aircraft Identification: 7022
7 Pilot: B. DOLORES	8 Co-Pilot: F. DE GUANO	9 Route: PIPOLC	10 Date: NOV. 3, 2014	11 Airport of Arrival (Airport, City/Province):	12 Airport of Departure (Airport, City/Province):
13 Engine On: 9:21	14 Engine Off: 13:20	15 Total Engine Time: cloudy	16 Take off:	17 Landing:	18 Total Flight Time:
19 Weather	20 Remarks: SUCCESSFUL FLIGHT.				

21 Problems and Solutions:

Acquisition Flight Approved by

J. Alon

Signature over Printed Name
(End User Representative)

Acquisition Flight Certified by

[Signature]

Signature over Printed Name
(PAF Representative)

Pilot-in-Command

[Signature]

Signature over Printed Name

Lidar Operator

[Signature]

Signature over Printed Name

Figure A-6.1 Flight Log for 1BLK70C307A Mission

Annex 7. Flight Status Reports

Table A-7.1. Flight Status Report

DIPOLOG-ZAMBOANGA DEL NORTE (October 8 to November 11, 2014)					
FLIGHT NO	AREA	MISSION	OPERATOR	DATE FLOWN	REMARKS
2157P	BLK 75A	1BLK70C307A	I. Roxas	November 3, 2014	Surveyed BLK 73B; Clouds caused gaps

LAS BOUNDARIES PER FLIGHT

Flight No.: 2157P

Area: BLK 73B

Mission Name: 1BLK70C307A

Parameters: Altitude: 1000 m; Scan Frequency: 30 Hz;
 Scan Angle: 25 deg; Overlap: 20%

LAS

ANNEX 8. MISSION SUMMARY REPORT

Table A-8.1. Mission Summary Report for Mission Blk73B

Flight Area	Dipolog
Mission Name	Blk73B
Inclusive Flights	2157P
Mission Name	1BLK70C307A
Range data size	25.1 GB
POS	240MB
Base data size	35.5
Image	40.6 GB
Transfer date	November 19, 2014
<i>Solution Status</i>	
Number of Satellites (>6)	Yes
PDOP (<3)	Yes
Baseline Length (<30km)	Yes
Processing Mode (<=1)	Yes
<i>Smoothed Performance Metrics(in cm)</i>	
RMSE for North Position (<4.0 cm)	1.05
RMSE for East Position (<4.0 cm)	1.85
RMSE for Down Position (<8.0 cm)	2.85
<i>Boresight correction stdev (<0.001deg)</i>	
IMU attitude correction stdev (<0.001deg)	0.000185
GPS position stdev (<0.01m)	0.000500
<i>Minimum % overlap (>25)</i>	
Ave point cloud density per sq.m. (>2.0)	0.0089
Elevation difference between strips (<0.20m)	24.96%
<i>Number of 1km x 1km blocks</i>	
Maximum Height	306
Minimum Height	537.22 m
<i>Classification (# of points)</i>	
Ground	62.35 m
Low vegetation	221,447,994
Medium vegetation	167,724,982
High vegetation	312,332,327
Building	222,386,814
Orthophoto	4,523,324
Processed by	Yes
	Engr. Angelo Carlo Bongat, Engr. Antonio Chua Jr., AilynBiñas



Figure A-8.1 *Solution Status*

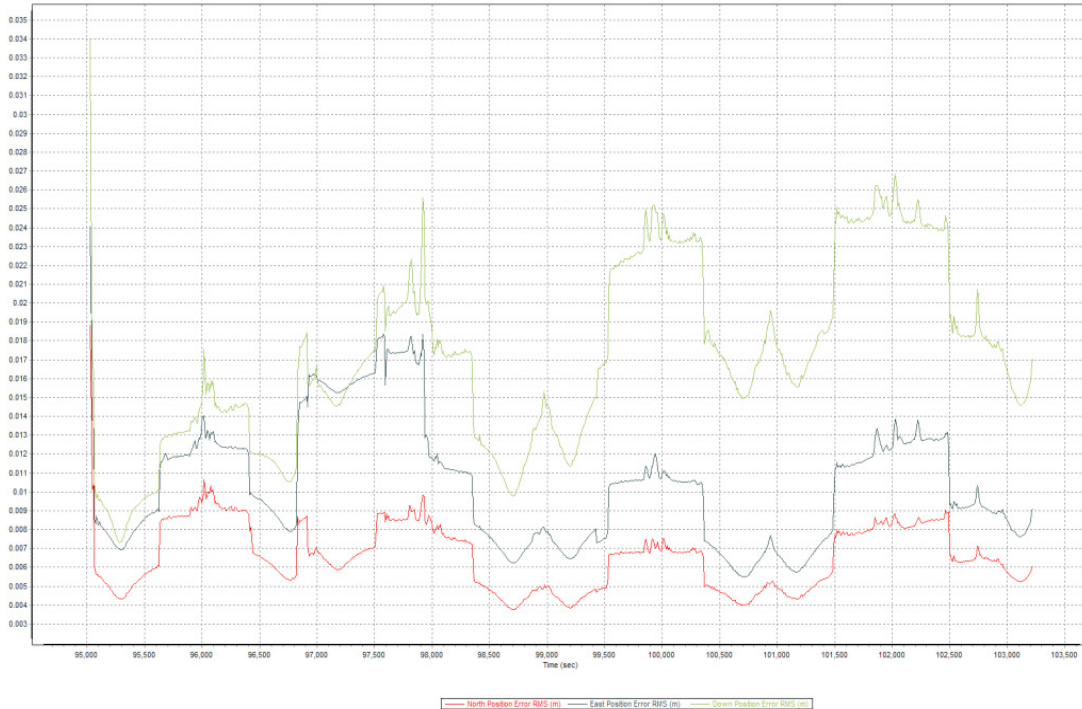


Figure A-8.2 *Smoothed Performance Metric Parameters*

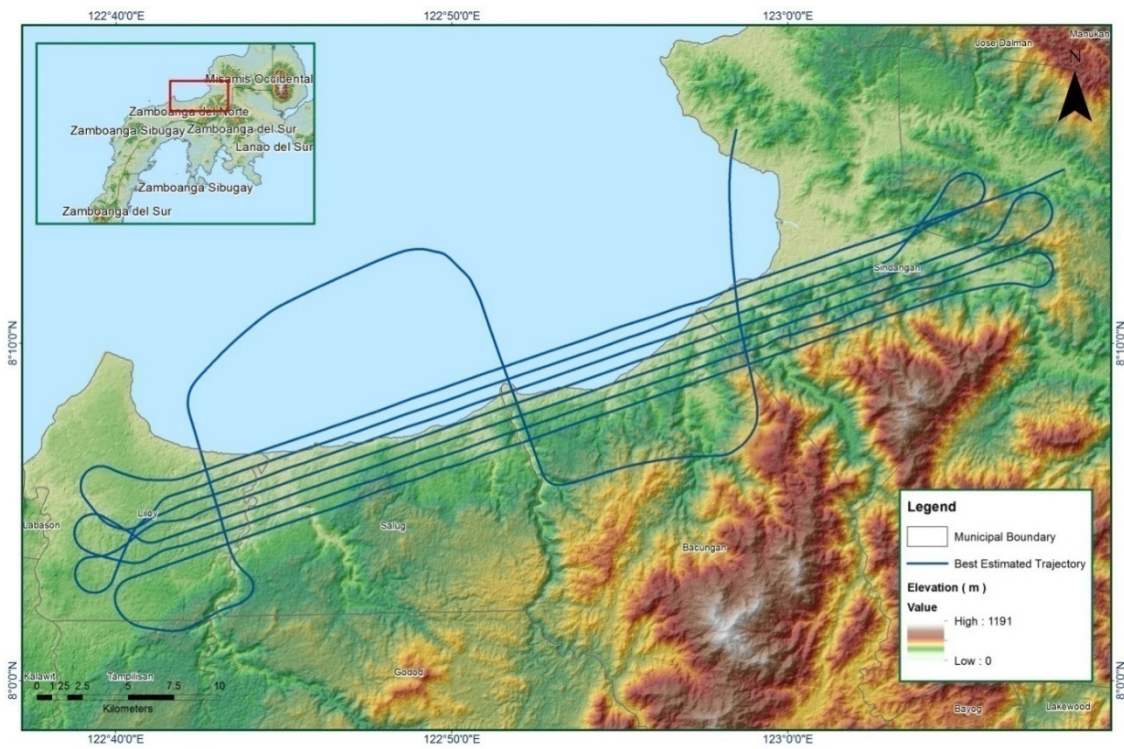


Figure A-8.3 *Best Estimated Trajectory*

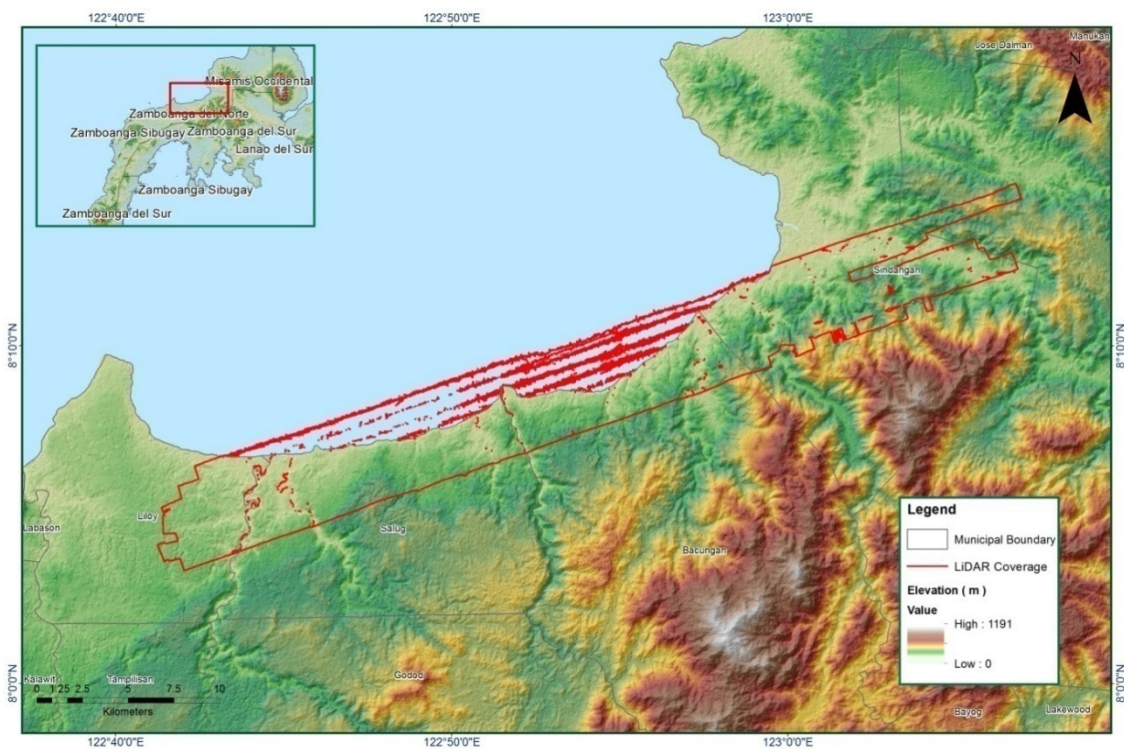


Figure A-8.4 *Coverage of LiDAR Data*

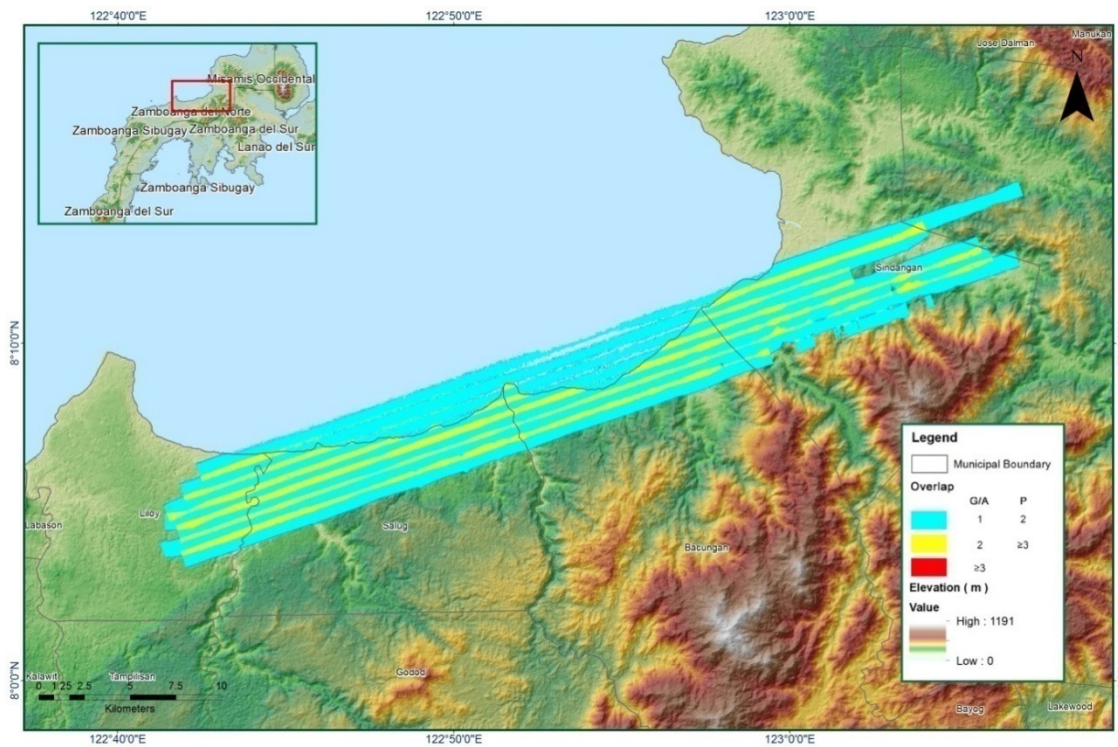


Figure A-8.5 *Image of Data Overlap*

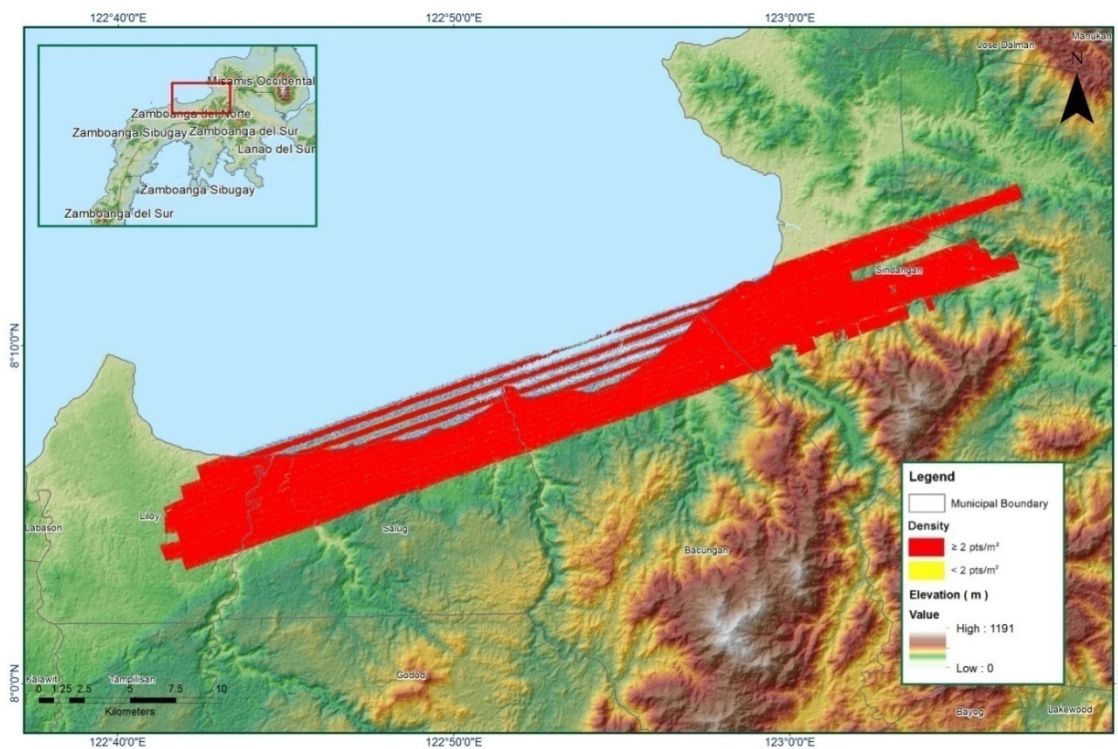


Figure A-8.6 *Density map of merged LiDAR data*

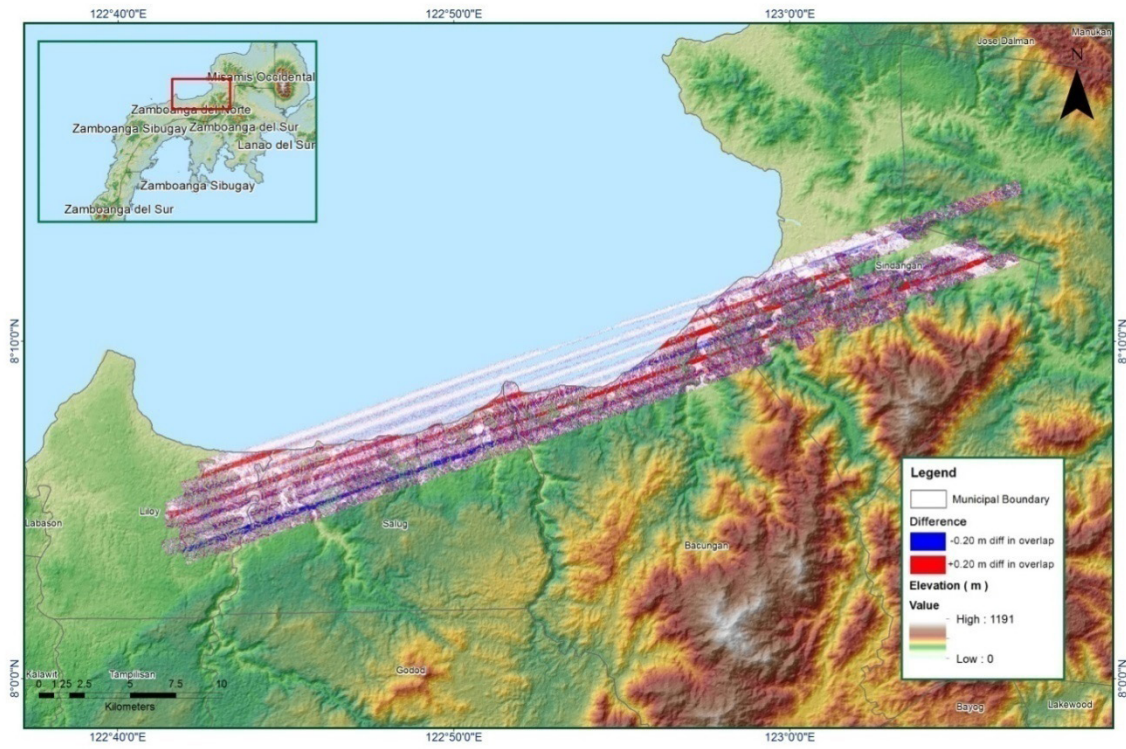


Figure A-8.7 Elevation difference between flight lines

Annex 9. Palandok Model Basin Parameters

Table A-9.1. Palandok Model Basin Parameters

Basin Number	SCS Curve Number Loss			Clark Unit Hydrograph Transform		Recession Baseflow				
	Initial Abstraction (mm)	Curve Number	Impervious (%)	Time of Concentration (HR)	Storage Coefficient (HR)	Initial Type	Initial Discharge (M3/S)	Recession Constant	Threshold Type	Ratio to Peak
W520	29.214	23.8204	0	0.17777	0.0454376	Discharge	0.0660664	0.05	Ratio to Peak	0.1
W530	40.331	18.0064	0	0.1606	0.0410470	Discharge	0.0162005	0.05	Ratio to Peak	0.1
W540	31.380	22.3078	0	0.15986	0.0408591	Discharge	0.0636676	0.05	Ratio to Peak	0.1
W550	41.089	17.4478	0	0.1296	0.0331256	Discharge	0.0154366	0.05	Ratio to Peak	0.1
W560	50.568	21.118	0	0.35707	0.0912628	Discharge	0.17635	0.05	Ratio to Peak	0.1
W570	29.176	23.3116	0	0.1603	0.0409718	Discharge	0.0330883	0.05	Ratio to Peak	0.1
W580	41.605	24.5722	0	0.44246	0.25444	Discharge	0.0222793	0.05	Ratio to Peak	0.1
W590	38.268	25.5934	0	0.88125	0.33924	Discharge	0.0577924	0.05	Ratio to Peak	0.1
W600	41.670	24.4618	0	1.7038	0.65640	Discharge	.000805202	0.05	Ratio to Peak	0.1
W610	58.100	25.3246	0	0.5036	0.19345	Discharge	0.16801	0.05	Ratio to Peak	0.1
W620	40.722	24.8026	0	0.93477	0.53756	Discharge	0.0705608	0.05	Ratio to Peak	0.1
W630	47.355	22.0252	0	0.20889	0.12013	Discharge	0.0691341	0.05	Ratio to Peak	0.1
W640	39.400	25.0228	0	0.1258	0.0321531	Discharge	0.0447454	0.05	Ratio to Peak	0.1
W650	67.623	22.5814	0	0.20606	0.0526660	Discharge	0.0517335	0.05	Ratio to Peak	0.1
W660	39.801	24.9124	0	0.40485	0.23282	Discharge	0.0638008	0.05	Ratio to Peak	0.1
W670	38.689	24.9898	0	0.073444	0.0275933	Discharge	0.0125801	0.05	Ratio to Peak	0.1
W680	71.712	21.6382	0	0.12683	0.0729370	Discharge	0.0375338	0.05	Ratio to Peak	0.1
W690	37.706	25.4212	0	0.19349	0.0494547	Discharge	0.0505364	0.05	Ratio to Peak	0.1
W700	36.825	26.407	0	0.32345	0.12152	Discharge	.000913889	0.05	Ratio to Peak	0.1
W710	39.484	23.4736	0	0.24841	0.0634910	Discharge	0.0841528	0.05	Ratio to Peak	0.1
W720	22.661	27.919	0	0.044448	0.0167	Discharge	0.0028320	0.05	Ratio to Peak	0.1
W730	58.194	25.2844	0	0.093445	0.0351083	Discharge	0.0250914	0.05	Ratio to Peak	0.1
W740	60.621	17.3434	0	0.12926	0.0495557	Discharge	0.0513156	0.05	Ratio to Peak	0.1
W750	25.478	25.9642	0	0.13807	0.0352892	Discharge	0.0594457	0.05	Ratio to Peak	0.1
W760	78.659	20.1472	0	0.1104	0.0414816	Discharge	0.0216440	0.05	Ratio to Peak	0.1
W770	64.365	16.1692	0	0.14161	0.0535070	Discharge	0.0420114	0.05	Ratio to Peak	0.1
W780	75.356	21.028	0	0.11213	0.0421300	Discharge	0.0349697	0.05	Ratio to Peak	0.1
W790	87.062	18.1222	0	0.13824	0.0795005	Discharge	0.0441330	0.05	Ratio to Peak	0.1
W800	99	15.5734	0	0.18143	0.0698525	Discharge	0.0811310	0.05	Ratio to Peak	0.1
W810	77.144	20.6212	0	0.19339	0.0494288	Discharge	0.10794	0.05	Ratio to Peak	0.1
W820	38.596	25.5274	0	0.27774	0.15972	Discharge	0.10837	0.05	Ratio to Peak	0.1
W830	39.695	24.9592	0	0.13592	0.0522408	Discharge	0.0149866	0.05	Ratio to Peak	0.1
W840	38.674	25.4104	0	0.34394	0.19780	Discharge	0.10155	0.05	Ratio to Peak	0.1
W850	38.246	25.6	0	0.19419	0.0748186	Discharge	0.0378369	0.05	Ratio to Peak	0.1
W860	63.789	16.3798	0	0.18944	0.10895	Discharge	0.0576761	0.05	Ratio to Peak	0.1
W870	97.964	15.724	0	0.14479	0.0832639	Discharge	0.0461292	0.05	Ratio to Peak	0.1
W880	64.146	16.1656	0	0.17589	0.10115	Discharge	0.0471763	0.05	Ratio to Peak	0.1
W890	66.107	15.5788	0	0.20343	0.11699	Discharge	0.0573990	0.05	Ratio to Peak	0.1
W900	38.057	25.6	0	0.25578	0.0980613	Discharge	0.0053532	0.05	Ratio to Peak	0.1
W910	50.206	20.0866	0	0.34022	0.19565	Discharge	0.0505900	0.05	Ratio to Peak	0.1
W920	46.769	22.5676	0	0.3163	0.18190	Discharge	0.16196	0.05	Ratio to Peak	0.1
W930	45.975	22.8514	0	0.34311	0.13154	Discharge	0.0936698	0.05	Ratio to Peak	0.1
W940	69.871	22.5688	0	0.24602	0.0943214	Discharge	0.0874288	0.05	Ratio to Peak	0.1
W950	101.2685089	15.1252	0	0.25458	0.0977113		0.0668134	0.05	Ratio to Peak	0.1
W960	66.163	15.4846	0	0.26294	0.15121		0.11058	0.05	Ratio to Peak	0.1
W970	97.815	15.7588	0	0.14743	0.0567301		0.0416394	0.05	Ratio to Peak	0.1
W980	47.1673079	22.0138	0	0.20811	0.11968		0.11541	0.05	Ratio to Peak	0.1
W990	50.272	20.6044	0	0.18845	0.10837		0.11720	0.05	Ratio to Peak	0.1
W1000	40.0029865	24.7828	0	0.2451	0.14095		0.0975443	0.05	Ratio to Peak	0.1
W1010	94.024	16.5028	0	0.14203	0.0816806		0.0415261	0.05	Ratio to Peak	0.1
W1020	86.422	18.004	0	0.18719	0.0716896		0.0632574	0.05	Ratio to Peak	0.1

Annex 10. Palandok Model Reach Parameters

Table A-10.1. Palandok Model Reach Parameters

Reach Number	Muskingum Cunge Channel Routing						
	Time Step Method	Length (m)	Slope	Manning's n	Shape	Width	Side Slope
R10	Automatic Fixed Interval	2411.5	0.0404465	0.044	Trapezoid	30	0.01
R30	Automatic Fixed Interval	1877.1	0.0223902	0.044	Trapezoid	30	0.01
R40	Automatic Fixed Interval	892.67	0.0138171	0.044	Trapezoid	30	0.01
R60	Automatic Fixed Interval	695.27	0.0836177	0.044	Trapezoid	30	0.01
R70	Automatic Fixed Interval	38.284	0.0029422	0.044	Trapezoid	30	0.01
R80	Automatic Fixed Interval	1694.1	0.0332265	0.044	Trapezoid	30	0.01
R110	Automatic Fixed Interval	2473.5	0.0158731	0.044	Trapezoid	30	0.01
R120	Automatic Fixed Interval	28.284	0.11095	0.044	Trapezoid	30	0.01
R140	Automatic Fixed Interval	268.70	0.0218626	0.044	Trapezoid	30	0.01
R170	Automatic Fixed Interval	1363.3	0.0455737	0.044	Trapezoid	30	0.01
R180	Automatic Fixed Interval	1252.3	0.0235759	0.044	Trapezoid	30	0.01
R200	Automatic Fixed Interval	1504.3	0.0351911	0.044	Trapezoid	30	0.01
R210	Automatic Fixed Interval	1824.1	0.0443836	0.044	Trapezoid	30	0.01
R230	Automatic Fixed Interval	3103.5	0.0094950	0.044	Trapezoid	30	0.01
R240	Automatic Fixed Interval	1866.8	0.0133855	0.044	Trapezoid	30	0.01
R270	Automatic Fixed Interval	5038.9	0.0040137	0.044	Trapezoid	30	0.01
R290	Automatic Fixed Interval	759.83	0.0073660	0.044	Trapezoid	30	0.01
R310	Automatic Fixed Interval	3550.9	0.0408339	0.044	Trapezoid	30	0.01
R340	Automatic Fixed Interval	1952.4	0.0755265	0.044	Trapezoid	30	0.01
R350	Automatic Fixed Interval	7073.8	0.0019376	0.044	Trapezoid	30	0.01
R370	Automatic Fixed Interval	565.69	0.0178275	0.044	Trapezoid	30	0.01
R380	Automatic Fixed Interval	4188.5	0.0081160	0.044	Trapezoid	30	0.01
R390	Automatic Fixed Interval	2085.8	0.0109545	0.044	Trapezoid	30	0.01
R410	Automatic Fixed Interval	9262.4	0.0049640	0.044	Trapezoid	30	0.01
R490	Automatic Fixed Interval	3389.6	0.0060254	0.044	Trapezoid	30	0.01

Annex 11. Palandok Field Validation Points

Table A-11.1. Palandok Field Validation

Point Number	Validation Coordinates		Model Var (m)	Validation Points (m)	Error	Event/Date	Rain Return / Scenario
	Lat	Long					
1	8.145988	122.862	0.03	0.50	-0.47	Amihan	5 -Year
2	8.14466	122.8639	0.03	0.30	-0.27	Amihan	5 -Year
3	8.142812	122.86477	0.03	0.65	-0.62	Amihan	5 -Year
4	8.142165	122.86497	0.03	0.50	-0.47	Amihan	5 -Year
5	8.131745	122.86362	5.88	0.30	5.58	Typhoon Lawin	5 -Year
6	8.130237	122.86358	5.21	3.00	2.21	Typhoon Lawin	5 -Year
7	8.129563	122.86342	4.24	2.00	2.24	Typhoon Lawin	5 -Year
8	8.128729	122.86322	10.16	2.00	8.16	Typhoon Lawin	5 -Year
9	8.127672	122.86354	9.15	2.00	7.15	Typhoon Lawin	5 -Year
10	8.127386	122.86384	8.04	2.00	6.04	Typhoon Lawin	5 -Year
11	8.12686	122.86402	8.93	2.00	6.93	Typhoon Lawin	5 -Year
12	8.126106	122.86483	14.25	2.00	12.25	Typhoon Lawin	5 -Year
13	8.129765	122.86344	2.86	2.00	0.86	Typhoon Lawin	5 -Year
14	8.139233	122.85906	4.18	2.00	2.18	Typhoon Lawin	5 -Year
15	8.139584	122.85872	4.05	2.00	2.05	Typhoon Lawin	5 -Year
16	8.14039	122.85871	1.02	2.00	-0.98	Typhoon Lawin	5 -Year
17	8.139148	122.85813	0.06	2.00	-1.94	Typhoon Lawin	5 -Year
18	8.140008	122.85792	0.03	2.00	-1.97	Typhoon Lawin	5 -Year
19	8.140699	122.85786	3.63	2.00	1.63	Typhoon Lawin	5 -Year
20	8.139924	122.85592	0.03	0.00	0.03	Typhoon Lawin	5 -Year
21	8.139382	122.8555	0.03	0.00	0.03	Typhoon Lawin	5 -Year
22	8.139	122.85526	0.03	0.00	0.03	Typhoon Lawin	5 -Year
23	8.138734	122.85497	0.03	0.00	0.03	Typhoon Lawin	5 -Year
24	8.138612	122.85534	0.03	0.00	0.03	Typhoon Lawin	5 -Year
25	8.138357	122.85457	0.03	0.00	0.03	Typhoon Lawin	5 -Year

Point Number	Validation Coordinates		Model Var (m)	Validation Points (m)	Error	Event/Date	Rain Return / Scenario
	Lat	Long					
26	8.138706	122.85482	0.03	0.00	0.03	Typhoon Lawin	5 -Year
27	8.139616	122.85603	0.03	0.00	0.03	Typhoon Lawin	5 -Year
28	8.141612	122.86547	0.03	0.00	0.03	Amihan	5 -Year
29	8.14168	122.86557	0.03	0.00	0.03	Amihan	5 -Year
30	8.141361	122.86611	0.10	0.00	0.10	Amihan	5 -Year
31	8.14183	122.8651	0.03	0.00	0.03	Typhoon Lawin	5 -Year
32	8.141525	122.86505	0.08	0.00	0.08	Amihan	5 -Year
33	8.142033	122.86471	0.07	0.00	0.07	Amihan	5 -Year
34	8.14355	122.86435	0.05	0.00	0.05	Amihan	5 -Year
35	8.142393	122.86454	0.08	0.00	0.08	Amihan	5 -Year
36	8.14294	122.8639	0.05	0.00	0.05	Amihan	5 -Year
37	8.145053	122.86342	0.03	0.00	0.03	Typhoon Lawin	5 -Year
38	8.143843	122.86393	0.03	0.00	0.03		5 -Year
39	8.142382	122.86425	0.08	0.00	0.08	Typhoon Lawin	5 -Year
40	8.144156	122.86353	0.07	0.00	0.07	Amihan	5 -Year
41	8.143556	122.86351	0.08	0.00	0.08	Amihan	5 -Year
42	8.144261	122.85897	0.11	0.20	-0.10	Typhoon lawin	5 -Year
43	8.14439	122.85898	0.31	0.20	0.11	Typhoon Lawin	5 -Year
44	8.146264	122.86065	0.03	0.50	-0.47	Amihan	5 -Year
45	8.144363	122.86403	0.03	0.50	-0.47	Amihan	5 -Year
46	8.143174	122.86444	0.03	0.50	-0.47	Amihan	5 -Year
47	8.142722	122.86487	0.03	0.20	-0.17	Amihan	5 -Year
48	8.142176	122.86487	0.03	0.20	-0.17	Amihan	5 -Year
49	8.137098	122.86026	0.07	0.30	-0.23	Typhoon lawin	5 -Year
50	8.13559	122.86066	0.03	0.30	-0.27	Typhoon Lawin	5 -Year
51	8.132351	122.86401	0.03	3.00	-2.97	Typhoon lawin	5 -Year
52	8.13146	122.86392	5.69	3.00	2.69	Typhoon Lawin	5 -Year
53	8.13075	122.86378	5.23	2.00	3.23	Typhoon lawin	5 -Year
54	8.129712	122.8635	2.89	3.00	-0.11	Typhoon Lawin	5 -Year
55	8.129138	122.86361	3.41	2.00	1.41	Typhoon lawin	5 -Year
56	8.128817	122.86358	3.84	3.00	0.84	Typhoon Lawin	5 -Year
57	8.128299	122.86381	4.00	3.00	1.00	Typhoon Lawin	5 -Year
58	8.127986	122.86384	5.68	2.00	3.68	Typhoon lawin	5 -Year
59	8.12812	122.864	3.32	3.00	0.32	Typhoon Lawin	5 -Year
60	8.127424	122.86416	5.00	3.00	2.00	Typhoon Lawin	5 -Year
61	8.12691	122.86461	4.04	3.00	1.04	Typhoon Lawin	5 -Year
62	8.126683	122.86467	5.09	3.00	2.09	Typhoon lawin	5 -Year
63	8.126509	122.8646	8.29	3.00	5.29	Typhoon lawin	5 -Year
64	8.126426	122.86489	2.78	3.00	-0.22	Typhoon Lawin	5 -Year
65	8.129846	122.86352	2.65	3.00	-0.35	Typhoon	5 -Year
66	8.139414	122.85893	4.20	2.00	2.20	Typhoon Lawin	5 -Year
67	8.141637	122.85986	0.36	2.00	-1.64	Typhoon lawin	5 -Year
68	8.139764	122.85843	3.93	0.10	3.83	Typhoon Lawin	5 -Year
69	8.138678	122.85822	0.09	0.10	-0.01	Typhoon lawin	5 -Year
70	8.138458	122.85841	0.03	0.10	-0.07	Typhoon	5 -Year
71	8.139845	122.85792	0.03	0.10	-0.07	Typhoon Lawin	5 -Year
72	8.140263	122.85784	0.03	0.10	-0.07	Typhoon Lawin	5 -Year
73	8.142562	122.85735	0.03	0.10	-0.07	Typhoon lawin	5 -Year
74	8.142706	122.8574	0.03	0.00	0.03		5 -Year
75	8.139873	122.85641	0.03	0.00	0.03	Amihan	5 -Year
76	8.139971	122.85596	0.03	0.05	-0.02	Amihan	5 -Year
77	8.139638	122.8559	0.03	0.05	-0.02	Amihan	5 -Year

Point Number	Validation Coordinates		Model Var (m)	Validation Points (m)	Error	Event/Date	Rain Return / Scenario
	Lat	Long					
78	8.139441	122.85565	0.03	0.10	-0.07	Amihan	5 -Year
79	8.139217	122.85541	0.03	0.05	-0.02	Amihan	5 -Year
80	8.139029	122.85531	0.03	0.00	0.03	Amihan	5 -Year
81	8.138782	122.85504	0.03	0.00	0.03	Amihan	5 -Year
82	8.138771	122.855	0.03	0.07	-0.04	Amihan	5 -Year
83	8.138616	122.85479	0.03	0.00	0.03	Amihan	5 -Year
84	8.138442	122.85419	0.14	0.10	0.04	Amihan	5 -Year
85	8.137985	122.854	0.03	0.00	0.03	Amihan	5 -Year
86	8.138224	122.85399	0.03	0.00	0.03	Amihan	5 -Year
87	8.14121	122.86658	0.04	0.10	-0.06	Typhoon lawin	5 -Year
88	8.141187	122.86637	0.04	0.00	0.04	Amihan	5 -Year
89	8.141254	122.86622	0.04	0.00	0.04	Amihan	5 -Year
90	8.141363	122.86613	0.10	0.05	0.05	Amihan	5 -Year
91	8.141341	122.86597	0.07	0.04	0.03	Amihan	5 -Year
92	8.141363	122.86583	0.03	0.00	0.03	Amihan	5 -Year
93	8.141479	122.8656	0.03	0.06	-0.03	Amihan	5 -Year
94	8.141928	122.8654	0.10	0.05	0.05	Amihan	5 -Year
95	8.141498	122.86516	0.04	0.00	0.04	Amihan	5 -Year
96	8.142022	122.86518	0.03	0.00	0.03	Amihan	5 -Year
97	8.141794	122.86517	0.03	0.06	-0.03	Amihan	5 -Year
98	8.142803	122.86461	0.03	0.00	0.03	Amihan	5 -Year
99	8.141639	122.86146	0.03	0.00	0.03		5 -Year
100	8.141256	122.86225	0.03	0.00	0.03		5 -Year
101	8.141319	122.86254	0.03	0.00	0.03		5 -Year
102	8.143002	122.86196	0.05	0.00	0.05		5 -Year
103	8.127181	122.86779	0.03	0.00	0.03		5 -Year
104	8.127149	122.86782	0.03	0.00	0.03		5 -Year
105	8.143483	122.86138	0.03	0.00	0.03		5 -Year
106	8.144744	122.8609	0.03	0.00	0.03		5 -Year
107	8.145779	122.85985	0.16	0.00	0.16		5 -Year
108	8.145327	122.85914	0.07	0.00	0.07		5 -Year

Point Number	Validation Coordinates		Model Var (m)	Validation Points (m)	Error	Event/Date	Rain Return / Scenario
	Lat	Long					
109	8.145788	122.85894	0.03	0.00	0.03		5 -Year
110	8.141932	122.86405	0.06	0.00	0.06		5 -Year
111	8.142024	122.86446	0.03	0.00	0.03		5 -Year
112	8.141418	122.86321	0.03	0.00	0.03		5 -Year
113	8.141272	122.86347	0.03	0.00	0.03		5 -Year
114	8.140536	122.86457	0.14	0.00	0.14		5 -Year
115	8.140298	122.86625	0.03	0.00	0.03		5 -Year
116	8.139604	122.86576	0.03	0.00	0.03		5 -Year
117	8.139489	122.86551	0.03	0.00	0.03		5 -Year
118	8.141082	122.86507	0.96	0.00	0.96		5 -Year
119	8.138168	122.85481	0.03	0.00	0.03		5 -Year
120	8.138797	122.85552	0.03	0.00	0.03		5 -Year
121	8.139213	122.85571	0.08	0.00	0.08		5 -Year
122	8.138892	122.85628	0.03	0.00	0.03		5 -Year
123	8.138507	122.85623	0.03	0.00	0.03		5 -Year
124	8.138877	122.85687	0.03	0.00	0.03		5 -Year
125	8.138771	122.85738	0.07	0.00	0.07		5 -Year
126	8.139397	122.85766	0.03	0.00	0.03		5 -Year
127	8.138974	122.858	0.07	0.00	0.07		5 -Year
128	8.143975	122.86149	0.03	0.00	0.03		5 -Year
129	8.140854	122.86245	0.03	0.00	0.03		5 -Year
130	8.144483	122.8625	0.03	0.00	0.03		5 -Year
131	8.139721	122.85988	0.03	0.00	0.03		5 -Year
132	8.139715	122.85989	0.03	0.00	0.03		5 -Year
133	8.133365	122.86543	0.03	0.00	0.03		5 -Year
134	8.133363	122.86544	0.03	0.00	0.03		5 -Year
135	8.131118	122.86641	0.03	0.00	0.03		5 -Year
136	8.142253	122.86137	0.03	0.00	0.03		5 -Year
137	8.142871	122.86103	0.03	0.00	0.03		5 -Year
138	8.143971	122.86084	0.03	0.00	0.03		5 -Year
139	8.145191	122.85952	0.03	0.00	0.03		5 -Year
140	8.143929	122.8628	0.03	0.00	0.03		5 -Year
141	8.141207	122.86399	0.07	0.00	0.07		5 -Year
142	8.142041	122.86341	0.06	0.00	0.06		5 -Year
143	8.142564	122.8632	0.07	0.00	0.07		5 -Year
144	8.140459	122.86414	0.12	0.00	0.12		5 -Year
145	8.139878	122.86497	0.05	0.00	0.05		5 -Year
146	8.140386	122.86512	0.04	0.00	0.04		5 -Year
147	8.140639	122.86552	0.05	0.00	0.05		5 -Year
148	8.139175	122.86584	0.03	0.00	0.03		5 -Year
149	8.141465	122.86461	0.03	0.00	0.03		5 -Year
150	8.1381	122.8552	0.03	0.00	0.03		5 -Year
151	8.13904	122.85552	0.03	0.00	0.03		5 -Year
152	8.138338	122.85717	0.03	0.00	0.03		5 -Year
153	8.138517	122.85767	0.03	0.00	0.03		5 -Year
154	8.138292	122.85775	0.03	0.00	0.03		5 -Year
155	8.138204	122.85839	0.03	0.00	0.03		5 -Year
156	8.139776	122.85767	0.03	0.00	0.03		5 -Year
157	8.140158	122.85758	0.03	0.00	0.03		5 -Year
158	8.141297	122.85977	0.45	0.00	0.45		5 -Year
159	8.140192	122.86028	0.03	0.00	0.03		5 -Year

Point Number	Validation Coordinates		Model Var (m)	Validation Points (m)	Error	Event/Date	Rain Return / Scenario
	Lat	Long					
160	8.141078	122.8602	0.03	0.00	0.03		5 -Year
161	8.136484	122.8605	0.03	0.00	0.03		5 -Year
162	8.141006	122.86147	0.08	0.00	0.08		5 -Year
163	8.141797	122.86203	0.03	0.00	0.03		5 -Year
164	8.142081	122.86202	0.03	0.00	0.03		5 -Year
165	8.142167	122.86228	0.03	0.00	0.03		5 -Year
166	8.142377	122.86259	0.03	0.00	0.03		5 -Year
167	8.142149	122.86271	0.05	0.00	0.05		5 -Year
168	8.134936	122.86408	0.03	0.00	0.03		5 -Year
169	8.129169	122.8674	0.03	0.00	0.03		5 -Year
170	8.126708	122.86735	0.03	0.00	0.03		5 -Year
171	8.142464	122.86179	0.03	0.00	0.03		5 -Year
172	8.144464	122.86115	0.05	0.00	0.05		5 -Year
173	8.145272	122.86093	0.03	0.00	0.03		5 -Year
174	8.14483	122.8618	0.03	0.00	0.03		5 -Year
175	8.144585	122.86175	0.03	0.00	0.03		5 -Year
176	8.143788	122.86227	0.06	0.00	0.06		5 -Year
177	8.143592	122.8623	0.09	0.00	0.09		5 -Year
178	8.143391	122.86306	0.03	0.00	0.03		5 -Year
179	8.141691	122.86444	0.05	0.00	0.05		5 -Year
180	8.14185	122.86271	0.04	0.00	0.04		5 -Year
181	8.140692	122.86405	0.03	0.00	0.03		5 -Year
182	8.139419	122.86472	0.03	0.00	0.03		5 -Year
183	8.139423	122.86473	0.03	0.00	0.03		5 -Year
184	8.140242	122.865	0.06	0.00	0.06		5 -Year
185	8.140772	122.86621	0.32	0.00	0.32		5 -Year
186	8.139985	122.866	0.03	0.00	0.03		5 -Year
187	8.140953	122.86093	0.03	0.00	0.03		5 -Year
188	8.139502	122.85618	0.03	0.00	0.03		5 -Year
189	8.139272	122.85599	0.04	0.00	0.04		5 -Year
				RMSE	1.66		

Annex 12. Educational Institutions affected by flooding in Palandok Floodplain

Table A-12.1. Educational Institutions affected by flooding in Palandok Floodplain

Bacungan				
Barangay	Buildings	Rainfall Scenario		
		5-year	25-year	100-year
Rizon	Arabic center	None	None	None
Rizon	Palandok Elementary School	None	None	None
Rizon	Palandok National High School	None	None	None
Rizon	Palandok Day care center	None	None	None
Rizon	PALANDOK SCHOOL	None	None	None

Annex 13. Health Institutions affected by flooding in Palandok Floodplain

Table A-13.1. Health Institutions affected by flooding in Palandok Floodplain

Bacungan				
Barangay	Buildings	Rainfall Scenario		
		5-year	25-year	100-year
Rizon	Brgay. Health Center	None	None	None

